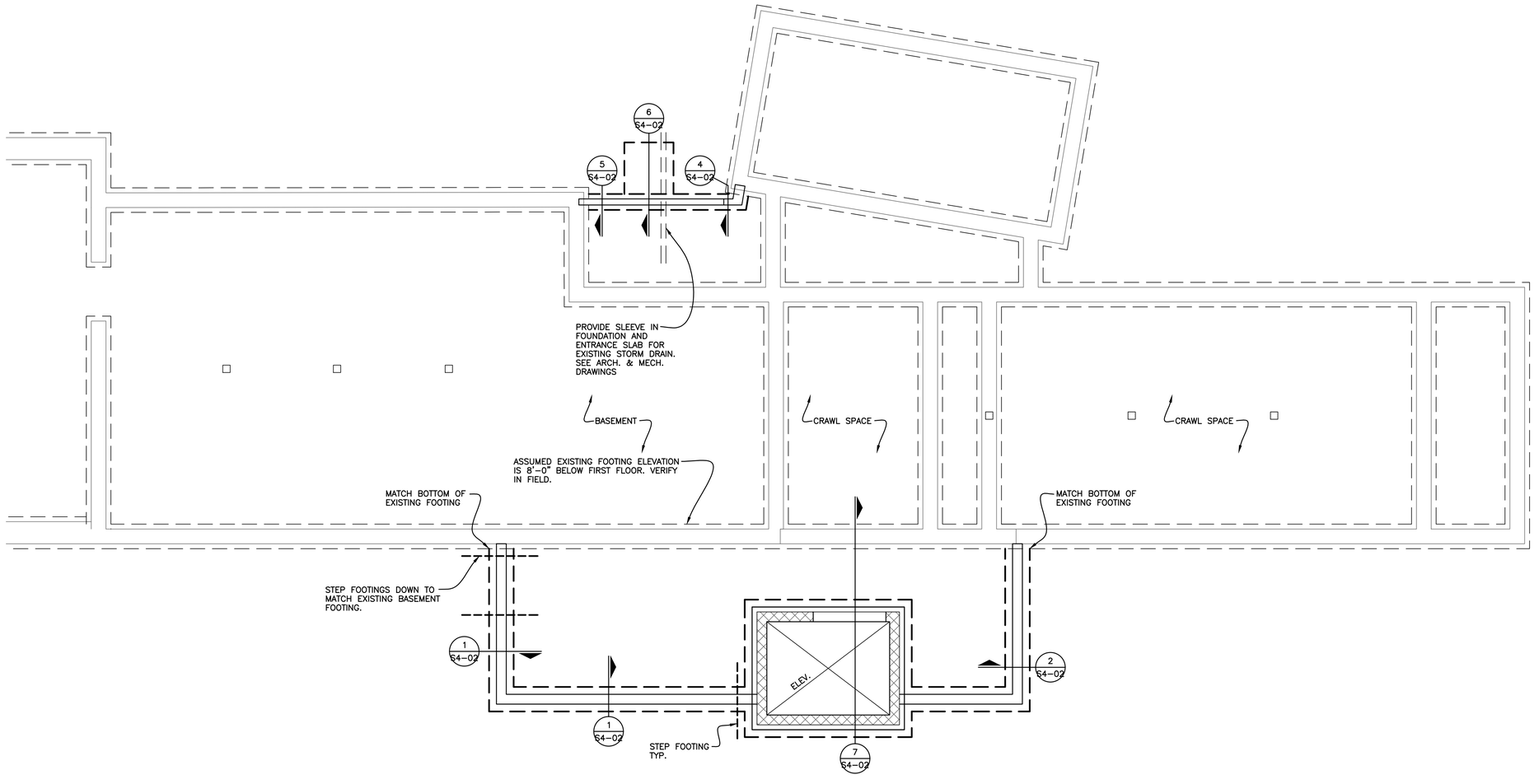

NORTH

UNIT A
FOUNDATION PLAN

SCALE : 1/4" = 1'-0"

BOTTOM OF FOOTING = 94'-5 3/4".
STEP DOWN TO MATCH EXISTING FOOTING ELEVATIONS.

NOTE "A": REINFORCE MASONRY WALLS OF ELEVATOR WITH #5 @ 32" O.C. FULL HEIGHT VERTICAL, UNLESS OTHERWISE NOTED GROUT REINFORCED CORES SOLID, DOWEL TO FOOTING. PLACE BARS AT CORNERS AND FACE OF OPENINGS AND EACH SIDE OF CONTROL JOINTS. SEE DETAILS 10, 11, 12, 13, 14, & 15/S4-02

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KEY PLAN

C B

OWNER

City of Dexter

PROJECT NAME

City Hall Renovations

3515 Broad St.
Dexter, MI 48130

PROJECT NO.

21-113

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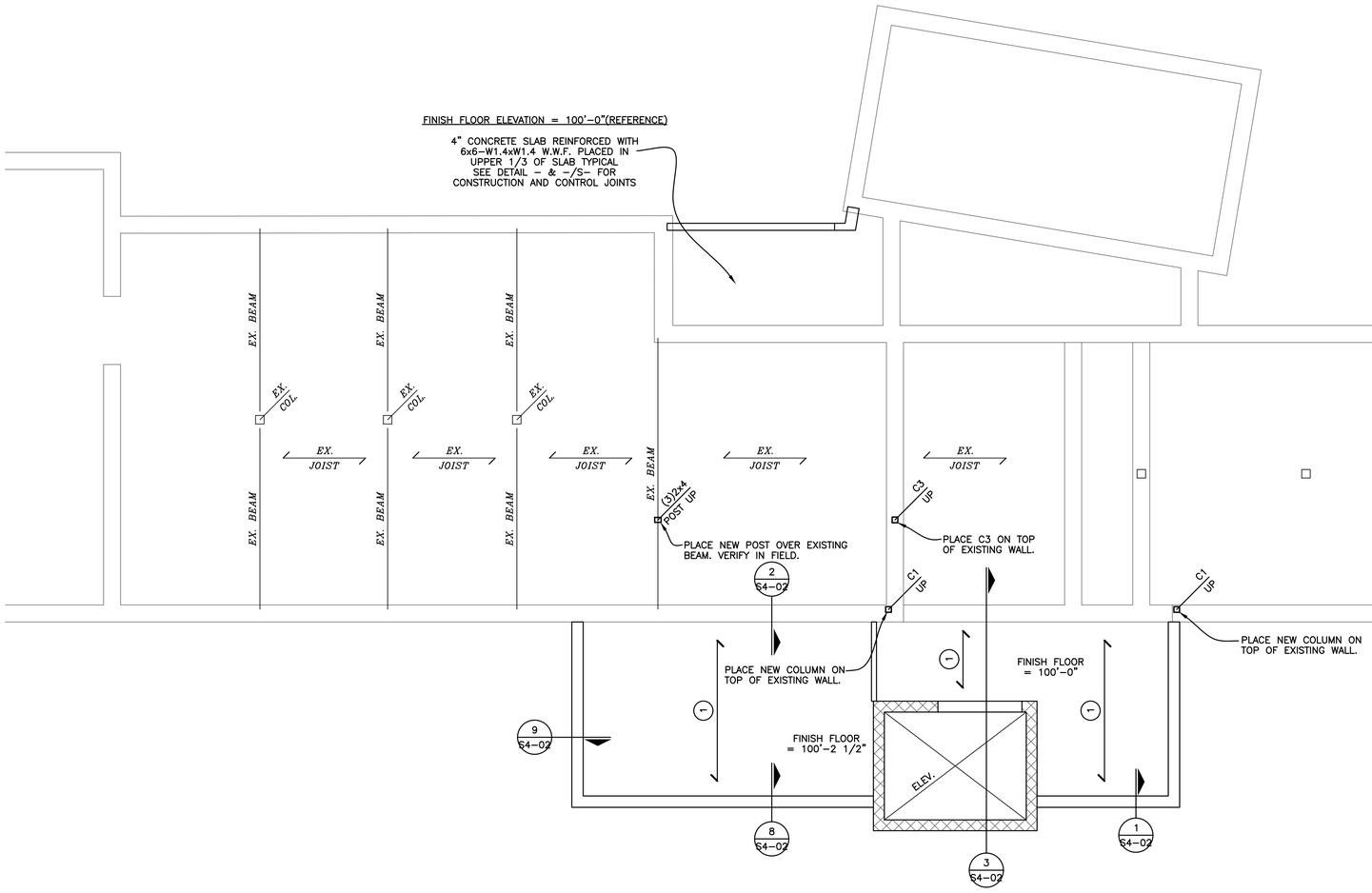
SHEET NAME

UNIT A
FOUNDATION PLAN

SHEET NO.

S3-01

MATERIAL SCHEDULE		
MARK	DESCRIPTION	LIVE LOAD
1	2x12 @ 16" O.C.	
2	2x12 SLOPED ROOF RAFTER @ 16" O.C.	
3	2x8 @ 16" O.C.	
4	2x10 @ 16" O.C.	
5	PRE-ENGINEERED WOOD ROOF TRUSSES 24" O.C. BY TRUSS SUPPLIER	



NORTH

UNIT A
FIRST FLOOR FRAMING PLAN

SCALE : 1/4" = 1'-0"

TYPICAL FLOOR SHEATHING IS 3/4" T&G SHEATHING. STAGGER JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW) GLUED AND SCREWED 6" O.C. AT EDGES AND 12" O.C. IN THE FIELD

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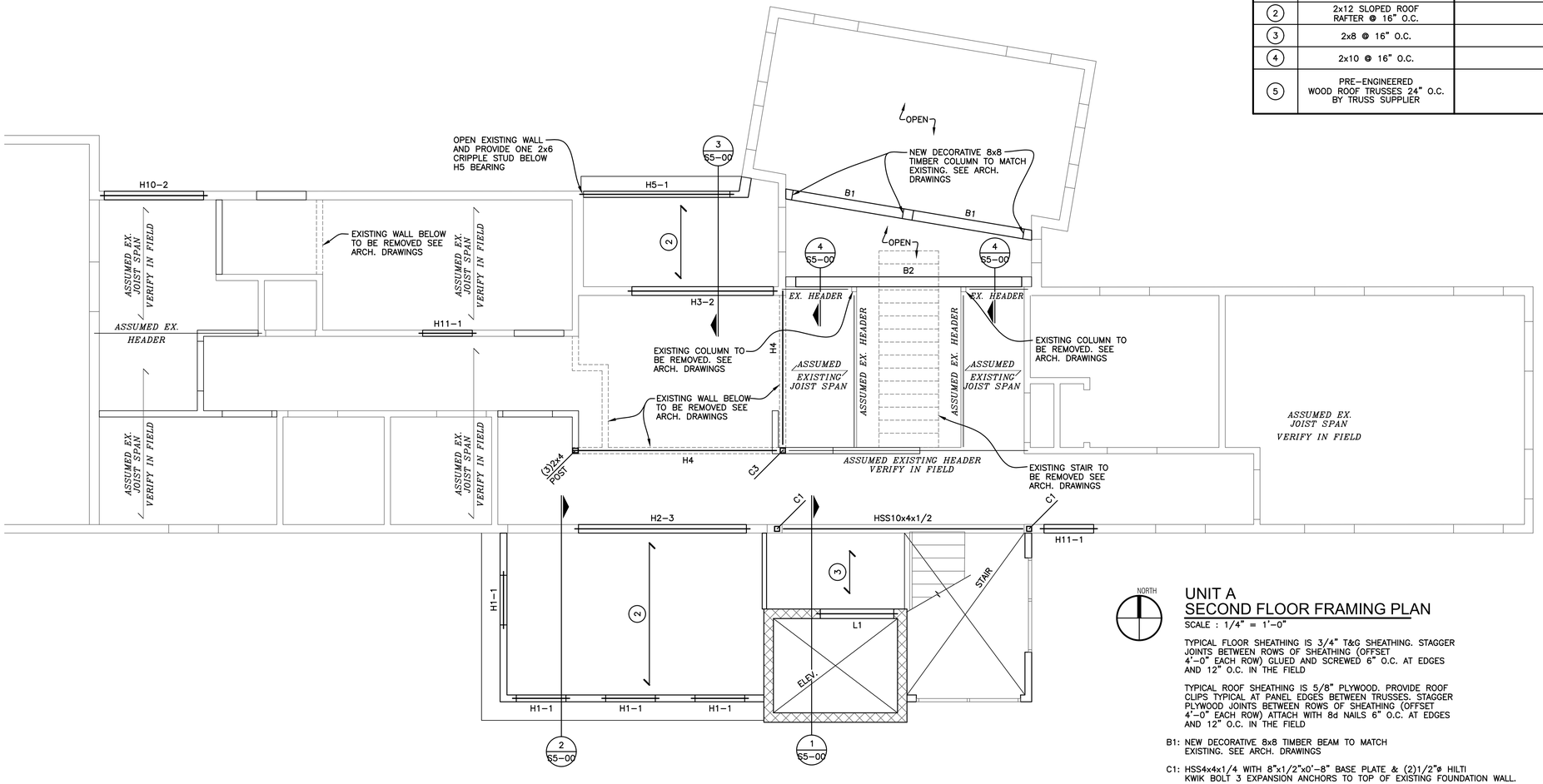
SHEET NAME
UNIT A
FIRST FLOOR
FRAMING PLAN

SHEET NO.
S3-02

MATERIAL SCHEDULE		
MARK	DESCRIPTION	LIVE LOAD
1	2x12 @ 16" O.C.	
2	2x12 SLOPED ROOF RAFTER @ 16" O.C.	
3	2x8 @ 16" O.C.	
4	2x10 @ 16" O.C.	
5	PRE-ENGINEERED WOOD ROOF TRUSSES 24" O.C. BY TRUSS SUPPLIER	



UNIT A
SECOND FLOOR FRAMING PLAN

SCALE : 1/4" = 1'-0"

TYPICAL FLOOR SHEATHING IS 3/4" T&G SHEATHING. STAGGER JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW) GLUED AND SCREWED 6" O.C. AT EDGES AND 12" O.C. IN THE FIELD

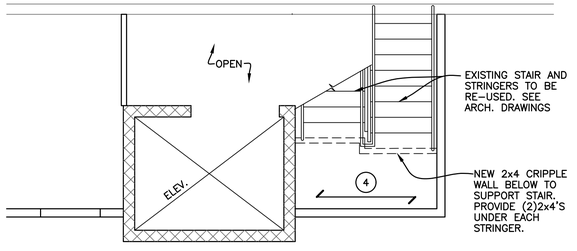
TYPICAL ROOF SHEATHING IS 5/8" PLYWOOD. PROVIDE ROOF CLIPS TYPICAL AT PANEL EDGES BETWEEN TRUSSES. STAGGER PLYWOOD JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW) ATTACH WITH 8d NAILS 6" O.C. AT EDGES AND 12" O.C. IN THE FIELD

B1: NEW DECORATIVE 8x8 TIMBER BEAM TO MATCH EXISTING. SEE ARCH. DRAWINGS

C1: HSS4x4x1/4 WITH 8"x1/2"x0'-8" BASE PLATE & (2)1/2"ø HILTI KWIK BOLT 3 EXPANSION ANCHORS TO TOP OF EXISTING FOUNDATION WALL.

STUD WALL NOTES

1. ALL WALL STUDS ARE 2x6 @ 16" O.C. TYPICAL (UNLESS NOTED OTHERWISE) HEM FIR NO. 2 OR SPF NO. 2 WITH THE FOLLOWING MINIMUM PROPERTIES Fc = 1,300 PSI, Fb = 850 PSI, E = 1,300,000



HEADER KEY: H1-2

- # OF CRIPPLE STUDS
- HEADER MARK SEE SHEET S4-01 FOR HEADER SCHEDULE

UNIT A
STAIR LANDING FRAMING PLAN

SCALE : 1/4" = 1'-0"

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KEY PLAN

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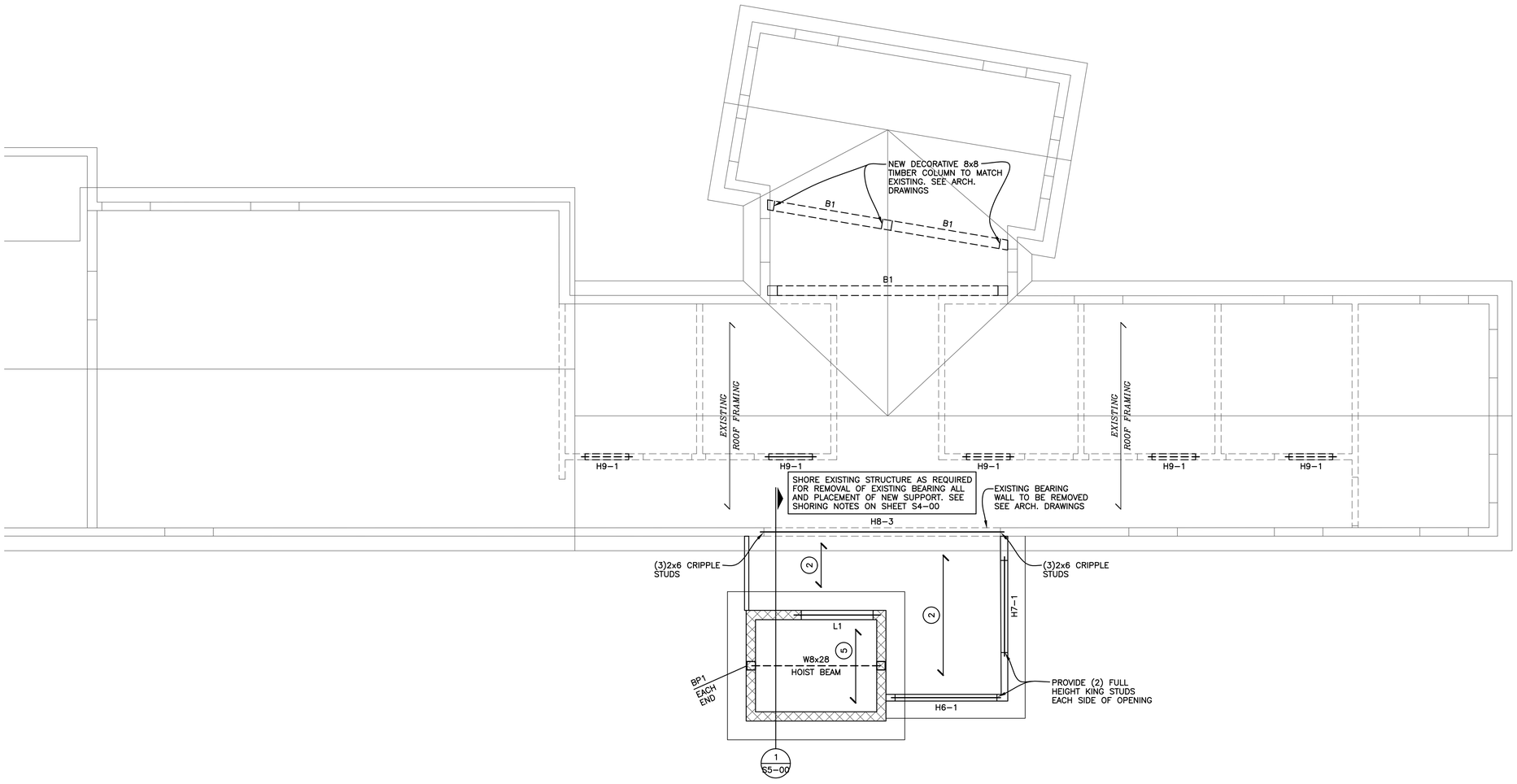
SHEET NAME

UNIT A
SECOND FLOOR
FRAMING PLAN

SHEET NO.

S3-03

MATERIAL SCHEDULE		
MARK	DESCRIPTION	LIVE LOAD
1	2x12 @ 16" O.C.	
2	2x12 SLOPED ROOF RAFTER @ 16" O.C.	
3	2x8 @ 16" O.C.	
4	2x10 @ 16" O.C.	
5	PRE-ENGINEERED WOOD ROOF TRUSSES 24" O.C. BY TRUSS SUPPLIER	



NORTH

UNIT A
ROOF FRAMING PLAN

SCALE : 1/4" = 1'-0"

TYPICAL ROOF SHEATHING IS 5/8" PLYWOOD. PROVIDE ROOF CLIPS TYPICAL AT PANEL EDGES BETWEEN TRUSSES. STAGGER PLYWOOD JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW) ATTACH WITH 8d NAILS 6" O.C. AT EDGES AND 12" O.C. IN THE FIELD

B1: NEW DECORATIVE 8x8 TIMBER BEAM TO MATCH EXISTING. SEE ARCH. DRAWINGS

STUD WALL NOTES

1. ALL WALL STUDS ARE 2x6 @ 16" O.C. TYPICAL (UNLESS NOTED OTHERWISE) HEM FIR NO. 2 OR SPF NO. 2 WITH THE FOLLOWING MINIMUM PROPERTIES Fc = 1,300 PSI, Fb = 850 PSI, E = 1,300,000

HEADER KEY: H1-2

- # OF CRIPPLE STUDS
- HEADER MARK SEE SHEET S4-01 FOR HEADER SCHEDULE

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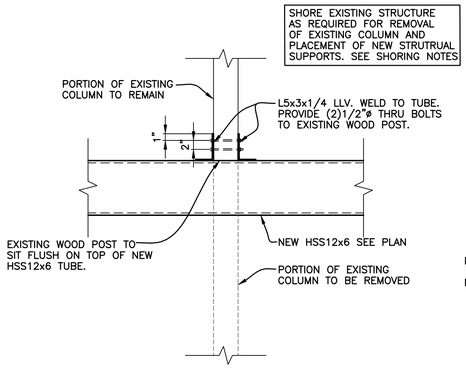
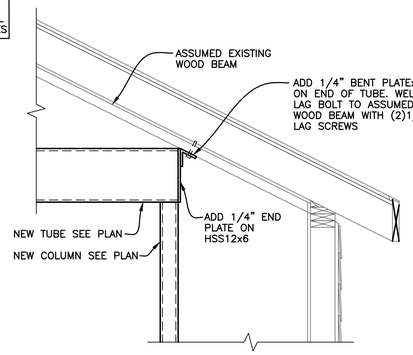
UNIT A
ROOF FRAMING
PLAN

SHEET NO.

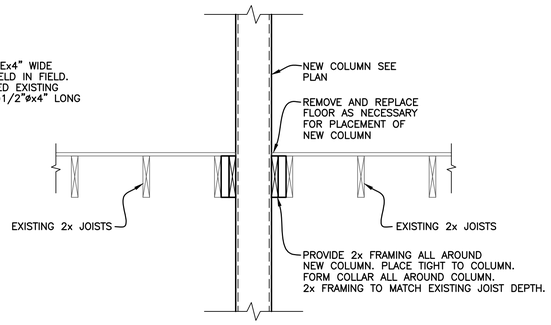
S3-04



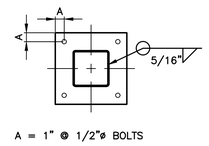
5 S3-05 SECTION
SCALE : 3/4" = 1'-0"



4 S3-05 SECTION
SCALE : 3/4" = 1'-0"

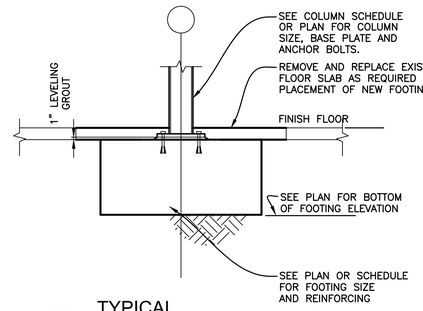


3 S3-05 SECTION
SCALE : 3/4" = 1'-0"

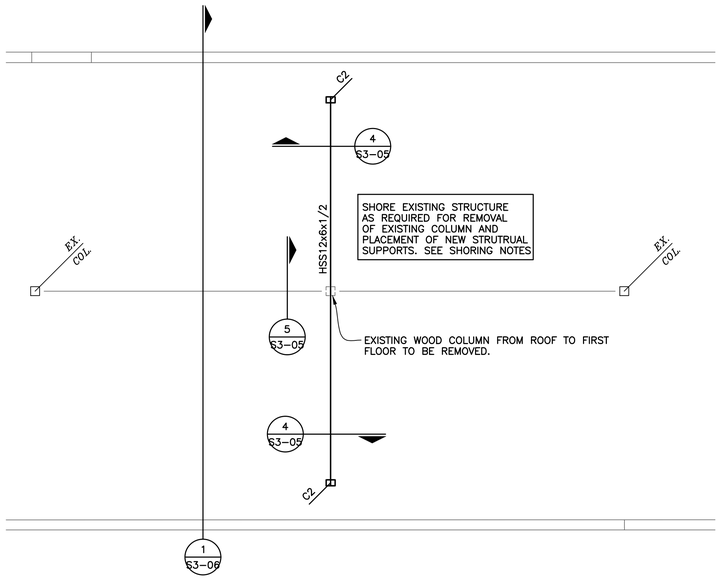


A = 1" @ 1/2"ø BOLTS

2 S3-05 TYPICAL BASE PLATES
SCALE : NONE



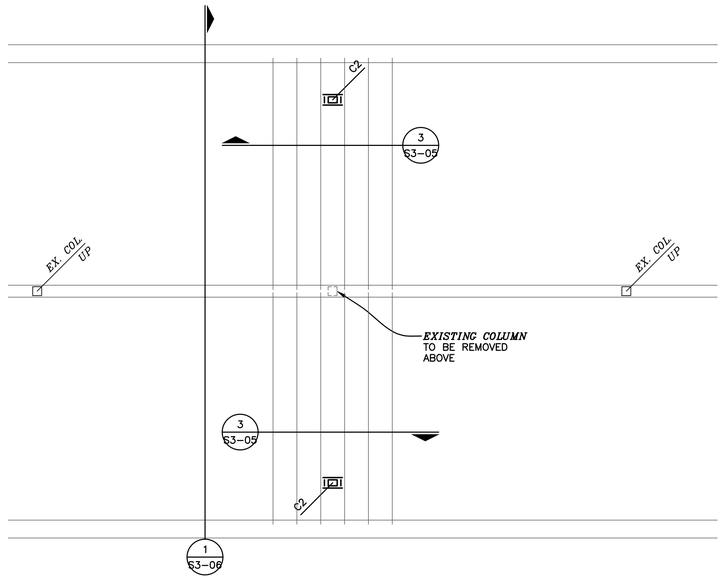
1 S3-05 TYPICAL INTERIOR COLUMN FOOTING
SCALE : 1/2" = 1'-0"



NORTH

SECOND FLOOR FRAMING PLAN
SCALE : 1/4" = 1'-0"

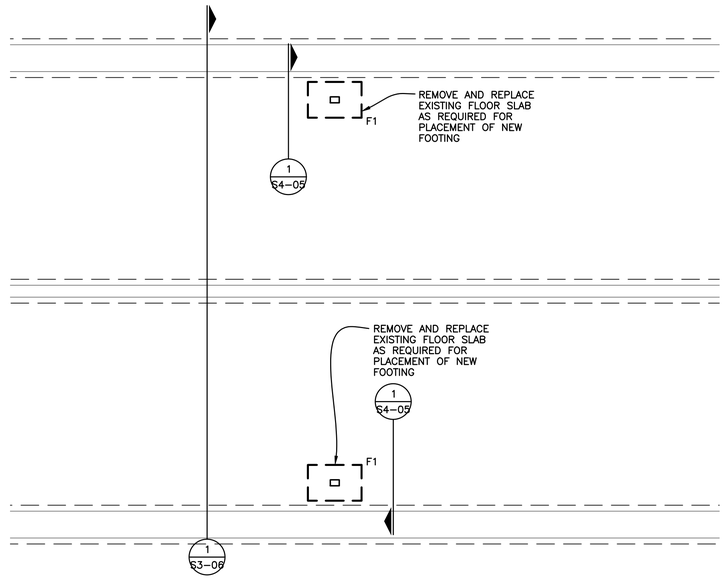
C2: HSS6x4x1/2" COLUMN WITH 10x3/4x0'-8" BASE PLATE WITH (4)1/2"ø x4 1/2" LONG EXPANSION ANCHORS



NORTH

FIRST FLOOR FRAMING PLAN
SCALE : 1/4" = 1'-0"

C2: HSS6x4x1/2" COLUMN WITH 10x3/4x0'-8" BASE PLATE WITH (4)1/2"ø x4 1/2" LONG EXPANSION ANCHORS



NORTH

FOUNDATION PLAN
SCALE : 1/4" = 1'-0"

F1: 3'-0"x2'-0" FOOTING WITH (4)#4 EACH WAY BOTTOM

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KEY PLAN

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OWNER
City of Dexter

PROJECT NAME
City Hall Renovations

3515 Broad St.
Dexter, MI 48130

PROJECT NO.
21-113

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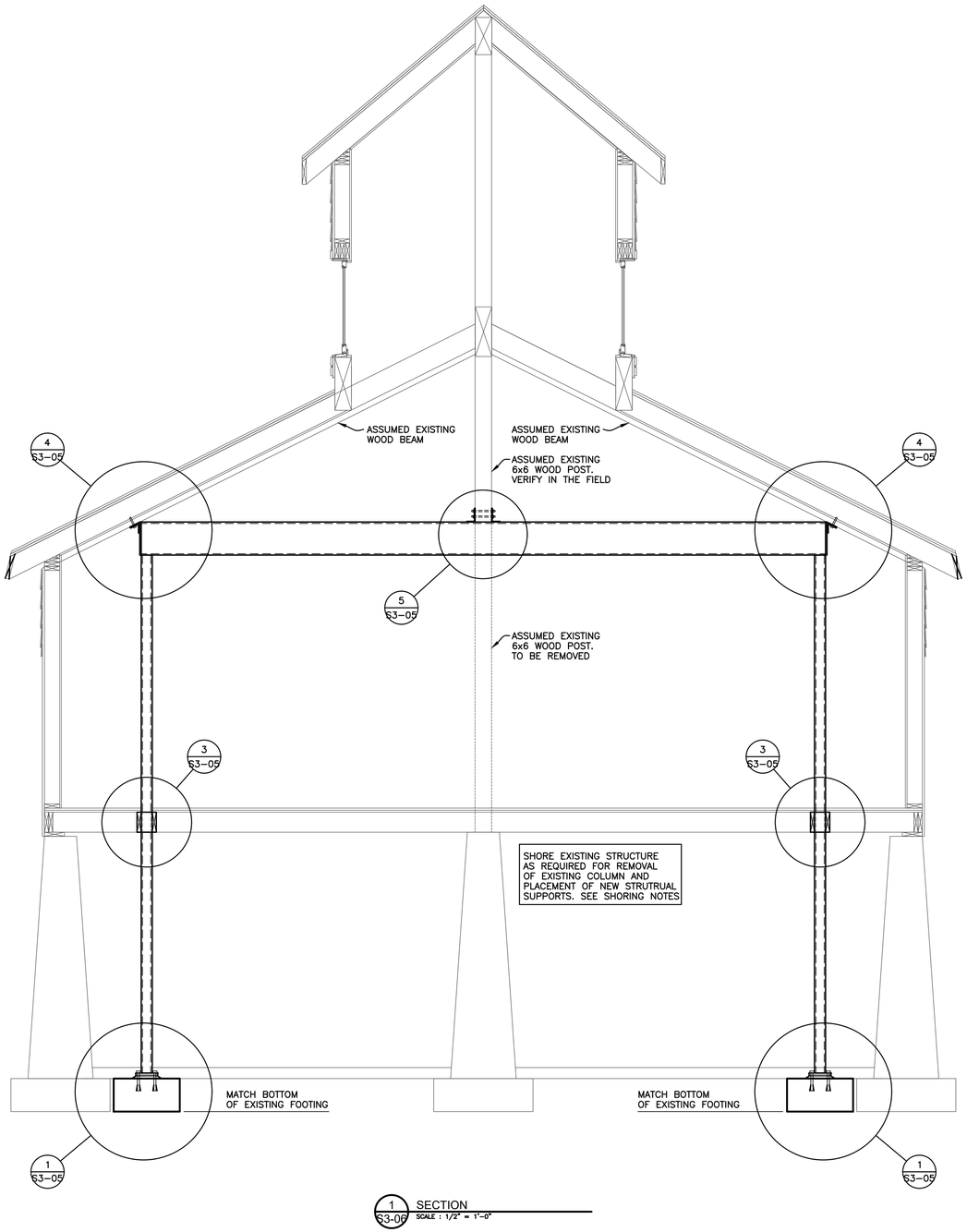
SHEET NAME
UNIT B
FOUNDATION, FIRST & SECOND FLOOR FRAMING PLAN

SHEET NO.
S3-05



1 S3-06 SECTION
SCALE : 1/2" = 1'-0"

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KEY PLAN

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PROJECT NO.
21-113

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DETAILS

SHEET NO.
S3-06

G E N E R A L N O T E S

GENERAL CONDITIONS

1. IF ANY GENERAL NOTE CONFLICTS WITH ANY DETAIL OR NOTE ON THE PLANS OR IN THE SPECIFICATIONS, THE STRICTEST PROVISION SHALL GOVERN.

2. THE STRUCTURAL DRAWINGS ARE FOR THE PLACEMENT AND SIZE OF STRUCTURAL COMPONENTS ONLY. O.S.H.A., LOCAL GOVERNMENT CODES AND SAFETY CODE REQUIREMENTS SHALL BE ADHERED TO BY THE CONTRACTOR.

3. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER IT IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES PROVIDING TEMPORARY BRACING, SHORING, GUYS OR TIE- DOWNS. THESE TEMPORARY SUPPORTS WILL REMAIN IN PLACE UNTIL ALL STRUCTURAL COMPONENTS ARE IN PLACE AND COMPLETED.

4. USE OF ENGINEERING DRAWINGS AS ERECTION DRAWINGS BY THE CONTRACTOR IS STRICTLY PROHIBITED. DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY AND SHOULD NOT BE USED FOR BUILDING LAYOUT AND LOCATION. SEE ARCHITECTURAL DRAWINGS AND SITE PLAN FOR THESE PURPOSES.

5. THE CONTRACTOR SHALL CHECK SHOP DRAWINGS PRIOR TO SUBMITTAL AND IS SOLELY RESPONSIBLE FOR ERRORS & OMISSION IN THE PREPARATION OF SHOP DRAWINGS TO CONFORM TO THE DESIGN DRAWINGS. SUBMIT NO MORE THAN ONE REPRODUCIBLE AND TWO PRINTS OF SHOP DRAWINGS FOR ENGINEER REVIEW. TWO COPIES WILL BE TO THE ARCHITECT.

6. DO NOT BACK-FILL AGAINST BASEMENT WALLS UNTIL FIRST FLOOR AND BASEMENT SLABS ARE IN PLACE OR WALLS ARE ADEQUATELY, LATERALLY BRACED.

7. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL RELEVANT DIMENSIONS AND ELEVATIONS FOR EQUIPMENT INSTALLATIONS AGAINST PURCHASED MANUFACTURER'S CERTIFIED EQUIPMENT DRAWINGS. DIMENSIONS THAT DEPEND UPON SPECIFIC EQUIPMENT SUCH AS ELEVATOR OPENINGS, MECHANICAL EQUIPMENT SUPPORTS, ETC. SHALL BE COORDINATED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. SUCH DIMENSIONS SHALL BE PROVIDED ON THE SHOP DRAWINGS BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER.

8. PRE-MANUFACTURED ITEMS SUCH AS CANOPIES, AWNINGS, SUNSHADES, ETC. SHALL BE DESIGNED BY SUPPLIER. SUPPLIER SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS BY A REGISTERED ENGINEER IN THE STATE OF MICHIGAN FOR RECORD TO ARCHITECT. SHOP DRAWINGS SHALL INDICATE ALL DESIGN LOADS AND INCLUDE ALL CONNECTIONS AND MATERIAL NECESSARY FOR INSTALLATION OF PRE-MANUFACTURED ITEMS.

EXISTING CONDITIONS

1. VERIFY ALL EXISTING ASSUMED DIMENSIONS AND CONDITIONS (I.E. EXISTING MATERIALS; FRAMING MEMBER SIZES AND LOCATIONS; METHODS OF CONSTRUCTION; ETC.) AT THE SITE PRIOR TO CONSTRUCTION AND FABRICATION. IF DISCREPANCIES ARE FOUND, NOTIFY ARCHITECT BEFORE PROCEEDING WITH WORK.

FOUNDATIONS

1. FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL WITH AN ASSUMED SAFE BEARING CAPACITY OF 2500 P.S.F. IF SOIL OF THIS CAPACITY IS NOT FOUND AT THE ELEVATIONS INDICATED, FOOTINGS SHALL BE ENLARGED OR LOWERED AT THE DIRECTION OF THE ARCHITECT. VERIFY FOUNDATION SOIL BEARING PRESSURE IN FIELD BY SOILS ENGINEER.

2. WHERE NEW FOOTINGS ABUT EXISTING FOUNDATIONS, CAREFULLY HAND EXCAVATE AND PLACE BOTTOM OF NEW FOOTING AT THE SAME ELEVATION AS THE EXISTING.

3. PROVIDE NECESSARY SHEETING SHORING BRACING, ETC. AS REQUIRED DURING EXCAVATIONS TO PROTECT SIDES OF EXCAVATIONS.

4. COMPLY FULLY WITH REQUIREMENTS OF OSHA AND OTHER REGULATORY AGENCIES FOR SAFETY PROVISIONS.

CONCRETE

1. MINIMUM CONCRETE STRENGTH TO BE 3000 P.S.I. @ 28 DAYS, U.O.N.; SLABS SHALL BE 3500 P.S.I. MIN. U.O.N. EXPOSED CONCRETE SHALL BE 4000 PSI WITH 6% + 1% ENTRAINED AIR U.O.N.

   A. PROVIDE 3000 P.S.I. 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.58 MAXIMUM (NON-AIR-ENTRAINED), 5.0 BAG CEMENT MIX FOR ALL FOUNDATION WORK UNLESS NOTED OTHERWISE.

   B. PROVIDE 3500 P.S.I. 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.53 MAXIMUM (NON-AIR-ENTRAINED), 5.5 BAG CEMENT MIX FOR ALL INTERIOR SLABS UNLESS NOTED OTHERWISE.

   C. PROVIDE 4000 P.S.I. 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.45 MAXIMUM (AIR-ENTRAINED), 6.0 BAG CEMENT MIX FOR ALL EXTERIOR CONCRETE UNLESS NOTED OTHERWISE.

2. FLYASH OR GROUND GRANULATED BLAST FURNACE SLAG MAY BE SUBSTITUTED UP TO 25% MAXIMUM OF MIX DESIGN CEMENT CONTENT IN NON-EXPOSED CONCRETE MIXES. DO NOT USE IN EXPOSED MIX DESIGNS.

3. ALL CONCRETE WORK AND PLACEMENT SHALL CONFORM TO THE LATEST RECOMMENDATIONS OF A.C.I.

4. ALL REINFORCING BARS, DOWELS AND TIES SHALL CONFORM TO A.S.T.M. A615 GRADE 60. REINFORCING STEEL SHALL BE CONTINUOUS AND SHALL HAVE MINIMUM 36 BAR DIAMETER LAP AND BE FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. - 315 LATEST EDITION.

5. REINFORCED CONCRETE WALLS AND WALL FOOTINGS SHALL HAVE CORNER BARS AT ALL INTERSECTIONS OF THE SAME SIZE AND SPACING AS THE MAIN HORIZONTAL REINFORCING. PROVIDE 2-#5 BARS EACH SIDE OF ALL OPENINGS AND 2-#5 X 4'-0" DIAGONAL BARS AT CORNERS OF OPENINGS.

6. WHERE SLAB REINFORCING RUNS PARALLEL TO A SUPPORTING BEAM, GIRDER SPANDREL, OR WALL, PROVIDE #4 @ 12" O.C. IN TOP OF SLAB AT RIGHT ANGLES TO MAIN REINFORCING. HOOK BARS AT EXTERIOR WALLS OR SPANDRELS.

7. ALL SLABS ON GROUND SHALL BE 4" THICK AND HAVE 6" X 6" W1.4 X W1.4 WELDED WIRE FABRIC IN THE TOP 1/3 OF THE SLAB, UNLESS OTHERWISE NOTED.

8. ALL WALLS SHALL HAVE #4 @ 12" O.C. BOTH WAYS, INSIDE FACE, AND #3 @ 12" O.C. BOTH WAYS, OUTSIDE FACE, EXCEPT AS NOTED, AND ALL HORIZONTAL WALL STEEL SHALL BEND 2'0" AROUND CORNERS. BEND VERTICAL WALL STEEL 2'0" INTO FLOOR SLAB.

9. FIELD AND SHOP TESTING OF CONCRETE WORK SHALL INCLUDE INSPECTION OF REINFORCING STEEL PLACEMENT, REBARS, NUMBER, LOCATION, AND LAP SPLICE LENGTH.

10. PROVIDE DOWELS INTO FOUNDATION TO MATCH SIZE AND SPACING OF VERTICAL REINFORCEMENT AT ALL COLUMNS AND WALLS, UNLESS OTHERWISE NOTED.

CONCRETE (CONT.)

11. UNLESS OTHERWISE SHOWN, PROVIDE THE FOLLOWING COVER FOR REINFORCING STEEL:

A. UNFORMED SURFACES IN CONTACT WITH EARTH	-3	IN.
B. UNFORMED SURFACES OVER MOISTURE BARRIERS	-2	IN.
C. FORMED SURFACES EXPOSED TO EARTH OR WEATHER OR WATER PROOFING/DAMP PROOFING		
#6 OR LARGER	-2	IN.
#5 OR SMALLER	-1 1/2	IN.
D. FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER		
SLABS AND WALLS	-3/4	IN.
COLUMNS	-1 1/2	IN.
BEAMS AND GIRDERS	-1 1/2	IN.

MASONRY

1. THE MASONRY PORTIONS OF THIS STRUCTURE ARE DESIGNED ACCORDING TO THE LATEST ALLOWABLE STRESS DESIGN PROVISIONS OF THE MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 602) INCLUDING SECTIONS 2106 AND 2107 OF CHAPTER 21 IN THE MICHIGAN BUILDING CODE. MASONRY COMPONENTS HAVE BEEN DESIGNED ACCORDING TO THE PROVISIONS FOR SEISMIC DESIGN CATEGORY B.

2. ALL STRUCTURAL MASONRY IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402/ACI 530/ASCE 5) AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602/ACI 530.1/ASCE 6) MASONRY SUBMITTALS ARE REQUIRED BY ACI 530.1/ASCE 6/TMS 602. SECTION 1.5 MASONRY TESTING AND INSPECTIONS ARE REQUIRED BY ACI 530.1/ASCE 6/TMS 602 SECTION 1.6, TABLE 5.

3. ALL STRUCTURAL MASONRY HAS BEEN ENGINEERED IN ACCORDANCE WITH CHAPTER 2 ALLOWABLE STRENGTH DESIGN. COMPRESSION STRENGTH SHALL BE DETERMINED ACCORDING TO THE UNIT STRENGTH METHOD FOR CONCRETE MASONRY MSJC SECTION 1.4. B.2.b.

4. ALL BLOCK SHALL CONFORM TO ASTM C90, TYPE I, WITH A MINIMUM UNIT NET AREA COMPRESSIVE STRENGTH OF 2800 PSI.

5. MASONRY COMPRESSIVE STRENGTH f'm = 2000 PSI MINIMUM.

6. MORTAR SHALL BE TYPE "S" (1800 PSI) CONFORMING TO ASTM C-270. USE MORTAR CEMENT WHERE EXTERIOR WALLS ARE UNREINFORCED.

7. PROVIDE HORIZONTAL WIRE TYPE REINFORCING WITH 9 GAUGE SIDE AND CROSS MEMBERS IN EVERY SECOND COURSE (16" O.C.), IN ALL MASONRY WALLS. WALLS WITH VERTICAL REINFORCING SHALL ONLY HAVE "LADDER" TYPE REINFORCING.

8. ALL REINFORCING BARS, DOWELS AND TIES SHALL CONFORM TO A.S.T.M. A615 GRADE 60. REINFORCING STEEL SHALL BE CONTINUOUS, FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. - 315 LATEST EDITION AND HAVE THE FOLLOWING MINIMUM LAP LENGTHS:

BAR SIZE	8" CMU	12" CMU
#3	18"	18"
#4	24"	24"
#5	30"	30"
#6	38"	36"
#7		42"
#8	PROVIDE MECH. SPLICE	

9. ALL MASONRY BEARING STEEL BEAMS AND LINTELS TO BEAR 8" MINIMUM ON 3 COURSES SOLID MASONRY, WITH 2-3/4" DIAMETER BOLTS EACH END, UNLESS OTHERWISE NOTED.

10. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.

11. MASONRY GROUT SHALL CONFORM TO ASTM C 476, WITH PEA GRAVEL AGGREGATE AND A MINIMUM STRENGTH OF 2000 PSI, BUT NOT LESS THAN SPECIFIED f'm.

12. UNLESS OTHERWISE NOTED, AT ALL MASONRY WALLS PROVIDE THE FOLLOWING LINTELS:

    8" WALLS

    (2) L4x3 1/2 x 5/16 LLV FOR OPENINGS UP TO 4'-0"
    (2) L5x3 1/2 x 5/16 LLV FOR OPENINGS UP TO 5'-4"
    W8x18 + 3/8" PLATE FOR OPENINGS UP TO 8'-0"
    W8x28 + 3/8" PLATE FOR OPENINGS UP TO 12'-4"

    12" WALLS:

    (3) L4x3- 1/2 x 5/16 LLV FOR OPENINGS UP TO 4'-0"
    (3) L5x3-1/2 x 5/16 LLV FOR OPENINGS UP TO 5'-4"
    W8x18 + 3/8" PLATE FOR OPENINGS UP TO 8'-0"
    W8x28 + 3/8" PLATE FOR OPENINGS UP TO 12'-4"

13. ALL DOUBLE ANGLE LINTELS SHALL BE WELDED BACK TO BACK WITH A MINIMUM 2 INCH STITCH WELD EVERY 8 INCHES.

14. UNLESS OTHERWISE NOTED, PROVIDE L5 X 3-1/2 X 5/16 L.L.V. LINTEL FOR EACH 4" OF MASONRY FOR SPANS UP TO 5'-4" MAX.

15. PROVIDE DOWELS INTO FOUNDATION TO MATCH SIZE AND SPACING OF VERTICAL REINFORCEMENT AT ALL COLUMNS AND WALLS, UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL

1. STEEL DESIGN, FABRICATION AND ERECTION TO BE IN ACCORDANCE WITH THE LATEST A.I.S.C. MANUAL AND SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS. ALL WIDE FLANGE BEAMS AND COLUMNS SHALL CONFORM TO THE LATEST ASTM. SERIAL DESIGNATION A992, GR50; ALL MISCELLANEOUS STEEL PLATES, BARS, ANGLES, ETC., SHALL CONFORM TO ASTM A36; STEEL TUBING TO BE ASTM A500, GRADE B; STEEL PIPE ASTM. A-53, GRADE B. ANCHOR BOLTS TO BE ASTM F1554 GRADE 36 KSI MINIMUM UNLESS OTHERWISE NOTED

2. UNLESS OTHERWISE NOTED OR SHOWN, ALL BEAM CONNECTIONS TO HSS 5 X 5 OR SMALLER COLUMN,5"Ø OR SMALLER COLUMN, OR ANY TUBE COLUMN REGARDLESS OF SIZE WITH A WALL THICKNESS LESS THAN 3/8" SHALL BE MADE WITH THRU PLATES WELDED TO BOTH WALLS OF COLUMN.

3. ALL WELDED CONNECTIONS SHALL BE IN ACCORDANCE WITH THE LATEST AWS CODE, E70XX ELECTRODES, WITH WELDING PERFORMED BY QUALIFIED WELDERS.

4. BOLTED CONNECTIONS SHALL BE MADE WITH A-325 OR A-490 BOLTS. ALL BOLTS ARE TO BE INSTALLED IN ACCORDANCE WITH THE LATEST SPECIFICATIONS FOR "STRUCTURAL JOINTS USING A.S.T.M. A-325 OR A-490 BOLTS." TYPICAL BOLTED CONNECTIONS ARE "BEARING TYPE" UNLESS NOTED OTHERWISE.

5. DESIGN CONNECTIONS FOR MINIMUM ONE-HALF THE TOTAL ALLOWABLE UNIFORM LOAD PER A.I.S.C. BEAM LOAD TABLES, UNLESS OTHERWISE NOTED. (MIN. 2 BOLTS EACH CONNECTION).

6. THE DESIGN, CONFIGURATION & ERECTION SAFETY OF ALL STRUCTURAL STEEL CONNECTIONS SHALL BE THE RESPONSIBILITY OF THE STRUCTURAL STEEL FABRICATOR. REVIEW AND ACCEPTANCE OF THE SHOP DRAWINGS BY THE ENGINEER SHALL CONSTITUTE APPROVAL OF THE LOAD CARRYING ADEQUACY ONLY.

7. TYPE OF CONSTRUCTION PER ASCE A2.2 IS TYPE 2 "SIMPLE FRAMING" UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL (CONT.)

8. TEMPORARY ERECTION SEATS SHALL BE PROVIDED AS RECOMMENDED ON PAGE 3-59 OF THE A.I.S.C. PUBLICATION "ENGINEERING FOR STEEL CONSTRUCTION".

9. ALL PROVISIONS OF THE RECOMMENDED CODE OF STANDARD PRACTICE FOR STEEL JOISTS AS ADOPTED BY THE STEEL JOIST INSTITUTE SHALL BE ADHERED TO.

10. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL ANGLES, PLATES, BARS, CLIPS, ETC., ATTACHED TO STRUCTURAL STEEL.

11. THE DESIGN OF THE STEEL FRAMED STAIRS AND RAILINGS SHALL BE THE RESPONSIBILITY OF THE STEEL FABRICATOR. PROVIDE COMPLETE ENGINEERED STAIR ASSEMBLIES, CONFORMING TO THE ARCHITECTURAL INTENT (SHOP DRAWINGS), AND CALCULATIONS UNDER THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF MICHIGAN INCLUDING METAL FRAMING, HANGERS, MASONRY BEARING PLATES, COLUMNS, RAILING ASSEMBLIES, AND OTHER COMPONENTS NECESSARY TO SUPPORT THE STAIRS AND LANDINGS INCLUDING ANCHORAGE TO THE SUPPORTING STRUCTURE.

12. THE CONTRACTOR SHALL FURNISH ALL ACCESSORIES INCLUDING CLOSURES, "Z" CLOSURES, COLUMN CLOSURES, SCREED ANGLES AND GIRDER FILLERS AS REQUIRED.

13. MASONRY AND BRICK LINTELS SHALL BE GALVANIZED G90 PER ASTM A123.

14. PROVIDE L4X4X1/4 SEATS AT COLUMN WEBS WHERE REQUIRED FOR SUPPORT OF ROOF AND FLOOR DECKS. PROVIDE ANGLE OUTRIGGER FROM EXTERIOR COLUMNS FOR SLAB AND ROOF EDGE PLATE SUPPORT.

15. ALL WIDE FLANGE LINTELS TO HAVE MINIMUM 7"x3/8"xO'-7" BEARING PLATE, ALL WIDE FLANGE FLOOR OR ROOF BEAMS TO HAVE MINIMUM 7"x3/8"xO'-7" BEARING PLATE UNLESS OTHERWISE NOTED

SHORING

1. SHORE STRUCTURE AS REQUIRED TO MAINTAIN STRUCTURAL INTEGRITY.

2. ALL SHORING, UNDERPINNING, ETC., SHALL BE PERFORMED BY EXPERIENCED CONTRACTORS.

3. SHORE, UNDERPIN, ETC., ALL QUESTIONABLE AREAS PRIOR TO REMOVAL OF ANY STRUCTURAL SUPPORT TO INSURE STRUCTURAL INTEGRITY.

4. MAINTAIN SHORING UNTIL NEW PERMANENT STRUCTURE IS IN PLACE AND SECURE TO MAINTAIN STRUCTURAL INTEGRITY.

5. REMOVE SHORING AFTER NEW WORK IS IN PLACE AND CONNECTED.

WOOD

1. WOOD CONSTRUCTION SHALL BE PER AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) STANDARDS AND SPECIFICATIONS, AND NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY NATIONAL FOREST PRODUCTS ASSOCIATION.

2. ALL LUMBER FRAMING MEMBERS ARE TO HAVE THE FOLLOWING MINIMUM BASE DESIGN VALUES IN ACCORDANCE WITH THE LATEST ISSUE OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION (NFPA):

   Fb = 850 P.S.I.
   fv = 135 P.S.I.
   E = 1,300,000 P.S.I.
   NO. 2 OR BETTER

3. PLYWOOD TO BE CONTINUOUS OVER TWO (2) OR MORE SPANS AND FACE GRAIN PERPENDICULAR TO SUPPORT.

   PLYWOOD OR ORIENTED STRAND BOARD FOR ROOF: 5/8" (32/16) 5 PLY STANDARD GRADE. PROVIDE ROOF CLIPS TYPICAL AT SHEATHING EDGES BETWEEN TRUSSES. STAGGER PLYWOOD JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW).

4. PLYWOOD OR ORIENTED STRAND BOARD FOR FLOORS: 3/4" TONGUE AND GROOVE, U.N.O. STAGGER PLYWOOD JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW).

5. HANGERS, STRAPS, CLIPS AND HOLDOWNS SHALL BE MANUFACTURED BY THE "SIMPSON MANUFACTURING COMPANY". ALL EXTERIOR CONNECTIONS ARE TO BE GALVANIZED.

6. PROVIDE DOUBLE CRIPPLE STUD AT EACH END OF WOOD HEADERS, TYPICAL, UNLESS NOTED OTHERWISE.

7. ALL STRUCTURAL LAMINATED VENEER LUMBER TO HAVE THE FOLLOWING MINIMUM DESIGN PROPERTIES BASED ON AITC GRADING STANDARDS:

   Fb = 2800 PSI
   Fv = 285 PSI
   Fc⊥ = 750 PSI
   E = 2,000,000 PSI

8. TIMBER SUPPLIER SHALL BE A MEMBER OF AITC. TIMBER CONNECTIONS TO BE DESIGNED AND DETAILED BY TIMBER SUPPLIER, WITH FABRICATION BY STEEL FABRICATOR.

9. UNLESS OTHERWISE NOTED, PROVIDE 2-2X8 HEADER FOR OPENINGS IN STUD WALLS FOR SPANS UP TO 3'-0 MAXIMUM.

10. ALL ROOF TRUSSES ARE A PERFORMANCE DESIGN BY THE WOOD SUPPLIER. TRUSS MANUFACTURER SHALL PROVIDE SHOP DRAWINGS FOR ALL TRUSSES AND COMPONENTS (FLOOR TRUSSES, ROOF TRUSSES, "I" JOISTS, HEADERS, ETC.) AND LAYOUT, WITH DESIGN LOADS, SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MICHIGAN. TRUSS LAYOUT DRAWINGS SHALL INDICATE ALL REQUIRED BRIDGING FOR TRUSS COMPONENT DESIGN. TRUSS MANUFACTURER SHALL VERIFY WITH ARCHITECT AND MECHANICAL CONTRACTOR SIZE, LOCATION & SUPPORT OF MECHANICAL UNITS. TRUSS FRAMING AND TRUSS TO TRUSS CONNECTIONS ARE TO BE DESIGNED BY TRUSS MANUFACTURER FOR ALL REQUIRED LOADS. SHOP DRAWINGS NOT SIGNED AND SEALED BY A REGISTERED ENGINEER IN THE STATE OF MICHIGAN WILL BE REJECTED. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR MECHANICAL LOADS AT TRUSSES. TRUSS SUPPLIER TO DESIGN TRUSSES FOR SUPPORT OF ALL MECH. UNITS, PIPING, FIRE SUPPRESSION LINES, AND ALL UTILITIES. COORDINATE ADDITIONAL LOADING AND PIPE SUPPORT/UTILITY LOCATIONS WITH MECH. CONTRACTOR.

11. DESIGN OF THE LUMBER AND CONNECTOR PLATES FOR TRUSSES SHALL BE IN ACCORDANCE WITH LATEST TRUSS PLATE INSTITUTE (TPI) REQUIREMENTS.

12. THE TRUSS SUPPLIER SHALL DESIGN AND DETAIL ALL REQUIRED BRIDGING, BRACING AND SUPPLEMENTAL MATERIAL TO PROVIDE A COMPLETE SYSTEM FOR THE LOADS AND PERFORMANCE REQUIREMENTS NOTED.

13. TRUSS TOP CHORD MUST BE BRACED WITH ROOF SHEATHING OR CONTINUOUS LATERAL BRACING AT 3 -0" O.C. BOTTOM CHORD MUST BE BRACED WITH A RIGID CEILING OR CONTINUOUS BRACING AT 10'-0" O.C. PLYWOOD SHEATHING SHALL BE NAILED OR SCREWED TO TRUSS MEMBERS AT 6" O.C. MAXIMUM SPACING.

14. ALL ROOF TRUSSES PARALLEL TO AND OVER SHEARWALLS SHALL BE DESIGNED TO HORIZONTALLY TRANSFER THE LATERAL LOADS TO THE SHEARWALL BELOW. SUPPLY HORIZONTAL LOAD ON ONE FACE OF TRUSS.

15. ALL FABRICATION SHOPS SHALL BE APPROVED BY THE BUILDING DEPARTMENT AND ENGINEER PRIOR TO ANY WORK BEING PERFORMED. SUBMIT ALL CERTIFICATIONS AND DOCUMENTATION FOR THEIR REVIEW. WOOD TRUSS DOCUMENTS SUBMITTED BY THE WOOD TRUSS SUPPLIER IS A "DEFERRED SUBMITTAL" PER SECTION 107.3.4.1 OF THE MBC 2015

WOOD (CONT.)

16. TRUSSES: SHALL BE MANUFACTURED BY AN ACCEPTABLE TRUSS MANUFACTURER, RECOGNIZED BY THE GOVERNING BUILDING CODE. TRUSS MANUFACTURER SHALL SUPPLY ALL HANGERS, PLATES, BLOCKS, CLIPS, BRIDGING AND OTHER ITEMS RELATIVE TO THEIR UNITS. DESIGN CRITERIA ARE AS FOLLOWS:

    ROOF TRUSS:

    TC LL = 25 PSF
       DL = 10 PSF
    BC DL = 10 PSF LL = 10 PSF (ATTIC LIVE LOAD PER SECTION TABLE 1607.1)

    TL = 45 PSF MINIMUM + 10 PSF ATTIC LIVE LOAD + DRIFTING SNOW INDICATED ON PLAN

    OPEN WEB FLOOR TRUSS AT APARTMENTS
    LL = 40 PSF
    DL = 32 PSF
    TL = 72 PSF

    PUBLIC ROOMS, CORRIDOR AND STAIRS
    LL = 100 PSF
    DL = 25 PSF
    TL = 125 PSF

    * TOTAL LOAD DEFLECTION NOT TO EXCEED SPAN/360 FOR FLOOR TRUSSES
    LIVE LOAD DEFLECTION NOT TO EXCEED SPAN/480 FOR FLOOR TRUSSES

17. TRUSSES SHALL BE INSTALLED PER THE LATEST TPI BCSI REQUIREMENTS.

18. POWDER-DRIVEN FASTENERS (P.D.F.) SHALL HAVE MINIMUM SHANK DIAMETER OF 3/16" AND A MINIMUM EMBEDMENT OF 1-1/4". SPACING IS AS NOTED ON THE PLANS. POWDER-DRIVEN FASTENERS SHALL BE AS MANUFACTURED BY "HILTI", "RAMSET", "REDHEAD" OR AN APPROVED EQUAL.

19. STACK ALL BEARING STUDS, CRIPPLE STUDS AND POSTS FROM FLOORS AND ROOF ABOVE DOWN TO FOUNDATION.

20. NOTCHED AND/OR CUT STUDS TO CLEAR ANCHOR BOLTS ARE NOT ALLOWED. STUDS SHALL HAVE FULL-BEARING TO THE FOUNDATION PLATE.

21. LET-IN BRACES SHALL NOT BE USED FOR TEMPORARY BRACING ON ANY WALL FRAME. STEEL STRAPS WHICH DO NOT REQUIRE CUTTING OF STUDS ARE ACCEPTABLE.

22. FASTEN WOOD MEMBERS IN ACCORDANCE WITH MBC TABLE 2304.10.1.

SPECIAL INSPECTION

1. WORK CONSTRUCTED SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY TO ENSURE COMPLIANCE WITH THE REQUIREMENTS SHOWN ON THE DRAWINGS. INSPECTIONS REQUIRED BY CHAPTER 17 OF THE MICHIGAN BUILDING CODE; LOCAL BUILDING DEPARTMENTS AND THE CONTRACT DOCUMENTS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY. SITE VISITS BY THE DESIGN ENGINEER DO NOT CONSTITUTE OR REPLACE INSPECTION

2. THE FOLLOWING ITEMS SHALL BE INSPECTED IN ACCORDANCE WITH MBC 2015 SEC. 1704 & 1705 BY A CERTIFIED SPECIAL INSPECTOR UNLESS NOTED OTHERWISE IN REMARKS COLUMN. ALL INSPECTION SHALL BE CONTINUOUS UNLESS OTHERWISE NOTED. ALL PRODUCTS WITH ICC APPROVALS SHALL BE INSTALLED PER THE APPROVAL AND PER MANUFACTURER'S RECOMMENDATIONS. FOR MATERIAL TESTING REQUIREMENTS, SEE SPECIFICATIONS AND/OR GENERAL NOTES. TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ARCHITECT.

   INSPECTION OF FABRICATOR'S (SEC. 1704.2.5) *

   FABRICATION AND IMPLEMENTATION PROCEDURES 1704.2.5.1

   *SPECIAL INSPECTION IS NOT REQUIRED FOR FABRICATOR SHOP IF CERTIFICATE OF APPROVAL SUBMITTED BY FABRICATOR'S INSPECTION AGENCY PER EXCEPTION 1704.2.5.1

TABLE 1705.2.2
REQUIRED VERIFICATION AND
INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	NOT APPLICABLE	REFERENCED STANDARD
1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK:				
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	-	APPLICABLE ASTM MATERIAL STANDARDS
b. MANUFACTURER'S CERTIFIED TEST REPORTS.	-	X	-	-
2. INSPECTION OF WELDING:				
a. COLD-FORMED STEEL DECK:				
1) FLOOR AND ROOF DECK WELDS.	-	X	-	AWS D1.3
b. REINFORCING STEEL:				
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	-	X	-	AWS D1.4 ACI 318: SECTION 3.5.2
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	X	-	-	
3) SHEAR REINFORCEMENT.	X	-	-	
4) OTHER REINFORCING STEEL.	-	X	-	

TABLE N5.4-1
INSPECTION TASKS PRIOR TO WELDING

INSPECTION TASKS PRIOR TO WELDING	QC	QA	NOT APPLICABLE
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	P	P	-
MANUFACTURER CERTIFICATION FOR WELDING CONSUMABLES AVAILABLE	P	P	-
MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O	-
WELDER IDENTIFICATION SYSTEM†	O	O	-
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) • JOINT PREPARATION • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)	O	O	-
CONFIGURATION AND FINISH OF ACCESS HOLES	O	O	-
FIT-UP OF FILLET WELDS • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION)	O	O	-
CHECK WELDING EQUIPMENT	O	-	-

†THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.

O - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

P - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER.

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KEY PLAN

OWNER
City of Dexter

PROJECT NAME
City Hall Renovations

3515 Broad St.
Dexter, MI 48130

PROJECT NO.
21-113

ISSUES / REVISIONS

PERMITS	12/22/21

DRAWN BY
RC

CHECKED BY
TS

APPROVED BY
TS

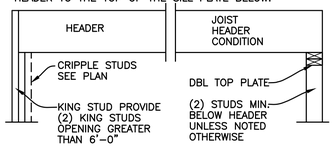
SHEET NAME
GENERAL NOTES

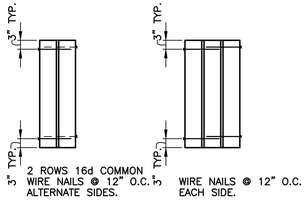
SHEET NO.
S4-00

HEADER SCHEDULE

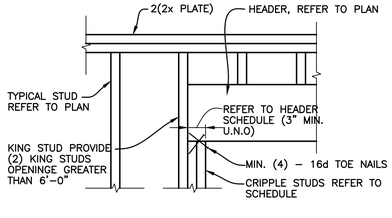
MARK	DESCRIPTION	CRIPPLE STUDS
H1	(2)2x8	ONE
H2	(3)1 3/4"x14" LVL	THREE
H3	(3)1 3/4"x11 7/8" LVL	TWO
H4	(2)1 3/4"x14" LVL	THREE
H5	(3)2x10	ONE
H6	(3)2x8	ONE
H7	(2)2x8	ONE
H8	(3)1 3/4"x16 7/8" LVL	THREE
H9	(2)2x8	ONE
H10	(2)1 3/4"x9 1/4" LVL	TWO
H11	(2)2x8	ONE

1. CRIPPLE STUDS SHALL BE CONTINUOUS FROM BOTTOM OF HEADER TO THE TOP OF THE SILL PLATE BELOW.

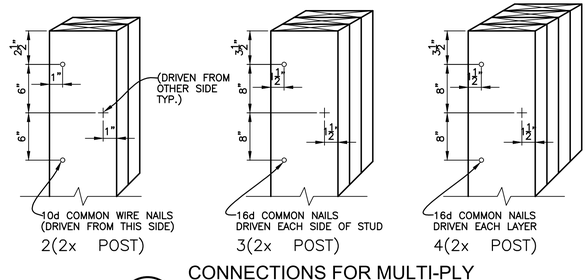




3 S4-01 CONNECTIONS FOR MULTI-PLY HEADERS
SCALE : 3/4" = 1'-0"



2 S4-01 TYP. HEADER BEARING DETAIL
SCALE : 3/4" = 1'-0"



1 S4-01 CONNECTIONS FOR MULTI-PLY 2x POST CRIPPLE STUDS
SCALE : 3/4" = 1'-0"

SPECIAL INSPECTION (CONT.)

TABLE N5.4-2
INSPECTION TASKS DURING WELDING

INSPECTION TASKS DURING TO WELDING	QC	QA	NOT APPLICABLE
USE OF QUALIFIED WELDERS	O	O	-
CONTROL AND HANDLING OF WELDING CONSUMABLES • PACKAGING • EXPOSURE CONTROL	O	O	-
NO WELDING OVER CRACKED TACK WELDS	O	O	-
ENVIRONMENTAL CONDITIONS • WIND SPEED WITHIN LIMITS • PRECIPITATION AND TEMPERATURE	O	O	-
WPS FOLLOWED • SETTINGS ON WELDING EQUIPMENT • TRAVEL SPEED • SELECTED WELDING MATERIALS • SHIELDING GAS TYPE/FLOW RATE • PREHEAT APPLIED • INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.) • PROPER POSITION (F, V, H, OH)	O	O	-
WELDING TECHNIQUES • INTERPASS AND FINAL CLEANING • EACH PASS WITHIN PROFILE LIMITATIONS • EACH PASS MEETS QUALITY REQUIREMENTS	O	O	-

TABLE N5.4-3
INSPECTION TASKS AFTER WELDING

INSPECTION TASKS AFTER WELDING	QC	QA	NOT APPLICABLE
WELDS CLEANED	O	O	-
SIZE, LENGTH AND LOCATION OF WELDS	P	P	-
WELDS MEET VISUAL ACCEPTANCE CRITERIA • CRACK PROHIBITION • WELD/BASE-METAL FUSION • CRATER CROSS SECTION • WELD PROFILES • WELD SIZE • UNDERCUT • POROSITY	P	P	-
ARC STRIKES	P	P	-
K-AREA[1]	P	P	-
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P	P	-
REPAIR ACTIVITIES	P	P	-
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	P	P	-

[1]WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OF STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75MM) OF THE WELD.

TABLE N5.6-1
INSPECTION TASKS PRIOR TO BOLTING

INSPECTION TASKS PRIOR TO BOLTING	QC	QA	NOT APPLICABLE
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	O	P	-
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	O	O	-
PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	O	O	-
PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	O	O	-
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	O	O	-
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	P	O	-
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTNER COMPONENTS	O	O	-

TABLE N5.6-2
INSPECTION TASKS DURING BOLTING

INSPECTION TASKS DURING BOLTING	QC	QA	NOT APPLICABLE
FASTENERS ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	O	O	-
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	O	O	-
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	O	O	-
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	O	O	-

TABLE N5.6-3
INSPECTION TASKS AFTER BOLTING

INSPECTION TASKS AFTER BOLTING	QC	QA	NOT APPLICABLE
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	O	O	-

O - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.

P - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER.

SPECIAL INSPECTION (CONT.)

REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION (LEVEL B QUALITY ASSURANCE)

MINIMUM TESTS
VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT
VERIFICATION OF f'_g AND f'_{AAC} IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE

MINIMUM INSPECTION						
INSPECTION TASK	FREQUENCY (a)			REFERENCE FOR CRITERIA		
	CONTINUOUS	PERIODIC	NOT APPLICABLE	IBC SECTION	TMS 402/ACI 530/ASCE 5	TMS 602/ACI 530.1/ASCE 6
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS		X				ART. 1.5
2. AS MASONRY CONCSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
a. PROPORTIONS OF SITE-PREPARED MORTAR.		X				ART. 2.1, 2.6A
b. CONSTRUCTION OF MORTAR JOINTS.		x				ART. 3.3B
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		X				ART. 2.4B, 2.4H
d. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		X				ART. 3.4, 3.6A
e. PRESTRESSING TECHNIQUE.		X				ART. 3.6B
f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	x(b)	x(c)				ART. 2.1C
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:						
a. GROUT SPACE		X				ART. 3.2D, 3.2F
b. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		X		SEC. 1.16		ART. 2.4, 3.4
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES		X		SEC. 1.16		ART. 3.2E, 3.4, 3.6A
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.						
e. CONSTRUCTION OF MORTAR JOINTS.		X				ART. 3.3B
4. VERIFY DURING CONSTRUCTION:						
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X				ART. 3.3F
b. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION		X			SEC. 1.16.4.3, 1.17.1	
c. WELDING OF REINFORCEMENT	X				SEC. 2.1.7.7.2, 3.3.3.4(c), 8.3.3.4(b),	
d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C)		X				ART. 1.8C, 1.8D
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X					ART. 3.6B
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X					ART. 3.5, 3.6C
g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	x(b)	x(c)				ART. 3.3 B.8
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X				ART. 1.4 B.2.a.3 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.

(a). FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE TASK LISTED OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE.

(b). REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF ACC MASONRY.

(c). REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF ACC MASONRY.

TABLE 1705.3
REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	NOT APPLICABLE	REFERENCED STANDARD [a]	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	-	ACI 318 CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" AND c. INSPECT ALL OTHER WELDS.	- - x	X X -	- - -	AWS D1.4 ACI 318: 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE	-	X	-	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.[b] a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENTION LOADS. b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	X -	- X	- -	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	-	ACI 318: CH.19. 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	-	ASTM C172 ASTM C31 ACI 318: 26.4,26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	-	ACI 318: 26.5	1908.6, 1908.7, 2908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	-	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS.	X X	- -	- -	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	-	ACI 318: CH. 26.8	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	-	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	-	ACI 318: 26.11.1.2(b)	-

FOR SI: 1 INCH = 25.4 MM

a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

TABLE 1705.6
REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	NOT APPLICABLE
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X	
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X	
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-	
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X	

SPECIAL INSPECTION (CONT.)

DESIGN CRITERIA		
CODE: MBC 2015 THE STRUCTURE IS DESIGNED FOR THE FOLLOWING LIVE LOADS, IN ADDITION TO THE LATERAL LOADS, SUPER-IMPOSED DEAD LOADS, & SELF WEIGHT OF THE STRUCTURE. WHERE APPLICABLE LIVE LOADS ARE REDUCED IN ACCORDANCE WITH THE PROVISIONS OF THE BUILDING CODE. A. AMERICAN CONCRETE INSTITUTE BUILDING CODE (ACI-318). B. MANUAL OF STEEL CONSTRUCTION BY AMERICAN INSTITUTE OF STEEL CONSTRUCTION (LATEST EDITION). C. LATEST MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402/ACI 530/ASCE 5) AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602/ACI 530.1/ASCE 6) D. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) STANDARDS AND SPECIFICATIONS. E. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY AMERICAN FOREST AND PAPER ASSOCIATION.		
		CODE REFERENCE
BUILDING OCCUPANCY CATEGORY	II	MBC-Table 1604.5 ASCE Table 1.5-1

FLOOR LIVE LOADS		
		CODE REFERENCE
STAIRS	100 PSF	ASCE Table 4-1
LOBBIES	100 PSF	ASCE Table 4-1

NOTE: HANDRAILS AND GUARDS TO BE DESIGNED TO RESIST A LINEAR LOAD OF 50 POUNDS PER LINEAR FOOT. PER SECTION 1607.8.1 OF THE MBC BUILDING CODE AND A CONCENTRATED LOAD OF 200 POUNDS CONCENTRATED LOAD PER SECTION 1607.8.1.1 OF THE MBC BUILDING CODE.

NOTE: GRAB BARS SHALL BE DESIGNED TO RESIST A SINGLE CONCENTRATED LOAD OF 250 POUNDS PER SECTION 1607.8.2 PER MBC BUILDING CODE

SNOW LOADS/ROOF LIVE LOADS		
SNOW CRITERIA		CODE REFERENCE
GROUND SNOW LOAD	Pg = 25 PSF	MBC FIG. 1608.2 ASCE Fig. 7-1
FLAT ROOF SNOW LOAD	Pf = 20 PSF (MINIMUM)	ASCE Sec. 7.3
EXPOSURE FACTOR	Ce = 1.0	ASCE Table 7-2
IMPORTANCE FACTOR	I = 1.0	ASCE Table 1.5-2
THERMAL FACTOR	Ct = 1.0	ASCE Table 7-3
ROOF LIVE LOADS	Lr = 20 PSF	ASCE Table 4-1
NOTE: SNOW LOADS ADJACENT VERTICAL PROJECTIONS, ON LOWER ROOFS, ADJACENT TO HIGH ROOFS, OR SLOPED ROOFS ARE INCREASED FOR THE EFFECT OR DRIFTING		

WIND LOADS		
WIND CRITERIA		CODE REFERENCE
BASIC WIND SPEED (3 SEC. GUST)	V = 115 MPH	ASCE FIG. 26.5-1A, 26.5-1B, 26.5-1C
RISK CATEGORY	II	ASCE Table 1.5-1
EXPOSURE CATEGORY	B	ASCE Sec. 26.7.3
INTERNAL PRESSURE COEFFICIENT	± 0.18 (ENCLOSED)	ASCE TABLE 26.11-1
MWFRS ANALYSIS PROCEDURE	DIRECTIONAL PROCEDURE	ASCE CHAP. 27
COMPONENTS AND CLADDING	± 33 PSF MINIMUM ULTIMATE AND PER CODE REQUIREMENTS BASED ON ABOVE INFORMATION	ASCE Sec. 30.2.2

SEISMIC LOADS		
SEISMIC CRITERIA		CODE REFERENCE
SEISMIC RISK CATEGORY	II	ASCE Table 1.5-1
SEISMIC IMPORTANCE FACTOR	I = 1.0	ASCE Table 1.5-2
0.2 SEC MAPPED SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING) Ss	Ss = .089	ASCE Sec. 11.4
1.0 SEC MAPPED SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING) S1	S_1 = .045	ASCE Sec. 11.4
SOIL SITE CLASS	D	ASCE Sec. 11.4.2
SEISMIC DESIGN CATEGORY	B	ASCE Sec. 11.6
SEISMIC FORCE RESISTING SYSTEM	STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC	ASCE Table 12.2-1
RESPONSE MODIFICATION FACTOR	R = 3.0	ASCE Table 12.2-1
DEFLECTION AMPLIFICATION FACTOR	Cd = 3.0	ASCE Table 12.2-1
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	ASCE Sec. 12.8

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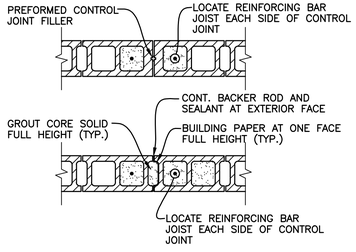
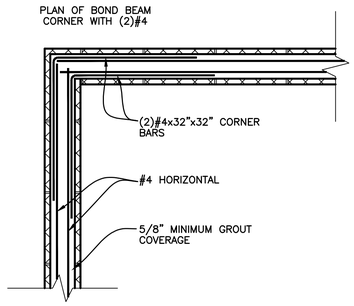
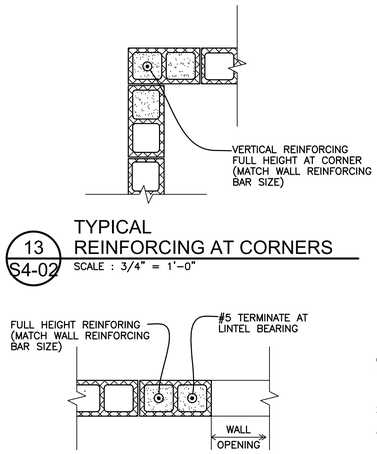


15 S4-02 TYPICAL MASONRY WALL CONTROL JOINTS
SCALE : 3/4" = 1'-0"

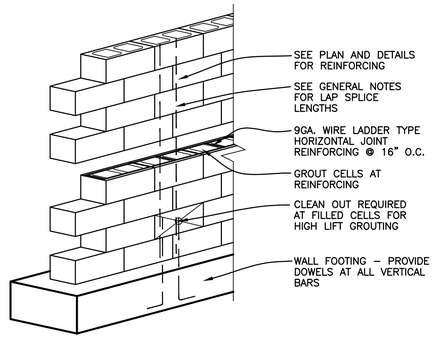


14 S4-02 TYPICAL BOND BEAM CORNER DETAIL
SCALE : 3/4" = 1'-0"



13 S4-02 TYPICAL REINFORCING AT CORNERS
SCALE : 3/4" = 1'-0"

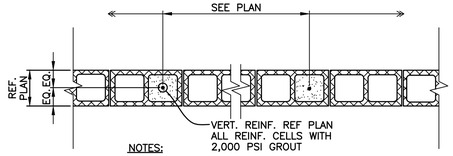
12 S4-02 TYPICAL REINFORCING ALONG SIDE OPENING
SCALE : 3/4" = 1'-0"



GROUT INSTRUCTIONS :

1. CONSTRUCT WALL TO HEIGHT OF 4'-0" – ALLOW MORTAR TO SET SUFFICIENTLY TO WITHSTAND GROUT PRESSURE.
2. INSPECT UNITS FOR ALIGNMENT, CLEAN OUT CELLS TO BE FILLED.
3. FILL CELLS TO 8" BELOW TOP COURSE WITH 2,000 PSI CONCRETE GROUT.
4. DELAY 3 – 5 MINUTES PRIOR TO CONSOLIDATING TO ALLOW WATER TO BE ABSORBED BY MASONRY.

11 S4-02 REINFORCED MASONRY CONSTRUCTION DETAIL
SCALE : NONE



NOTES:

1. PROVIDE CLEANOUTS AT BOTTOM OF EACH REINFORCED CELL IN ACCORDANCE WITH THE SPECIFICATIONS
2. PROVIDE HORIZONTAL JOINT REINF. PER GENERAL NOTES @ 16" O.C. MAX. U.N.O.

MASONRY REINF. LAP LENGTH		
BAR SIZE	8" WALL	12" WALL
4	24"	24"
5	30"	30"
6	38"	36"
7		42"

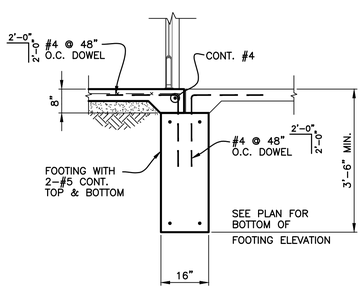
10 S4-02 TYPICAL WALL REINFORCING DETAIL
SCALE : 3/4" = 1'-0"

9 S4-02 SECTION
SCALE : 1/2" = 1'-0"

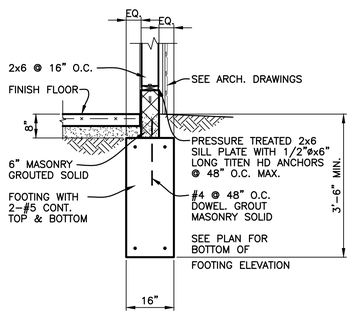
8 S4-02 SECTION
SCALE : 1/2" = 1'-0"

7 S4-02 SECTION
SCALE : 1/2" = 1'-0"

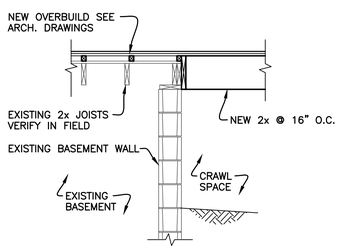
6 S4-02 TYPICAL MASS ENTRANCE SLAB
SCALE : 1/2" = 1'-0"



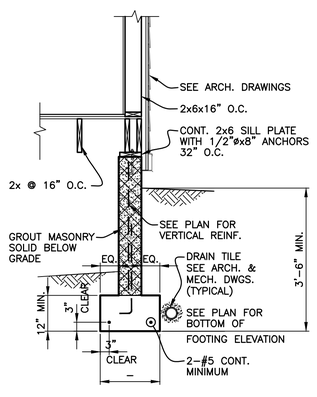
5 S4-02 SECTION
SCALE : 1/2" = 1'-0"



4 S4-02 TYPICAL EXTERIOR WALL FOOTING
SCALE : 1/2" = 1'-0"



3 S4-02 SECTION
SCALE : 1/2" = 1'-0"



2 S4-02 SECTION
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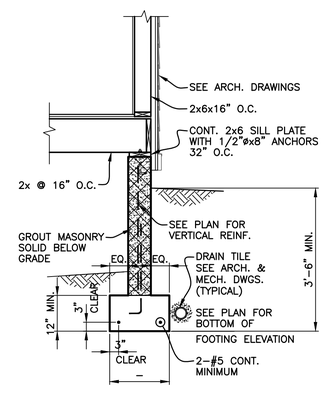

1 S4-02 SECTION
SCALE : 1/2" = 1'-0"

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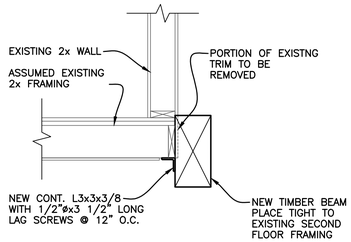
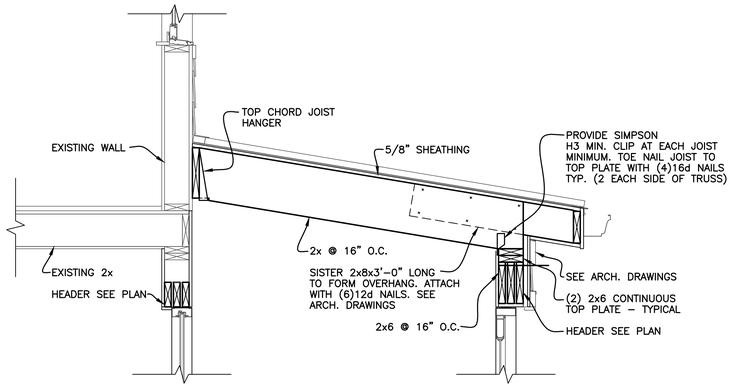
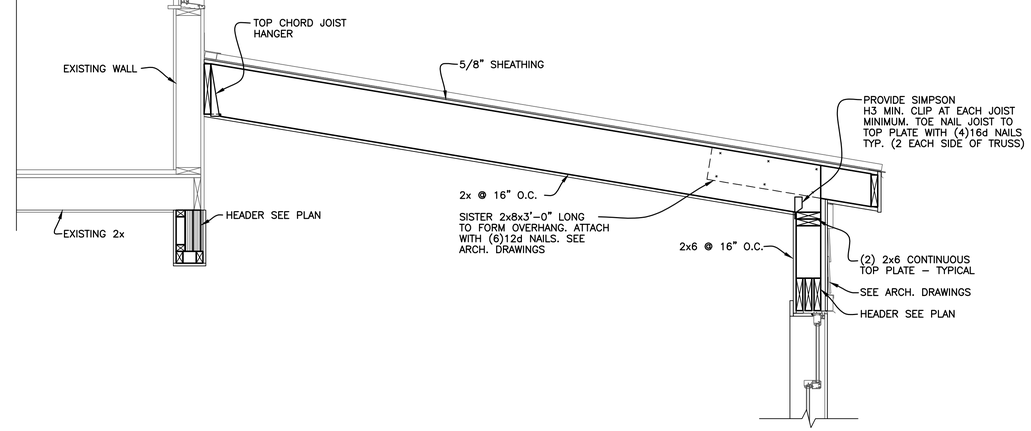


4 S5-00 SECTION
SCALE : 3/4" = 1'-0"





2 S5-00 SECTION
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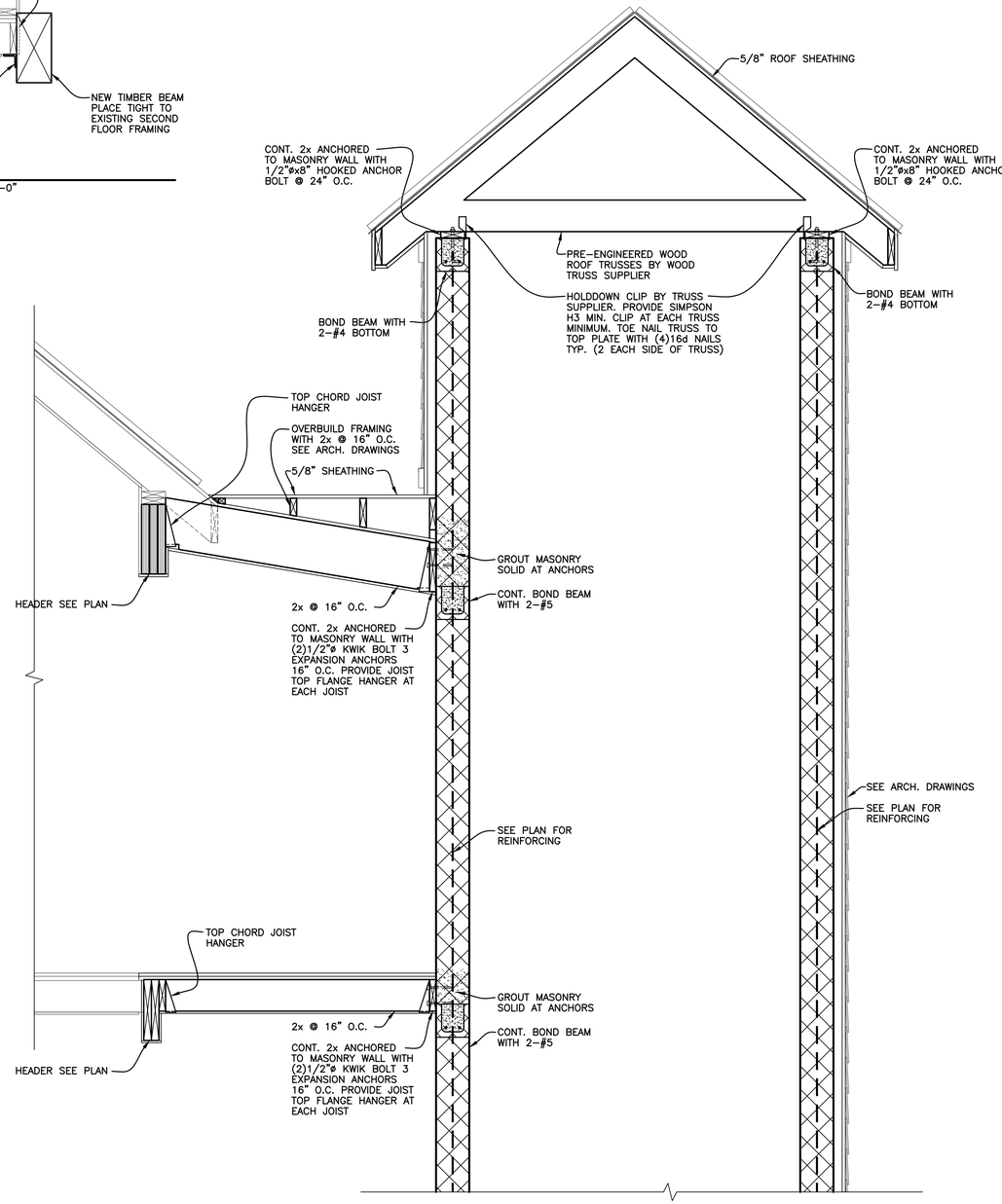

1 S5-00 SECTION
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