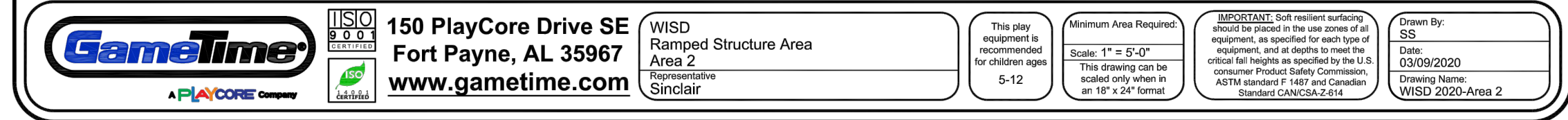
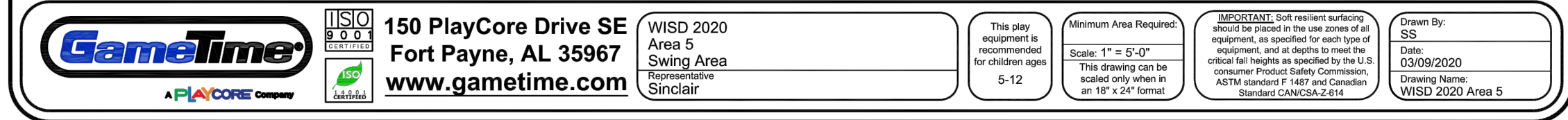


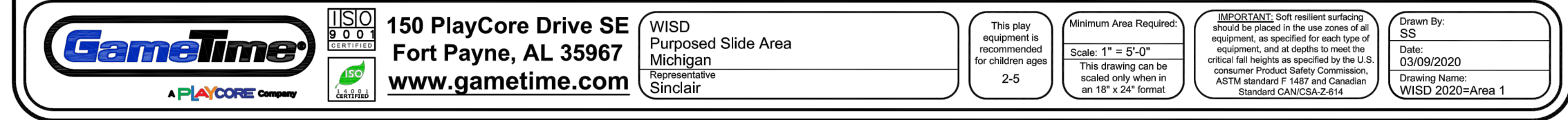
LOWER ELEMENTARY PLAYGROUND EQUIPMENT



UPPER ELEMENTARY PLAYGROUND EQUIPMENT



SWINGS AREA PLAYGROUND EQUIPMENT



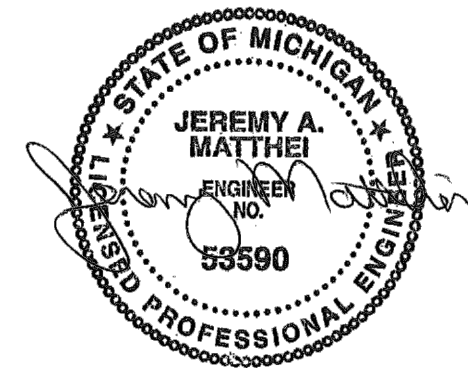
PRESCHOOL PLAYGROUND EQUIPMENT



ALTERNATE: HOUSE 5 PLAYGROUND EQUIPMENT



REGISTRATION SEAL



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MC PROJECT NO. 19147

PROJECT TITLE
New High Point
School
Washtenaw
Intermediate
School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

GAMETIME PLAYGROUND DETAILS

ISSUE DATES

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03-16-20	FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN RDW/JJR

CHECKED **JAN**

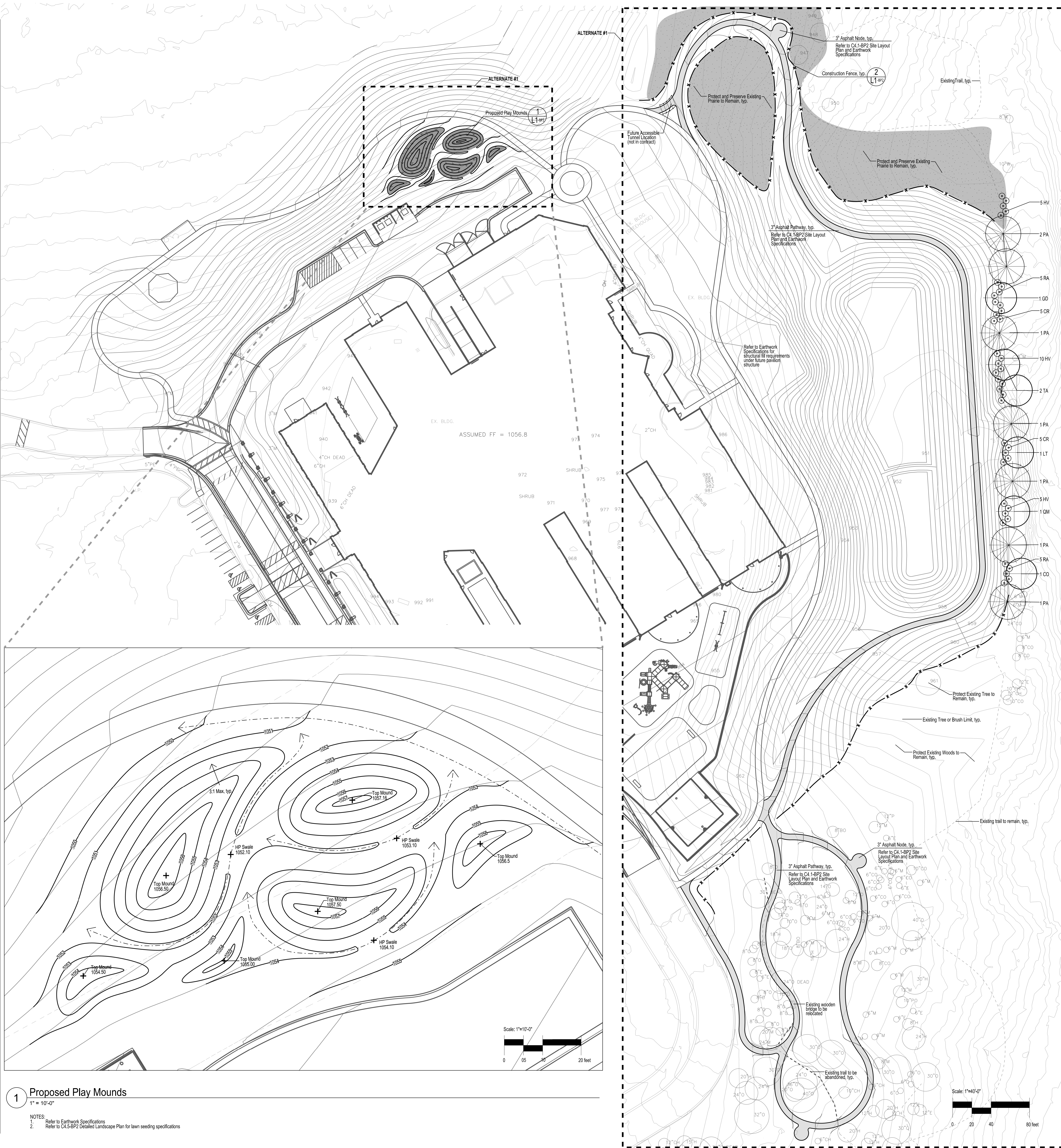
APPROVED RCV

PROJECT NO.

19040

DRAWING NO.

C10.1-BP2



LEGEND:

- Existing Property Line
- Existing Tree or Brush Limit
- Existing Tree to Remain
- Existing Prairie to Remain
- Existing Trail to Remain
- Existing Trail to be Abandoned
- Existing 1" Contour
- Proposed 1" Contour
- Proposed Construction Fence
- Proposed 3" Asphalt Pathway and Nodes
- Proposed Play Mounds
- Proposed Trees
- Proposed Shrubs
- Future Accessible Tunnel (not in contract)

Refer to Topographic and Boundary Survey, Sheet C1.1-BP2, for legend of existing site elements not indicated here.

ELEVATION

PLAN

NOTES:

- The fence shall be located a minimum of one (1) foot outside the drip line of the tree to be saved or closer only at the direction of City of Ann Arbor Forestry staff.
- Tree protection is to be erected prior to any earthwork or construction and is to remain in place until construction and grading is complete.
- All debris, fill, equipment, or material is to be kept clear of area within protective fence. No cleaning of equipment or storage or disposal of any material within the drip line of any trees to be saved.
- No activity including parking, driving of vehicles, pedestrian routing, staging lay down or storage of material will be allowed within the tree protective zones (to edge of drip line).

2 Construction fence

NO SCALE

Trees and Shrubs					
Key	Qty	Botanical Name	Common Name	Spacing	Size
CO	1	Celtis occidentalis	Hackberry	as shown	2" cal.
CR	10	Cornus racemosa	Gray Dogwood	60" o.c.	#5
GD	1	Gymnocladus dioica	Kentucky Coffeetree	as shown	2" cal.
HV	20	Hamelis virginiana	Witch-hazel	60" o.c.	#5
LT	1	Liriodendron tulipifera	Tulip Tree	as shown	2" cal.
PA	7	Picea abies	Norway Spruce	as shown	6-7' B&B
QM	1	Quercus macrocarpa	Bur Oak	as shown	2" cal.
RA	10	Rhus aromatica	Fragrant Sumac	as shown	#5
TA	2	Tilia americana	Basswood	as shown	2" cal.

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TMP ARCHITECTURE INC
1191 WEST SQUARE LAKE ROAD
BLOOMFIELD HILLS - MICHIGAN - 48302
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Mitchell and Mout architects
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REGISTRATION SEAL

SHANNAN GIBB-RANDALL
LANDSCAPE ARCHITECT
NO. 3901041515
STATE OF MICHIGAN
JULY 2010 - JANUARY 2011

CONSULTANT

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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

Natural Features Site + Landscape Plan

ISSUE DATES

03-12-20	ISSUED FOR CONSTRUCTION
02-26-20	95% BID PACK #2- CD REVIEW SET
01-14-20	BUILDING DEMOLITION - BID PACK #1
12-06-19	DESIGN DEVELOPMENT
08-13-19	SCHEMATIC DESIGN

DATE: ISSUED FOR:

DRAWN: SMN

CHECKED: SGR

APPROVED: SGR

PROJECT NO.

19040

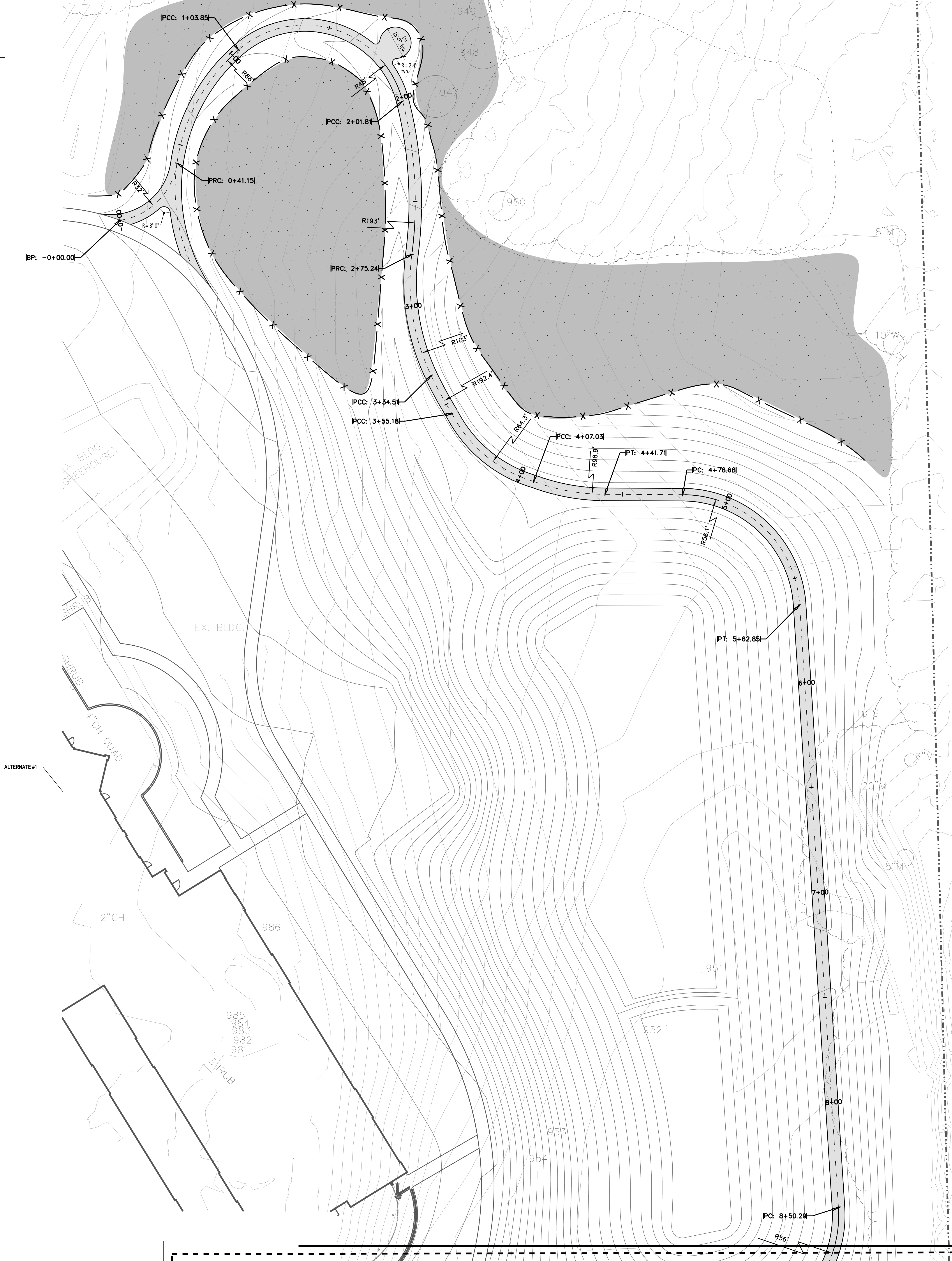
DRAWING NO.

L1-BP2

STATIONING NOTES

1. Any discrepancies between dimensioned layout and actual field conditions shall be immediately brought to the attention of the landscape architect. The contractor shall assume full responsibility for all necessary revisions due to failure to give such notification.

Abbreviation	Description
BP	Beginning Point
D	Diameter
EP	End Point
PCC	Point of Compound Curve
PRC	Point of Reverse Curve
PT	Point of Tangent
R	Radius



LEGEND:

- - - - - Existing Property Line
- - - - - Existing Tree or Brush Limit
- Existing Tree to Remain
- Existing Prairie to Remain
- - - - - Existing Path to Remain
- - - - - Existing 1" Contour
- - - - - Proposed 1" Contour
- x - x - Proposed Construction Fence
- Proposed 3" Asphalt Pathway and Nodes
- - - - - Centerline of Proposed 3" Asphalt Pathway

Refer to Topographic and Boundary Survey, Sheet C1.1-BP2, for legend of existing site elements not indicated here.



REGISTRATION SEAL



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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

Natural Features Layout Plan: North

ISSUE DATES

03-12-20	ISSUED FOR CONSTRUCTION
02-26-20	95% CONSTRUCTION DOCUMENTS
01-14-20	BUILDING DEMOLITION - BID PACK #1
12-06-19	DESIGN DEVELOPMENT
08-13-19	SCHEMATIC DESIGN

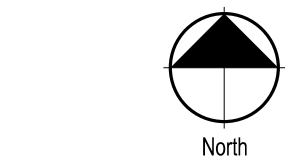
DATE:	ISSUED FOR:
DRAWN:	SMN
CHECKED:	SGR
APPROVED:	SGR

PROJECT NO.

19040

DRAWING NO.

L2-BP2

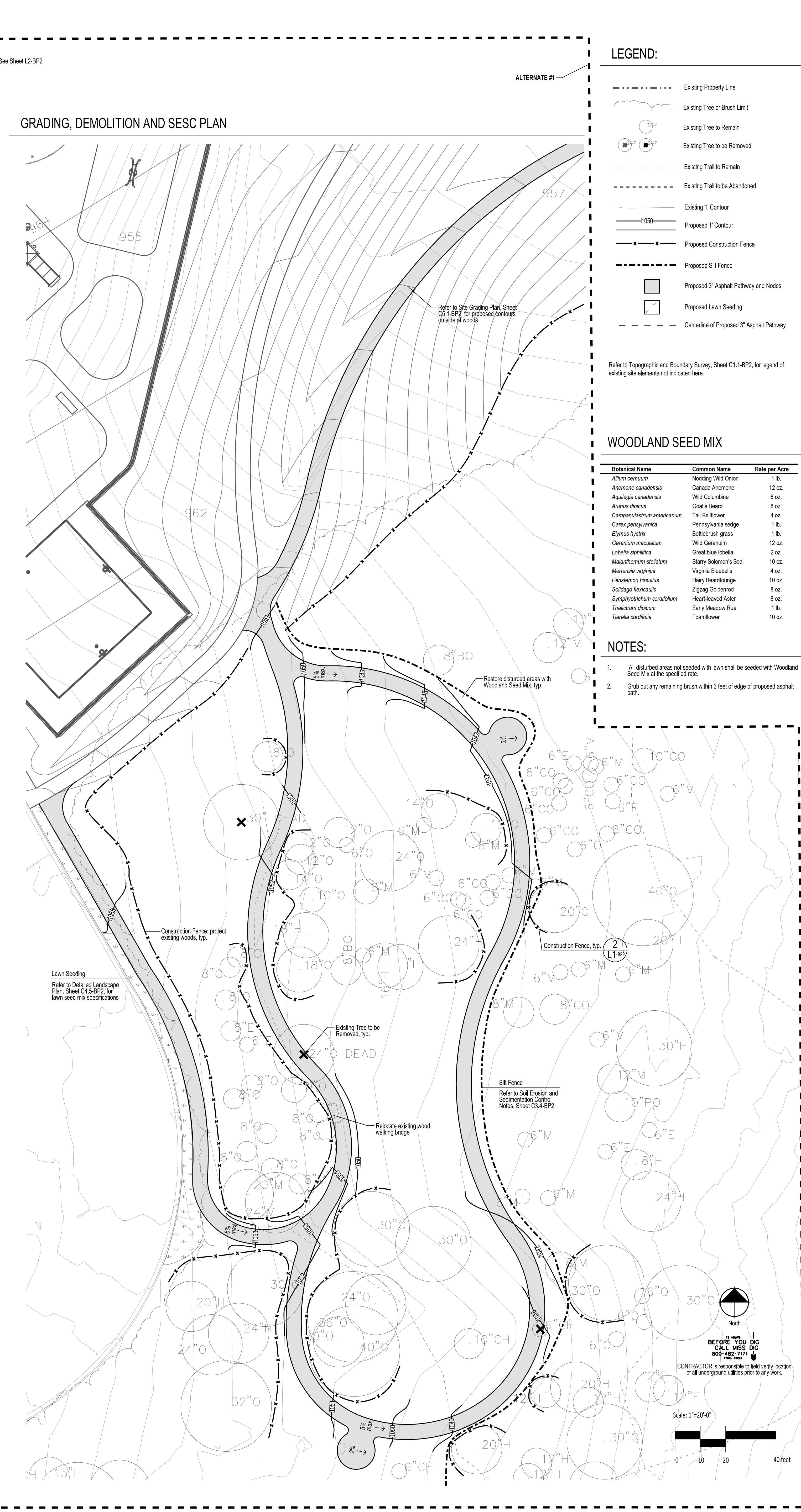
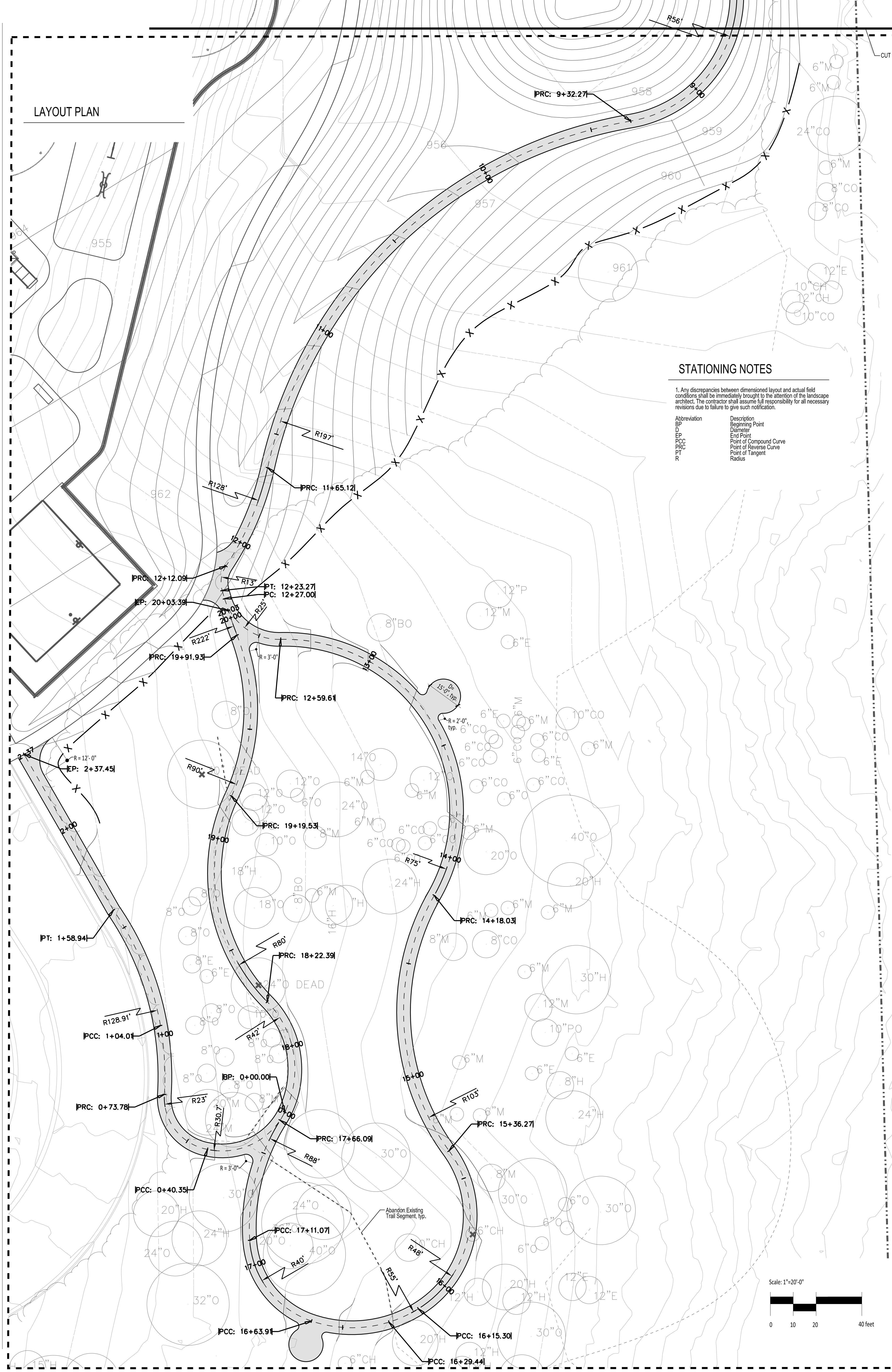


BEFORE YOU DIG
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CONTRACTOR is responsible to field verify location of all underground utilities prior to any work.

Scale: 1"=20'-0"





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PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
Natural Features Layout, Grading, and Demolition/ SESC Plan: South

ISSUE DATES

DATE	ISSUED FOR:
03-12-20	ISSUED FOR CONSTRUCTION
02-26-20	95% CONSTRUCTION DOCUMENTS
01-14-20	BUILDING DEMOLITION - BID PACK #1
12-06-19	DESIGN DEVELOPMENT
08-13-19	SCHEMATIC DESIGN

DRAWN: SMN
CHECKED: SGR
APPROVED: SGR

PROJECT NO.
19040

DRAWING NO.
L3-BP2

DESIGN CRITERIA				
The structure is designed for the following live loads, in addition to the lateral loads, superimposed dead loads, and self weight of the structure. Where applicable, the live loads are reduced in accordance with the provisions of the governing Building Code.				
Risk Category	III	CODE REFERENCE IBC Table 1604.5 ASCE Table 1.5-1		
FLOOR LIVE LOADS				
Slabs on Grade	100 PSF	CODE REFERENCE ASCE Table 4-1		
SNOW LOADS				
Ground Snow Load	P _g = 20 PSF	CODE REFERENCE ASCE Figure 7-1		
Flat Roof Snow Load	P _f = 22 PSF (minimum)	ASCE Section 7.3		
Exposure Factor	C _e = 1.0	ASCE Table 7-2		
Importance Factor	I = 1.1	ASCE Table 1.5-2		
Thermal Factor	C _t = 1.0	ASCE Table 7-3		
Snow loads adjacent to vertical projections, on lower roofs adjacent to high roofs, or sloped roofs are increased for the effects of drifting.				
WIND LOADS				
Ultimate Design Wind Speed (3 sec. gust)	V _{ult} = 120 MPH	CODE REFERENCE ASCE Figure 26.5-1B		
Nominal Design Wind Speed	V _{asd} = 93 MPH	IBC Section 1609.3.1		
Exposure Category	C	ASCE Section 26.7.3		
Internal Pressure Coefficient	± 0.18 (Enclosed)	ASCE Section 26.11-1		
Components and Cladding	Per Code Requirements Based on Above		ASCE Chapter 30	
ROOF COMPONENTS				
	Zone 1	Zone 2	Zone 3	CODE REFERENCE
Support Beams (A > 100 SF)	-31.1 PSF	-36.9 PSF	-36.9 PSF	ASCE Table 30-7.2
Roof Sheathing (A = 50 SF)	-32.6 PSF	-42.7 PSF	-51.3 PSF	ASCE Table 30-7.2
Deck Fasteners (A < 100 SF)	-34.0 PSF	-57.1 PSF	-85.9 PSF	ASCE Table 30-7.2
WALL COMPONENTS				
	Zone 4	Zone 5	CODE REFERENCE	
A = 100 SF	-31.1 PSF	-35.5 PSF	ASCE Table 30-7.2	
A = 50 SF	-32.6 PSF	-38.3 PSF	ASCE Table 30-7.2	
A = 10 SF	-36.9 PSF	-45.6 PSF	ASCE Table 30-7.2	
Refer to ASCE 7-10 for zone definitions. Calculate wind load for tributary area of design component. Wind loads shown are for use with ASCE 7-10 load combinations (i.e. 0.6 factor for ASD and 1.0 factor for LRFD), and include directionality factor K _d = 0.85, per ASCE Table 26.6-1.				
SEISMIC LOADS				
		CODE REFERENCE		
Seismic Importance Factor	I _e = 1.25	ASCE Table 1.5-2		
Short Period Mapped Spectral Response Acceleration Parameter (5% of Critical Damping)	S _S = 0.094g	ASCE Section 11.4.1		
1.0 sec. Mapped Spectral Response Acceleration Parameter (5% of Critical Damping)	S ₁ = 0.048 g	ASCE Section 11.4.1		
Soil Site Class	D	ASCE Section 11.4.2		
Design Spectral Response Acceleration Parameter (for short period)	SDS = 0.10 g	ASCE Section 11.4.4		
Design Spectral Response Acceleration Parameter (1 sec. period)	SD1 = 0.08 g	ASCE Section 11.4.4		
Seismic Design Category	B	ASCE Section 11.6		
Seismic Force Resisting System	Intermediate Reinforced Masonry Shear Walls	ASCE Table 12.2-1		
Design Base Shear	234 Kips	ASCE Section 12.8.1		
Seismic Response Coefficient	C _s = 0.031	ASCE Section 12.8.1.1		
Response Modification Factor	R = 4.0	ASCE Table 12.2-1		
Analysis Procedure	Equivalent Lateral Force	ASCE Section 12.8		
EARTH PRESSURE LOADS				
LATERAL EARTH EQUIVALENT FLUID PRESSURE				
Walls Unbraced at Top	40 PCF			
Walls Braced at Top	55 PCF			
Allowable Soil Bearing Capacity	3000 PSF			
1. Refer to Geotechnical Report for additional information. 2. Lateral earth pressure is based upon drained soil. Refer to drawings for foundation drainage.				
MECHANICAL / ELECTRICAL LOADS				
Typical Roofs	5 PSF			
Roof with Solar Panels	10 PSF			

GENERAL STRUCTURAL NOTES

- The structural notes are intended to augment the drawings and specifications. Should conflicts exist between the Drawings, Specifications and the Structural notes, the strictest provision shall govern.
- The Structural Drawings form an integral part of Contract Documents, which include Architectural, Structural, Mechanical, Electrical, Civil/SE drawings and Specifications. Coordinate the Structural drawings with the requirements shown in the other components of the Contract Documents.
- Typical details and other sections/details apply to conditions that are similar to the conditions described in the sections/details, even if they are not specifically referenced on the plans.
- The Contractor shall be responsible for means, methods, sequences and procedures of construction.
- The structure is designed to be self-supporting and stable after it is fully completed per requirements of Contract Documents. Contractor shall determine erection procedures and sequence, and ensure the safety of the building and its component parts during erection. This includes the addition of temporary bracing, guys or tie-downs if necessary. Contractor shall retain ownership of such material after completion of the project.
- Construction shall comply fully with the applicable provisions of OSHA and the local Governing Codes, current edition, and all requirements specified in the codes shall be adhered to as if they were called for or shown on the drawings. This shall not be construed to mean that requirements set forth on the drawing may be modified because they are more stringent than the code requirements or because they are not specifically required by code.
- Governing Building Code – Michigan (International) Building Code 2015.** Standards listed in structural note sections refer to the version and effective date identified in the REFERENCED STANDARDS Chapter in the Governing Building Code.
- Work constructed per these drawings shall be inspected by an Independent Testing Agency retained to ensure compliance with the requirements shown on the Drawings. Special Inspections required by the Governing Building Code, local building department and the Contract Documents shall be performed by a qualified Special Inspector. Project site visits by the Engineer do not constitute or replace inspection.

SHOP DRAWINGS

- Submit shop drawings for review as indicated in material section of general Structural notes.
- Use of Engineering Drawings as erection drawings by the Contractor is strictly prohibited.
- Allow in the schedule - detailing, fabrication and erection - a minimum of 10 working days for review of each shop drawing submittal by the Structural Engineer. Submit shop drawings in reasonable quantities at reasonable intervals (not more than 70 drawings per submittal per week). The 10 working days stated herein, will be in addition to the review time required by other project team members. Submit a shop drawing submittal schedule prior to the first submittal.
- Review of shop drawings and other submittals by the Structural Engineer does not relieve the Contractor of the responsibility to check the shop drawings prior to submittal. Errors and omissions associated with the preparation of shop drawings not conforming to the Construction Documents are the responsibility of the shop drawing preparer.
- Shop drawings are an aid for field placement and are superseded by the Contract Documents. Contractor shall ensure that construction is in accordance with the latest Contract Documents. Shop drawing review is only for general compliance with the Contract Documents. Review of the shop drawings by the Structural Engineer does not guarantee that the shop drawings are correct nor infer that the shop drawings supersede the Contract Documents.
- Contractor shall provide a set of approved shop drawings bearing the review stamp of the Structural Engineer to the state building department and to the project site.
- Notes on submitted shop drawings for work "by others" cannot be personally approved by Structural Engineer. Contractor shall coordinate responsibility for materials, connections, etc. prior to shop drawing submittal to the Structural Engineer.
- Contractor shall verify all relevant dimensions and elevations for equipment installations against purchased Manufacturer's certified equipment drawings. Contractor shall coordinate dimensions that depend upon specific equipment, such as elevator openings, mechanical equipment supports, etc., prior to submittal. Such dimensions shall be provided on the shop drawings prior to submittal to the Structural Engineer. Contractor's failure to provide such dimensions on submitted shop drawings will result in shop drawing return without review.

MECHANICAL & ELECTRICAL EQUIPMENT

- Pipes of 3 inches or greater in diameter shall be suspended from steel members, using pipe hangers and clamps. Hangers and clamps shall be aligned with structural member centerlines. One sided beam connections, expansion anchors or other anchors to slabs are not allowed for pipes of 3 inches or greater in diameter. Submit details for pipe hangers and attachments for review by Architect/Structural Engineer.
- Mechanical and electrical equipment weights assumed for structural design are shown on the plans. If the equipment weight varies from that listed, consult with the Architect/Structural Engineer prior to steel shop drawing submittal.

SHORING AND BRACING

- Contractor shall provide temporary shoring and bracing of existing construction, new construction, and underground utilities as follows:
 - Where shown or noted on the Drawings.
 - Where existing construction is to be altered or disturbed until permanent support is in place.
 - Where existing construction is not undergoing alteration and is to remain undisturbed but is disturbed as a result of the work of this contract.
 - As required for safe erection, installation of new construction, equipment, etc.
 - When needed for Contractor's "means and methods" of construction and other safety related issues.
- Shoring and bracing shown on the Drawings is conceptual. Contractor shall be responsible for verifying existing conditions, shoring and bracing calculations, methods of installation, transfer of loads through to final load support, and work sequence phasing with new construction.
- Shoring and bracing shall be performed by a Contractor with minimum 5 years demonstrated experience in similar size and scope of shoring and bracing projects.
- Shoring and bracing shall be designed by a Professional Engineer registered in the State of the Project with minimum 5 years demonstrated experience in similar size and scope of shoring and bracing projects. Design loads and methods shall conform to applicable codes. Soil and material strengths shall be verified by tests, unless conservative estimates that do not affect deflections and deformations are approved by the Architect/Structural Engineer.
- Contractor shall submit drawings and calculations sealed and signed by the Contractor's Professional Engineer showing complete design including temporary conditions, final conditions and sequence of work.
- Before starting work, Contractor shall perform condition survey of the existing building structure, exterior façade and interior finishes, including photographic documentation and submit survey to the Owner for record.
- During the shoring and bracing operations, Contractor shall:
 - Keep the existing and new construction in a safe condition.
 - Monitor existing and new construction to detect any signs of distress or deformation.
 - Take immediate steps to prevent distress, deformation or damage.
- Contractor shall continuously monitor the shoring and bracing system. Contractor shall review and ascertain that all field connections are completed according to the Contractor's design and issue approval for inspection of the work by the Testing Agency.
- After completion of shoring and bracing and completion of work requiring shoring and bracing, Contractor shall repair any damage to the existing and new construction, without any cost to the Owner, and to the satisfaction of the Owner and Architect/Structural Engineer.

EXISTING CONSTRUCTION

- Contractor shall visit the site and become familiar with the existing conditions.
- Existing building dimensions and conditions shown are based upon original drawings or partial survey and have not been completely field verified. The Owner and Architect/Structural Engineer take no responsibility for the accuracy of existing dimensions shown. Contractor shall field measure existing dimensions prior to shop drawing preparation and fabrication.
- The analysis of the existing structure is based upon information shown on original drawings by Guido A. Binda & Assoc. dated Sept. 1972, provided by the Owner.
- Contractor shall verify conditions covering or affecting the structural work; obtain and verify all dimensions and elevations to ensure the proper strength, fit and location of the structural work; report to the Architect/Structural Engineer any and all conditions/discrepancies which may interfere with or otherwise affect or prevent the proper execution and completion of the new work in compliance with the Construction Documents. All discrepancies shall be fully resolved prior to commencing work.
- Existing construction not undergoing alteration is to remain undisturbed. Where such construction is disturbed as a result of the operations of this contract, Contractor shall repair or replace as required and to the satisfaction of the Architect/Structural Engineer and Owner's Representative.
- Contractor shall verify the existence, location and elevation of existing utilities, sewers, drains, etc. in demolition areas before proceeding with the work. All discrepancies shall be documented and reported to the Architect/Structural Engineer and Owner's Representative for resolution.
- Should uncharted piping or other utilities be encountered during excavation, Contractor shall consult the Architect/Structural Engineer and Owner's Representative for resolution.
- Contractor shall provide fire watch during field cutting and welding operations, meeting the Owner's requirements.
- Contractor shall provide temporary protection of existing equipment during execution of work, satisfying the Owner's requirements.
- Contractor shall provide temporary protection to prevent damage from the weather and vandalism.
- Contractor shall coordinate work with the Owner's personnel to avoid any interference in their operations.
- Refer to SHORING AND BRACING notes for additional requirements.

FOOTINGS AND FOUNDATIONS

- Contractor shall verify all conditions, including underground utilities and field measurements at job site and report any discrepancies to Owner's Representative.
- Provide necessary sheeting, shoring, bracing, etc. as required during excavations to protect sides of excavations.
- Comply fully with requirements of OSHA and other regulatory agencies for safety provisions.
- Top of spread footing elevations noted on plan are minimum elevations. In all cases, footings are to bear on undisturbed natural soils or engineered fill having a minimum net allowable bearing capacity of 3,000 psf.
- Earth formed footings are acceptable where existing soil conditions allow neat, vertical cuts. Follow the Geotechnical Engineer's recommendations for specific preparation and procedures. Where soil is granular and/or does not hold a vertical cut, sides of foundations shall be formed. All concrete surfaces shall be maintained smooth and vertical. Slope sides of excavations as approved by the Geotechnical Engineer, and clean up sloping before and during concrete placement.
- Where footing steps are necessary, they shall be no steeper than one vertical to two horizontal unless noted otherwise.
- Footings shall be centered under columns and walls unless specifically detailed otherwise on the Drawings.
- No footings or slabs shall be placed on or against sub-grade containing free water, frost or ice. Should water or frost, however slight, enter a footing excavation after sub-grade approval, the sub-grade shall be re-inspected by the Geotechnical Engineer/Testing Laboratory after removal of water or frost.
- The Contractor shall provide all necessary measures to prevent any frost or ice from penetrating any footing or slab sub-grade before and after placing of concrete until the full building enclosure is completed and heated.
- Excavated material shall be legally disposed of off the Owner's property or stored at the site or used for backfilling operations as required in accordance with the Geotechnical Engineer's recommendations and Project Specification requirements.
- Contractor shall furnish all required de-watering equipment to maintain a dry excavation until backfill is complete.
- Where new footings are adjacent or abut existing foundations, carefully hand excavate and determine bottom of existing foundation. If different than anticipated, adjust new foundations to match existing. In no case shall the new footing be lower than the existing without protection against undermining such as underpinning or shoring.
- Foundation bearing soils shall be inspected by a qualified Geotechnical Engineer. The testing shall include, but not be limited to, identification of soils at and below the foundation bearing level, and the allowable bearing capacity of these soils.

- A Geotechnical Engineer registered in the State of the Project shall inspect the condition and assure the adequacy of all subgrades, fills, backfills before placement of foundations, footings, slabs and walls. They shall submit reports to the Architect/Engineer describing their investigations, including any non-conforming work.
- The design of foundations, retaining walls, and slab on grade is based on the criteria established in the Geotechnical Report No. 193506 by G2 Consulting Group, dated Jan. 23, 2020. Refer to the report for additional considerations related to ground water conditions and control, drainage, site preparations, earthwork operations and work adjacent to the existing building.

SITE PREPARATION

- Refer to Geotechnical Report No. 163506 dated Jan. 23, 2020 for consideration related to site preparation and earthwork operations. The requirements and recommendations contained in the report are part of Contract requirements.

BACKFILLING

- Do not place backfill against foundation walls - designed as supported at top and bottom - until basement level and first floor slabs are in place. Shore and/or brace walls as required if backfilling operations are to be carried out prior to placement of floor slabs.
- Place backfill against basement retaining walls - designed as cantilevered - after concrete has attained design strength and before lower level and first floor slabs are in place.
- Where backfill is to be placed on both sides of foundation walls, provide a balanced backfill against foundation walls to eliminate lateral load effects, or provide necessary temporary lateral support to the top of the wall until permanent support is installed.
- Backfill material shall consist of clean, well grade granular soils, free of organic material, silt and clay, or as specified in the Project Specifications.
- Backfill material shall be compacted to 95% of maximum density, as determined by the Modified Proctor Method (ASTM D1557), in lifts not exceeding 6 inches.

CAST-IN-PLACE CONCRETE

- Concrete structural framing has been designed by the Ultimate Strength Method per ACI 318 "Building Code Requirements for Structural Concrete".
- Concrete work shall conform to the requirements of ACI 301, "Specifications for Structural Concrete of Buildings", and ACI 318 "Building Code Requirements for Structural Concrete" except as modified by Structural requirements noted on the Drawings.
- All concrete work shall conform to ACI 201.2R, "Guide to Durable Concrete". Parking structures shall also conform to ACI 362.1R, "Guide to Durable Concrete for Parking Structures".
- Cement shall conform to ASTM C150 "Specification for Portland Cement" type I or III.
- Concrete aggregates shall conform to ASTM C33 "Specification for Concrete Aggregates"; and ASTM C330 "Specification for Light Weight Aggregates for Structural Concrete".
- Reinforcing shall conform to ASTM A615 grade 60, unless noted otherwise.
- Reinforcement shall be fabricated and erected according to the ACI standards: "Details and Detailing of Concrete Reinforcement", ACI 315 - and "Manual of Engineering and Placing Drawings for Reinforced Concrete Structures", ACI 315R.
- Welded wire fabric shall be furnished in flat sheets (rolls not permitted) and shall conform to ASTM A-185 and have a minimum side and end lap of 8 inches.
- Welding of reinforcing steel is prohibited unless specifically detailed. Welding where detailed shall conform to AWS D14 specification.
- A copy of ACI SP-15 "Field Reference Manual", ACI 301 "Specifications for Structural Concrete" with a selected ACI and ASTM references, shall be kept in the Contractor's field office.
- Concrete shall have a minimum 28-day compressive strength and properties as follows:

	f' _c	Max. W/C	Air
Foundations:	4,000 psi	0.50	-
Slab-on-grade:	4,000 psi	0.46	-
Exterior Concrete:	5,000 psi	0.40	6%

- Exterior concrete, and interior concrete subjected to freeze/thaw cycles, salt, etc., including walls, shall be air-entrained 6% +/- 1%.
- Concrete shall be normal weight, unless indicated otherwise. Light weight concrete, if specified for supported slabs shall be sand light-weight with a concrete unit weight not exceeding 120 pcf.
- Contractor shall submit the concrete mix designs for review by the Structural Engineer. Proportion mix designs and provide proof of mix design strength as defined in ACI 301. The submittal shall include cement type and source, cement cube strength, aggregate gradations, water tests, admixture catalog information and cylinder strength test results from 30 tests, on specimens with identical mix design, for each concrete mix, or other proof of strength per ACI 301.
- The approved materials and mix design shall be fully documented and reviewed by the Testing Agency for full compliance. Responsibility for obtaining the required design strength is the Contractor's responsibility.
- Use of calcium chloride, chloride ions, or other salts in concrete is not permitted.
- Samples for strength tests of each class of concrete placed each day shall be taken by the Testing Agency in accordance with Project Specification requirements or ACI 301.
- Contractor shall prepare and submit reinforcement shop drawings to the Structural Engineer for review prior to fabrication. The shop drawings shall clearly show reinforcement lengths and bends, locations of bars, methods of support, details of placement and placement coordination with formwork, embedments, concrete vibration and construction joints. The drawings shall also indicate openings, sleeves, cuts and concrete dimensions in accordance with ACI 315. Provide, at minimum wall, column and beam elevations; wall, column and beam sections, material schedules, bar lap schedules and locations.
- Contractor shall tie reinforcing steel securely in place prior to placing concrete and provide sufficient supports to maintain the position of reinforcing within specified tolerances during all construction activities. Inserting dowels into wet concrete is not permitted.
- Contractor shall provide continuous reinforcement wherever possible; splice only as shown or approved; stagger splices where possible; use splice lengths as noted. Dowels shall match the size and spacing of the specified reinforcement and shall be lapped with tension splices, unless noted otherwise.
- Horizontal wall reinforcement shall be continuous with the minimum lap per ACI 318 unless detailed or noted otherwise. Corner bars shall be provided at changes in wall direction (however small) and shall be of the same size and spacing as the horizontal steel. Each corner bar leg to provide lap splice per ACI 318 unless detailed or noted otherwise. Extend horizontal wall reinforcing through piers.
- Hooked bars shall be standard 90 degree hooks per ACI unless noted otherwise on the Drawings.
- Minimum lap splice shall be Class B per ACI 318. Location of lap splices shall be as indicated on Construction Documents and/or as shown on the approved reinforcing steel shop drawings.
- Approved rebar couplers may be used to aid placement of dowels through forms. Mechanical splices shall develop 125% of the tensile strength of the rebar.
- Reinforcing steel shall not be cut, bent or straightened in the field unless approved by the Structural Engineer or as indicated on the Drawings.
- Reinforcing steel shall be placed with the following concrete cover unless noted otherwise:
 - Concrete cast against earth (not formed): 3"
 - Formed concrete exposed to earth or weather:
 - #5 bars or smaller: 1 1/2"
 - #6 thru #16 bars: 2"
 - Formed concrete not exposed to earth or weather:
 - Slabs, joists, and walls, #11 bars or smaller: 3/4"
 - Slabs, joists, and walls, #14 bars or larger: 1 1/2"
 - Beams, columns, pedestals, and tension ties: 1 1/2"
 - Clearance between parallel bars in a horizontal layer shall not be less than the bar diameter, 1", or 4/3 d, whichever is greater.
 - Clearance between parallel bars in two or more horizontal layers, shall not be less than 1" between layers.
 - Clearance between longitudinal bars in columns, pedestals, struts, and boundary elements in walls shall not be less than 1.5 times the bar diameter, 1 1/2", or 4/3 d, whichever is greater.
 - Maximum deviation from these requirements shall be:
 - + 3/8" for sections with dimensions of 8" or less
 - + 1/2" for sections with dimensions over 8"
- Tie embeds securely in place prior to placing concrete.
- Do not place pipes or ducts exceeding one quarter the slab or wall thickness within the slab or wall unless specifically shown and detailed on the Structural Drawings. Pipes or duct shall be located within middle third of slab or wall thickness.
- Install inserts and anchors in concrete for suspending mechanical, electrical and architectural items. If additional fasteners are needed in conventionally reinforced concrete, use drilled-in type anchors located to avoid conflict with reinforcement.
- Anchor rods and steel embeds (furnished by Structural Steel Contractor) shall be set by template to within a 1/8" tolerance in any direction with minimum embedment and exact projection indicated on the drawings, prior to placing concrete.
- No aluminum conduit or products containing aluminum or any other material injurious to the concrete shall be embedded in the concrete. Dowels into foundation shall match size and spacing of vertical reinforcement at all columns, piers and walls, unless otherwise noted.
- Contractor shall coordinate all masonry dowel sizes and spacing to be cast into concrete with masonry reinforcing shop drawings.
- Provide two #5 bars (one each face) around unfamed openings in slabs and walls. Place bars parallel to sides of openings and extend them 24 inches beyond corners, unless otherwise noted.
- Locate sleeves, openings, embeds, etc. as indicated on the drawings. The Concrete Contractor shall check with other trades to make sure the sleeves, openings and embeds that are to be provided and set by them are in place prior to placing of concrete in the area involved.

- Contractor shall obtain approval prior to placing openings or sleeves not shown on the Drawings, through any structural member.
- Contractor shall review Architectural, Mechanical and Electrical drawings for bases, openings, sleeves, anchors, inserts, conduits, recesses and other devices in concrete work before placing concrete.
- Horizontal construction joints are permitted only where indicated. The location of vertical construction joints shall be submitted to the Structural Engineer for review and approval. Construction joints shall be thoroughly mechanically roughened, cleaned and bonding agent applied before placement of adjoining concrete.
- For control joints in slabs, space joints at maximum 15'-0" on center unless otherwise noted on the Drawings. For control joints in walls, space joints at maximum 10'-0" on center unless otherwise noted on the Drawings.
- Construction joints shall be furnished with a full length keyway centered on members. Where the size of key is not shown on the Drawings, the key shall be 25% of the cross section dimension of the member and minimum 1-1/2 inches into the first pour of concrete.
- Provide waterstops in construction joints in cast-in-place concrete elements that have one side exposed to the weather or soil and the other side occurring adjacent to enclosed space. Refer to Drawings and Specifications for other waterproofing and damp proofing details.
- Provide 3/4" by 3/4" chamfer strips at all exposed corners of concrete members, unless noted otherwise.
- Provide dovetail slots in concrete members where masonry abuts and where required for veneer attachment.
- The Concrete Contractor shall be responsible for all pour sequences and construction procedures for all concrete work to account for temperature differentials and shrinkage occurring during the construction phase until the building is permanently in a mechanically controlled environment.
- Coordinate vapor retarder requirements with floor finish requirements.
- Provide pockets or recesses in concrete work for steel columns and beams as required and/or as called for in the Specifications even if not shown on the Drawings. Provide concrete fill after steel erection.
- Refer to Architectural drawings for slab recesses and for floor finish materials and requirements.
- Provide recess in top of basement walls and grade beams, where applicable, for door openings, ramps, for support of thickened floor slabs, and to receive door jams.
- Concrete shall be placed to the constant top of slab elevations, while maintaining the minimum concrete thickness noted on the Drawings.
- Construction joints for supported slabs shall be as noted on the Drawing or as approved by the Structural Engineer.
- The use of chlorides such as deicing salts is prohibited for melting ice prior to placement of concrete.
- Sizes of concrete placements shall not exceed the following, unless otherwise indicated on the plans:
 - Walls: 40 feet maximum length.
 - Slabs on grade: 40 feet maximum length.
 - Supported slabs: Place in alternating strips (approximate width 30 feet & maximum length 200 feet). Place in sections with a maximum area of 12,000 square feet and a maximum length of 100 feet (all concrete slabs including those cast on metal deck).

CAST-IN-PLACE CONCRETE CONT.

- For floor finish tolerances for interior slabs, refer to Specifications.
- Curing of concrete surfaces shall conform to ACI 308.1 "Standard Specification for Concrete Curing" and ACI 308R "Guide to Curing Concrete".
- Joints between the structural (and architectural) members shall be properly prepared and filled with joint sealant unless noted otherwise. All joint edges, including top and bottom surfaces and vertical and horizontal surfaces shall be formed or tooled as required. Joint sealant shall be applied only to the top, vertical, and horizontal surfaces unless noted otherwise on the Drawings.
- Joints to be prepared and filled with joint sealant shall include, but are not limited to, construction joints, control joints, isolation joints, and all interface joints between similar and dissimilar members. Specific locations may be indicated on the Drawings, or may be required by approved shop drawings, or may occur due to the construction sequence selected by the Contractor.
- Prior to placing concrete adjacent to existing concrete, mechanically roughen, then thoroughly clean and de-grease existing concrete surfaces. Apply epoxy bonding agent prior to placing fresh concrete. Bonding agent shall be "Sika Armatec 110 EpoCem" by Sika Corporation, or approved equal. Follow all Manufacturer's instructions for surface preparation, mixing and application.
- Prior to placing concrete topping, mechanically roughen, then thoroughly clean and de-grease existing concrete surfaces. Soak existing concrete surfaces for minimum 12 hours. Place a concrete-slurry of cement and water within 1 hour of topping placement.
- Non-shrink grout shall conform to ASTM C1107. Grout shall be premixed, non-shrink, non-catalyzed natural aggregate grout with a minimum 7-day compressive strength of 7,000 psi plastic, 6,000 psi flowable, and 5,000 psi fluid consistency.
- Reinforcing steel, anchor rods and embed placement shall be inspected, prior to placement of concrete, in accordance with ACI 318 and code required Special Inspection by qualified Inspector prior. These inspections are not included in the basic services of the Structural Engineer of Record.

STRUCTURAL STEEL

- Design, fabrication and erection of structural steel shall be in accordance with the American Institute of Steel Construction (AISC) 360 Specification for Structural Steel Buildings and the Steel Construction Manual, Allowable Strength Design ASD.
- Structural steel shall conform to the following ASTM specifications and minimum yield strength:

W Shapes	A992 Gr. 50	F _y = 50 ksi
Miscellaneous shapes and plates	A36	F _y = 36 ksi
Round Tubes	A500 Grade B	F _y = 42 ksi
Pipe	A53 Grade B	F _y = 35 ksi
Square Tubes	A500 Grade B	F _y = 46 ksi
- Masonry and brick lintels shall be galvanized G90 per ASTM A123.
- Cheekered plate shall be F_y = 36 ksi steel per ASTM A786 and have medium raised lug pattern.
- Anchor rods shall conform to ASTM F1554 Grade 36, unless noted Grade 55 or other on Drawings.
- Structural steel bolting shall be ASTM A325 type N, 3/4" diameter snug tight except where other size, ASTM A490 N, pre-tensioned or slip-critical type bolts are indicated.
- ASTM A490 bolts in tension shall be pre-tensioned.
- Shear connectors shall conform to the requirements of "Structural Welding Code – Steel" of the American Welding Society, ANSI/AWS D1.1. F_y = 65 ksi, as manufactured by Nelson Stud Welding, Div. of TRW, or approved substitute, and welded as per Manufacturer's written instructions.
- Welding shall be done with appropriate E70 series electrodes compatible with the new and existing steel. Welds and welding procedures shall conform to, and welders shall be qualified in accordance with, the "Structural Welding Code - Steel" of the American Welding Society, ANSI/AWS D1.1.
- Where specified as AESS, steel and connections are Architecturally Exposed Structural Steel. Finish steel in compliance with AISC Code of Standard Practice for Steel Buildings and Bridges, Section 10 - Architecturally Exposed Structural Steel.
- Detailing shall be performed using rational engineering design and standard practice in accordance with the Contract Documents. The typical details shown are approximate only and do not indicate the required number of bolts or weld sizes, unless specifically noted.
- Contractor shall submit for review, typical connection details and calculations sealed by a Professional Engineer registered in the State in which the Project is being constructed for proposed connections and for connections not specifically designed and detailed. Follow the details shown where specific connections are detailed.
- Contractor shall submit for review, engineered drawings showing shop fabrication details, field assembly details and erection diagrams for all structural steel. Show at minimum all details included in these Contract Documents with additional erection details as required to completely define the interconnection of structural steel pieces.
- Fabricator shall be AISC Certified or have an AISC equivalent Quality Assurance program as certified by a qualified independent testing agency.
- Anchor rods, base plates and bearing plates shall be located and built into connecting work, pre-set by templates or similar method prior to concrete placement. Plates shall be set in full beds of non-shrink grout.
- Contractor shall reference Architectural drawings for miscellaneous shapes and plates not shown on structural drawings. These items shall be shop welded to the structural framing sections to minimize field welding.
- The length, dimension and connection detail from new structural member to existing structures shall be field verified before fabrication. Field modifications to the fabricated member or connection are not allowed without prior approval by the Structural Engineer. Contractor shall submit sketches or shop drawings detailing proposed modifications for approval.
- Roof openings, unless otherwise noted, to be framed with L5x3x1/4 LLV. Verify size and location of all openings with the trade involved.
- Contractor shall provide L4x4x1/4 seats at column webs where required for support of roof and floor decks. Provide angle outrigger from exterior columns for slab and roof edge plate support.
- Non-composite beam connections shall be capable of supporting minimum 50% of the Maximum Total Uniform Load, AISC Steel Construction Manual, unless specifically noted on the Drawings.
- Beam connections shall be standard AISC approved connections. Extended shear plate connections protruding from column web only approved where beams/girders on either side of column web have equally loaded bays.
- Simple shear connections shall be capable of end rotation as per the requirements of the AISC Specification, Simple Connections, Specification Section J1.2 and Manual Part 10.
- Connections shall be shop welded in accordance with latest AWS Specifications for E70XX electrodes and field bolted with ASTM A325 or ASTM A490 bolts.
- Contractor shall install A325 and A490 bolts in accordance with the "Specification for Structural Joints Using ASTM A325 or A490 Bolts." Snug tight condition shall be achieved using an impact wrench, to bring the connected plates into firm contact, except where noted as slip-critical, pre-tensioned or finger tight.
- Contractor shall provide slip-critical connections at braced frames, moment connections, beams and columns supporting cranes and equipment, mechanical penetrations and elevator room framing and where bolts are in tension.
- Contractor shall provide 3/4" diameter shoulder bolts, double nuts or tack welded nuts finger tight to allow vertical movement with lock washers at slotted connections of wind columns or as noted.
- Where field welding to existing structural steel is indicated, contractor shall thoroughly clean all surfaces to receive weld, removing rust, paint, dirt and other foreign matter in area of work. Provide fire watch protection acceptable to the Owner.
- Stiffener plates and bearing stiffeners are to be provided in pairs.
- Brick relieving angles shall be bolted to plate work as indicated and field welded after alignment to a tolerance of +/- 1/8" from the location specified on the Architectural drawings. The alignment and adjustment shall be done after placement of concrete on floor slabs and after placement of roofing including ballast where applicable.
- Fabrication and erection tolerances for brick frames shall conform to the requirements for Architecturally Exposed Structural Steel (AESS) per AISC 303 Code of Standard Practice for Steel Buildings and Bridges Section 10, with adjustment to meet the brick shell tolerance noted above unless otherwise specified.
- Secondary steel framing supporting exterior façade shall have connections with minimum 1 inch lateral) and 1/2 inch vertical adjustment - allowance each direction from center of attachment point. Contractor shall provide slotted holes and shims as required to provide adjustment.
- Clean steel per SSPC-SP3 and shall receive one shop coat of paint. Omit paint at holes for slip critical type connections, at structural steel to be fireproofed, encased or in contact with concrete, and on top flange of beams receiving shear connectors.
- Steel above the roof and outside the building envelope (exposed to weather) shall be cleaned per SSPC-SP6 and hot dip galvanized.
- Contractor shall control erection procedures and sequences with relation to temperature differentials, especially with respect to structural steel framing into concrete walls, beams or columns.
- Contractor shall provide temporary bracing as required to ensure stability of the structure under full design loads until the permanent bracing is in place. Provide necessary shoring where required during construction.
- The steel frame is self supporting for lateral loads after:
 - Connections, braces and moment frames have been completely welded and bolted.
 - Masonry bearing and shear walls have reached design strength.
 - The roof deck has been properly installed and attached.

A

B

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D

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G

H

MASONRY NOTES

- Concrete masonry has been designed in accordance with ACI 530, "Building Code Requirements for Masonry Structures" and shall be constructed in accordance with ACI 530.1, "Specifications for Masonry Structures".
- Concrete Masonry to have a minimum 28-day compressive strength $f_m=2,000$ psi unless noted otherwise.
- Concrete Masonry units (CMU) shall conform to the following standards:
 - Load-Bearing Units: ASTM C90
 - Medium Weight Units: 105 to 125 pcf
 - Normal Weight Units: greater than 125 pcf
- Load-bearing CMU shall be at minimum medium weight units, unless noted otherwise.
- Mortar for all masonry shall conform to ASTM C270 with minimum compressive strength of 1,800 psi. Mortar below grade shall be type M. Elsewhere mortar may be either type M or S unless specifically indicated otherwise. Use either Portland cement/line or masonry cement for mortar.
- Grout shall conform to ASTM C476 with minimum 28-day compressive strength of 3,000 psi.
- Steel bar reinforcement shall conform to ASTM A615, grade 60.
- Horizontal joint reinforcement shall be "Ladder" type with 3/16 diameter longitudinal bars. Spacing of horizontal joint reinforcing shall be 16" on center, maximum.
- Minimum vertical CMU wall reinforcing shall be continuous #5 bars at 48" on center, unless noted otherwise. Dowels to concrete foundation to match size and spacing of reinforcing unless noted otherwise. Reinforce CMU core at corners, each side of control joints and each side of wall openings with additional (2) #5 continuous vertical reinforcing bars.
- Vertical cells containing reinforcing and grout shall form a continuous cavity, free of mortar droppings.
- Horizontal lintels shall be placed at the top of all masonry wall openings with (2) #5 minimum continuous horizontal reinforcing bars positioned at the bottom of the fully grouted intel, unless noted otherwise. Coordinate intel elevations with Architectural Drawings and approved masonry reinforcing shop drawings.
- Horizontal bond beams shall be placed at all floor levels, all stair landing levels, roof level, and top of parapets. Bond beams shall be reinforced with (2) #5 minimum continuous horizontal reinforcing bars positioned at the top of the fully grouted bond beam, unless noted otherwise. Coordinate bond beam elevations with Architectural Drawings and approved masonry reinforcing shop drawings.
- Horizontal bond beam and vertical reinforcing shall be continuous unless noted otherwise. Lap splice reinforcing per the schedule below or use mechanical splices adequate for 125% of specified yield strength of the bar. Lap vertical reinforcement with minimum dowels of same size and spacing that have been previously installed in the foundations. Dowel embedment in concrete shall conform to the requirements of the CAST-IN-PLACE CONCRETE notes.

BAR SIZE	LAP SPLICE LENGTH
#4	24"
#5	30"
#6	48"
#7	Provide mechanical splice
- Reinforcing bars shall be held in position by wire ties or other approved means to insure design location and lap. Place bars and lap prior to grouting.
- Grouting of masonry walls shall conform to recommended procedure for "low lift grouting" or "high lift grouting" as outlined in the NCMA TEK 3-2A - "Grouting Concrete Masonry Walls" and ACI 530.1/ASCE 6 "Specification for Masonry Structures". Grout lifts shall not exceed 5 feet without mechanically consolidated (vibrated) grout pours.
- Lifts of grout shall be keyed 4 inches into the previous course of masonry below.
- Masonry below grade shall be grouted solid.
- Sampling and Testing of mortar and grout shall be in accordance ASTM C780 and ASTM C1019, respectively. One test of each is required for each 5,000 square foot of wall.
- Construction and testing of masonry prisms shall be in accordance with the procedure outlined in the ASTM C1314. Special Inspection shall include at minimum:
 - Mortar and grout testing.
 - Reinforcement placement and lap verification.
 - Verification of clear grout space prior to grouting.
 - Verification of proper grouting procedures (grout lift and consolidation).
- Contractor shall brace masonry walls to resist wind loads until floors and roofs are in place, and the masonry has reached 75% of the required strength f_m . Bracing shall be provided in accordance with OSHA - Construction Safety Standards for Masonry Wall Bracing and NCMA TEK 3-4B - "Bracing Concrete Masonry Walls During Construction".
- Contractor shall shore masonry walls above masonry bond beam lintels until the masonry is placed full height and has reached the required strength.

STEEL OPEN WEB JOISTS AND JOIST GIRDERS

- Steel open web joists and joist girders shall be designed, fabricated and erected in accordance with latest Steel Joist Institute (SJI) specifications.
- Manufacturers shall be members of the Steel Joist Institute (SJI).
- Where noted, joists and joist girders shall be designed for additional distributed loads, concentrated loads and moments as indicated.
- All roof joist and joist girders exposed to a work floor shall be designed for a single panel point load on the lower chord in addition to the specified loads as follows:

All occupancies	300 lbs
-----------------	---------
- Contractor shall modify joist and joist girder seats where indicated and as required for end reactions and moments noted.
- Minimum joists bearing on a CMU wall shall be as follows (measured from the face of the wall) unless noted otherwise:

6" at an 8" wall
8" at a 12" wall
- The maximum center-to-center spacing of steel joists shall be 6'-6" U.N.O.
- Contractor shall provide top chord extensions as shown on Structural or Architectural drawings.
- Contractor shall provide bottom chord extensions at all columns and exterior spandrels as noted. Where moment connections are indicated on drawings, connect bottom chord after dead load is applied. Size connection for loads indicated.
- Contractor shall provide connections for additional members and bracing shown within the joist depth.
- Contractor shall provide bridging meeting the minimum requirements of SJI, including wind uplift considerations, unless detailed otherwise. (Provide bridging at first interior panel point from support for wind uplift.) Horizontal bridging shall be continuous top and bottom, anchored at each end, and welded to each joist. Diagonal bridging shall be bolted to each joist and clamped at the intersection.
- Joist bridging in first joist space adjacent to masonry walls and shorter span steel wide flange beams shall be horizontal, not diagonal bridging.
- Joists and joist girders shall be cambered for deflection due to dead loads or as specifically noted on plan.
- Live load deflection shall not exceed span/360 for joists, special joists and joist girders.
- Joist and joist girders shall be connected to support in accordance with SJI minimum requirements, or as required to resist loads shown on the drawings.
- Stagger joists as required to achieve minimum bearing length per SJI.
- Contractor shall submit shop drawings with erection plans, details and loading diagrams for special joists and joist girders and camber for review by Structural Engineer. Shop drawings shall be sealed and signed by a Professional Engineer registered in the State the Project. Provide sealed design calculations for all special joists and joist girders for review by Structural Engineer.
- Joists and joist girders shall receive one coat of shop primer paint except those receiving fireproofing. Refer to Architectural drawings for fireproofing requirements.

STEEL DECK

- Steel decks shall be as noted on drawings, fabricated and erected in accordance with the latest Steel Deck Institute (SDI) specifications.
- Manufacturer shall be a member of the Steel Deck Institute (SDI).
- Steel roof deck shall conform to the requirements of and be a Factory Mutual approved product.
- Steel deck shall have galvanized coating conforming to ASTM A653, coating designation G90 for roof deck and G60 for floor deck. Touch-up paint galvanized surfaces with zinc rich paint after cutting and welding. Clean as required to receive fireproofing. Refer to Architectural drawings for fireproofing requirements.
- Design of floor deck as a form shall conform to the requirements of SDI Specifications and Commentary for composite steel floor deck, except that calculated theoretical deflections as defined under Paragraph 3.3 shall not exceed span/240 or 1/2", whichever is smaller.
- Contractor shall provide engineering calculations or published Manufacturer's data and independently certified test data verifying the specified deck requirements. Provide engineered and checked shop drawings indicating location, gage and size of each piece of decking. Erection drawings shall clearly show details, size and spacing of connections to structural framing and side laps.
- Steel deck shall be continuous over 3 spans in the direction indicated. Single and double spans, if required, shall satisfy load and deflection requirements.
- Contractor shall provide accessories including closures, "Z" closures, column closures, screed angles and girder fillers, as required to contain the slab concrete and as required to adequately support the steel deck all sides on the steel framing.
- Steel decking shall be welded as shown or minimum at maximum 12" on center to the supporting steel with 5/8" diameter puddle welds unless otherwise noted. Weld roof deck at maximum 6" on center to perimeter, moment frame and braced frame steel members. Side laps shall be fastened at maximum 30" on center.
- Steel decking shall be welded to structural steel by qualified welders using pre-qualified procedures. Establish a welding procedure for the plug weld of the steel decking to the structural steel for the particular gage used. Prior to the start of erection of the steel deck, each welder shall be qualified according to AWS requirements.
- No loads shall be permitted to be hung from any roof deck. Mechanical piping over 2 1/2 inches in diameter shall not be hung from floor deck. Hangers for ceilings, ductwork, electrical conduit, piping, etc. shall be directly from structural steel work or supplementary members.

POST-INSTALLED ANCHORS


- Post-installed anchors include all mechanical and adhesive anchors noted on Construction Documents. All post-installed anchors shall conform to AC193 for mechanical anchors and AC308 for adhesive anchors.
- Use only code approved anchors with valid ICC-ESR Evaluation Report for use in base material shown on the Construction Documents. Submit ICC-ESR Evaluation Report to Structural Engineer and Special Inspection Agent for approval.
- Installer of post-installed anchors shall be trained by anchor Manufacturer.
- Clean existing concrete surface to solid structural concrete. Grind smooth for full steel contact and to prevent gaps between steel and concrete. Alternatively, provide non-shrink grout in all voids between steel and base material.
- Drill smaller diameter pilot hole in existing concrete and check for existing reinforcing. Do not cut or damage existing reinforcing.
- If existing reinforcing is found, shift hole to avoid existing reinforcing. Submit location of new hole to Structural Engineer for review.
- Install mechanical anchors and adhesive anchors in strict accordance with Manufacturer's written recommendations and procedure detailed in ICC-ESR Evaluation Report.
- Special Inspections are required for all mechanical and adhesive anchors. Inspect and test post installed anchors as specified in ICC-ESR Evaluation Report.
- The following anchors are approved. Submittals for alternative equal anchors will be reviewed by Structural Engineer and approved at their discretion.

Anchor Type:	Approved Anchor	ICC-ESR Report No.	Base Material
Screw Anchors	Hilti Kwik HUS-EZ	ESR-3027 ESR-3056	Concrete Grouted Masonry
Steel Drop-In Anchor	Hilti HD/HDi-L Hilti HDi-P	(n/a) (n/a)	Concrete Precast Concrete
Expansion Anchors	Hilti Kwik Bolt TZ Hilti Kwik Bolt 3	ESR-1917 ESR-2302	Concrete Concrete (un-cracked only)
Adhesive Anchors	Hilti HIT-HY200 SAFESET Hilti HIT-HY70 + HAS/REBAR Hilti HIT-HY10 + HAS/REBAR	ESR-3187 ESR-3342 ESR-2682	Concrete Grouted Masonry Hollow Masonry


Note: Refer to plan notes, details and/or schedules for diameter of anchor rods or size of rebar used and the embed depth required for post-installed anchors.

COLD FORMED METAL FRAMING

- All cold formed metal framing members shall be designed, fabricated and erected in accordance with the AISI "Specifications for Design of Cold-Formed Steel Structural Members" and in accordance with Manufacturer's written instructions.
- All material shall conform to ASTM A1003, with minimum yield point of 33 ksi for 18 gauge and 50 ksi for 16 gauge and heavier material, and shall have galvanized coating conforming to ASTM A653-G60
- All welding shall conform to "Structural Welding Code - Sheet Steel" of the American Welding Society, AWS D1.3 and AWS D19.0 welding zinc coated steel.
- Unless specifically noted, all material shall be of a minimum metal thickness of 43 mils (18 gauge). Studs serving as backup for brick veneer shall be of a minimum metal thickness of 54 mils (16 gauge).
- All cold formed metal framing members shall meet the deflection requirements of the finish material to be attached to the cold formed metal framing work. Deflection of cold formed metal framing members serving as back up for brick veneer shall not exceed span/600 under serviceability wind load.
- When not specifically designed, the contractor shall submit calculations and layout for stud size, spacing and connection prepared and sealed by a Professional Engineer registered in the State of the Project for review by the Architect/Engineer.
- All studs and joists shall be installed at spacing indicated on the drawings, unless noted, each side of the openings shall be framed with double studs.
- All studs and joists shall have a bridging line installed at a maximum distances of 4'-0" and 5'-0", respectively.
- All Joists shall have web stiffeners at reaction points and concentrated loads.
- The nomenclature used for the design of cold formed metal framing is from the AISI Manual. All members supplied shall meet or exceed the strength shown in the AISI Manual.
- Structural connections of cold formed metal framing members shall be made per manufacturer's recommendations, adequate to carry the imposed loads, and conforming to the AISI and AWS specifications. Connection design to be based on reactions given on the Drawings or as listed in the Manufacturer's uniform loading capacity tables, whichever is greater.




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REGISTRATION SEAL



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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE


GENERAL STRUCTURAL NOTES


ISSUE DATES


_____	_____
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03/16/20 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN 

CHECKED 

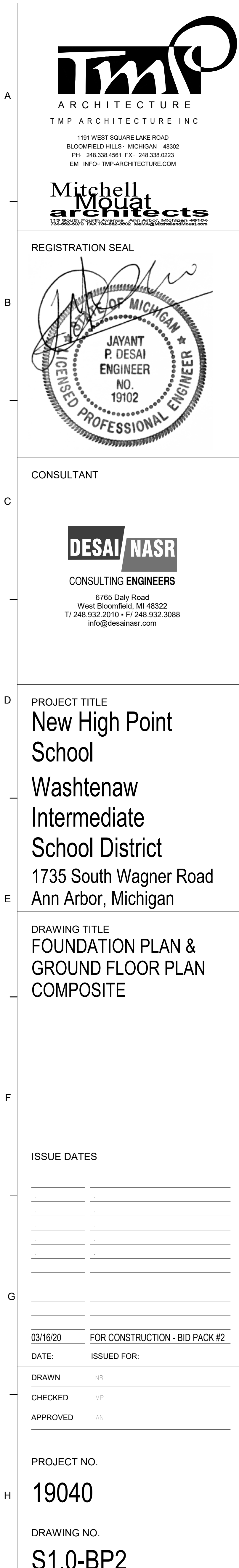
APPROVED 

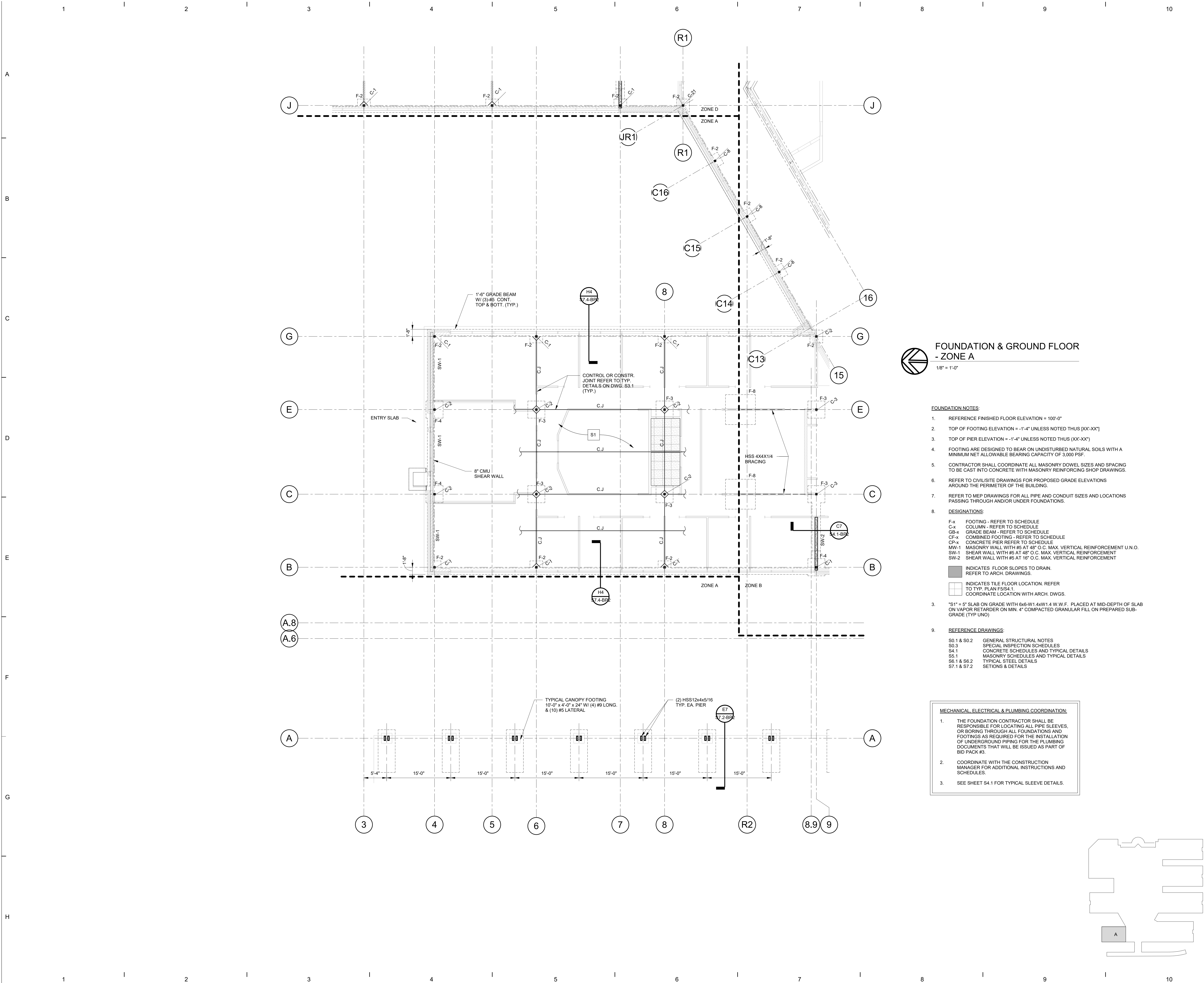
PROJECT NO.

19040

DRAWING NO.

S0.2-BP2





**FOUNDATION & GROUND FLOOR
- ZONE A**

FOUNDATION NOTES:

- REFERENCE FINISHED FLOOR ELEVATION = 100'-0"
- TOP OF FOOTING ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- TOP OF PIER ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- FOOTING ARE DESIGNED TO BEAR ON UNDISTURBED NATURAL SOILS WITH A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 3,000 PSF.
- CONTRACTOR SHALL COORDINATE ALL MASONRY Dowel SIZES AND SPACING TO BE CAST INTO CONCRETE WITH MASONRY REINFORCING SHOP DRAWINGS.
- REFER TO CIVIL/SITE DRAWINGS FOR PROPOSED GRADE ELEVATIONS AROUND THE PERIMETER OF THE BUILDING.
- REFER TO MEP DRAWINGS FOR ALL PIPE AND CONDUIT SIZES AND LOCATIONS PASSING THROUGH AND/OR UNDER FOUNDATIONS.
- DESIGNATIONS:**
 - F-x FOOTING - REFER TO SCHEDULE
 - C-x COLUMN - REFER TO SCHEDULE
 - GB-x GRADE BEAM - REFER TO SCHEDULE
 - CF-x COMBINED FOOTING - REFER TO SCHEDULE
 - CP-x CONCRETE PIER REFER TO SCHEDULE
 - MW-1 MASONRY WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT U.N.O.
 - SW-1 SHEAR WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT
 - SW-2 SHEAR WALL WITH #5 AT 16" O.C. MAX. VERTICAL REINFORCEMENT
- REFERENCE DRAWINGS:**
 - S0.1 & S0.2 GENERAL STRUCTURAL NOTES
 - S0.3 SPECIAL INSPECTION SCHEDULES
 - S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
 - S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
 - S6.1 & S6.2 TYPICAL STEEL DETAILS
 - S7.1 & S7.2 SETIONS & DETAILS

MECHANICAL, ELECTRICAL & PLUMBING COORDINATION:

- THE FOUNDATION CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL PIPE SLEEVES, OR BORING THROUGH ALL FOUNDATIONS AND FOOTINGS AS REQUIRED FOR THE INSTALLATION OF UNDERGROUND PIPING FOR THE PLUMBING DOCUMENTS THAT WILL BE ISSUED AS PART OF BID PACK #3.
- COORDINATE WITH THE CONSTRUCTION MANAGER FOR ADDITIONAL INSTRUCTIONS AND SCHEDULES.
- SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.

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REGISTRATION SEAL

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West Bloomfield, MI 48322
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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

FOUNDATION & GROUND FLOOR PLAN - ZONE A

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN: HB

CHECKED: WP

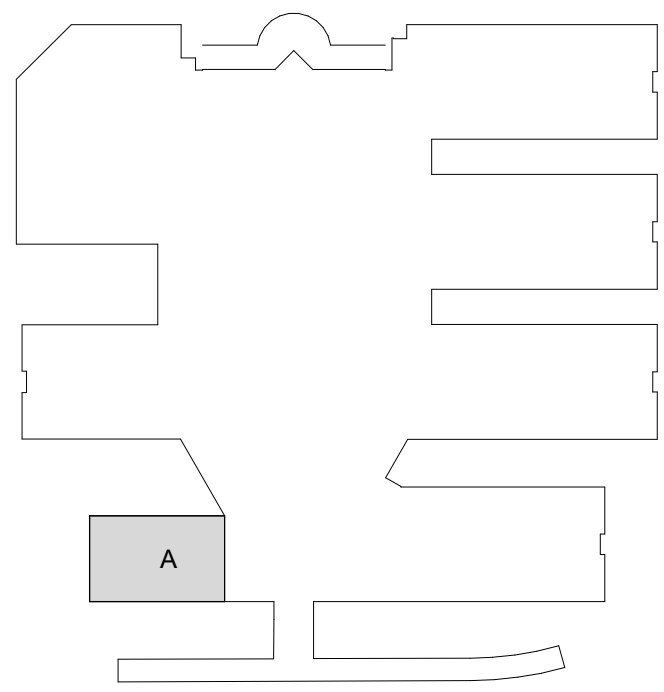
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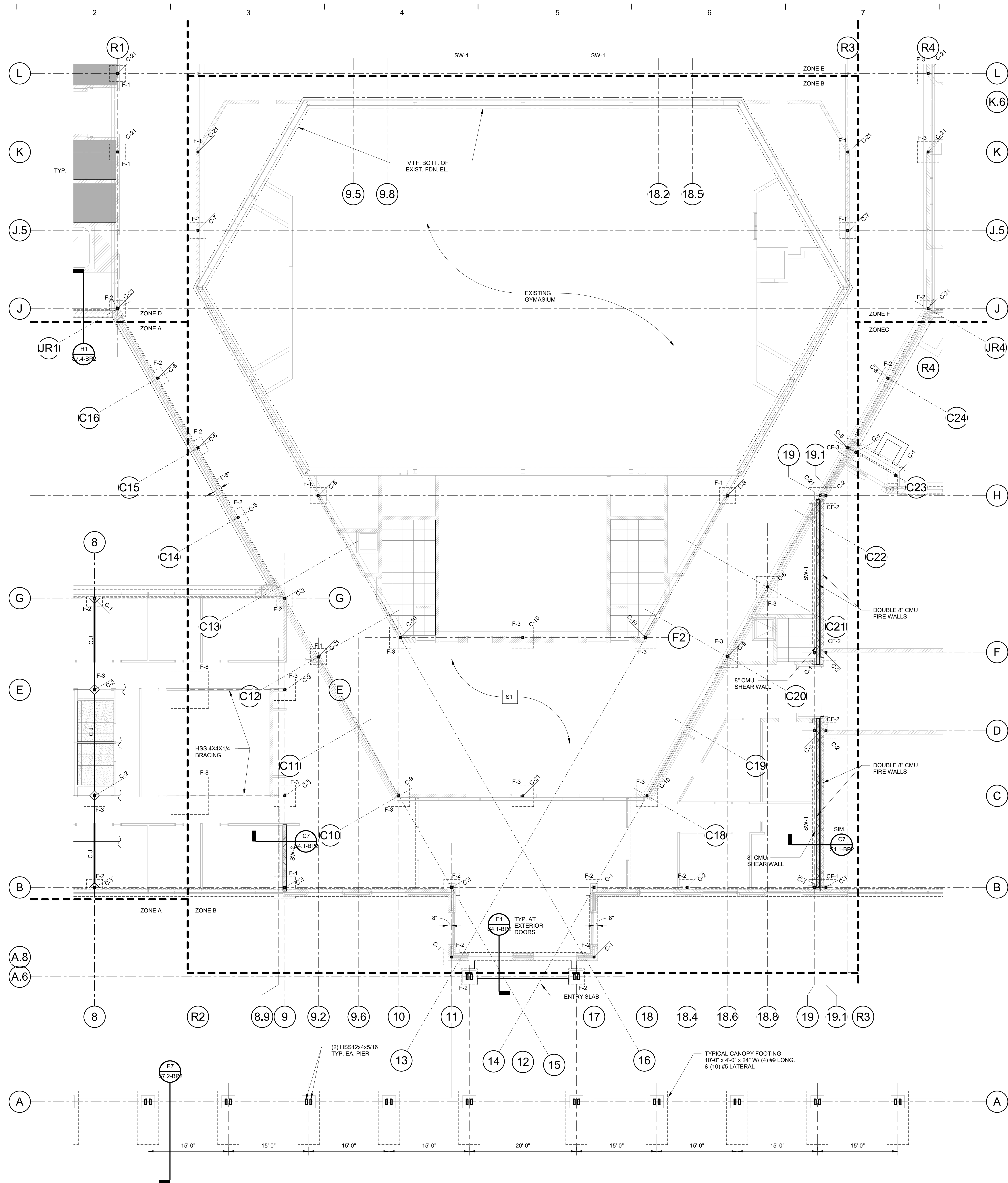
PROJECT NO.

19040

DRAWING NO.

S1.1-BP2





**FOUNDATION & GROUND FLOOR
- ZONE B**

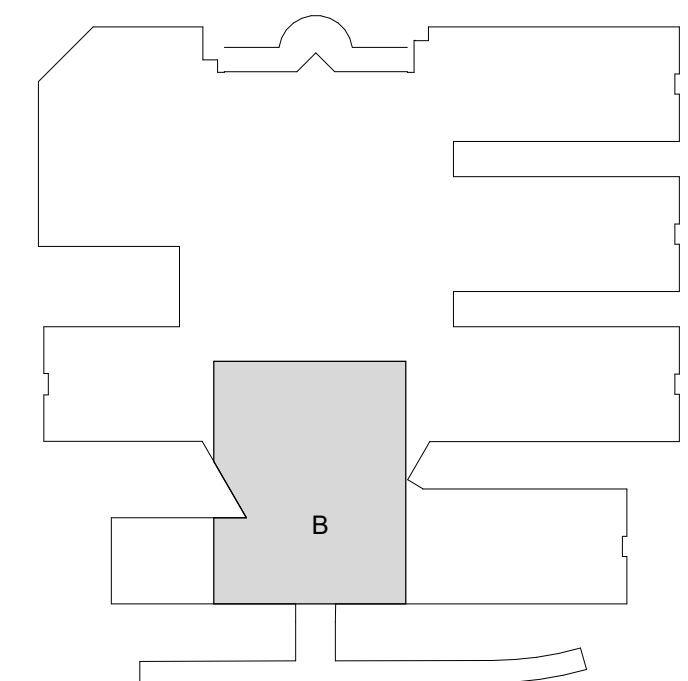
1/8" = 1'-0"

FOUNDATION NOTES:

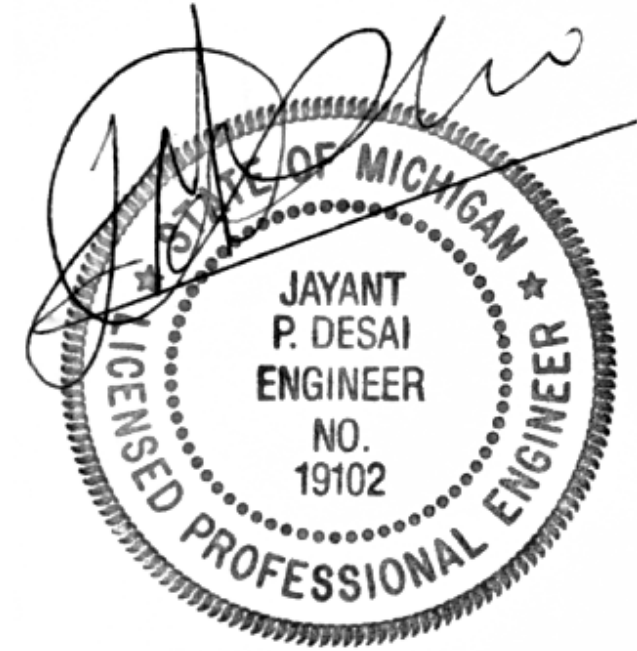
- REFERENCE FINISHED FLOOR ELEVATION = 100'-0"
- TOP OF FOOTING ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- TOP OF PIER ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- FOOTING ARE DESIGNED TO BEAR ON UNDISTURBED NATURAL SOILS WITH A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 3,000 PSF.
- CONTRACTOR SHALL COORDINATE ALL MASONRY DOVEL SIZES AND SPACING TO BE CAST INTO CONCRETE WITH MASONRY REINFORCING SHOP DRAWINGS.
- REFER TO CIVIL/SITE DRAWINGS FOR PROPOSED GRADE ELEVATIONS AROUND THE PERIMETER OF THE BUILDING.
- REFER TO MEP DRAWINGS FOR ALL PIPE AND CONDUIT SIZES AND LOCATIONS PASSING THROUGH AND/OR UNDER FOUNDATIONS.
- DESIGNATIONS:**
 - F-x FOOTING - REFER TO SCHEDULE
 - C-x COLUMN - REFER TO SCHEDULE
 - GB-x GRADE BEAM - REFER TO SCHEDULE
 - CF-x COMBINED FOOTING - REFER TO SCHEDULE
 - CP-x CONCRETE PIER REFER TO SCHEDULE
 - MM-1 MASONRY WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT U.N.O.
 - SW-1 SHEAR WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT
 - SW-2 SHEAR WALL WITH #5 AT 16" O.C. MAX. VERTICAL REINFORCEMENT
- INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
 - INDICATES TILE FLOOR LOCATION. REFER TO TYP. PLAN F5/S4.1. COORDINATE LOCATION WITH ARCH. DWGS.
- "S1" = 6" SLAB ON GRADE WITH 6x6-W1.4xW1.4 W.W.F. PLACED AT MID-DEPTH OF SLAB ON VAPOR RETARDER ON MIN. 4" COMPACTED GRANULAR FILL ON PREPARED SUB-GRADE (TYP UNO)
- REFERENCE DRAWINGS:**
 - S0.1 & S0.2 GENERAL STRUCTURAL NOTES
 - S0.3 SPECIAL INSPECTION SCHEDULES
 - S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
 - S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
 - S6.1 & S6.2 TYPICAL STEEL DETAILS
 - S7.1 & S7.2 SECTIONS & DETAILS

MECHANICAL, ELECTRICAL & PLUMBING COORDINATION:

- THE FOUNDATION CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL PIPE SLEEVES OR BORING THROUGH ALL FOUNDATIONS AND FOOTINGS AS REQUIRED FOR THE INSTALLATION OF UNDERGROUND PIPING FOR THE PLUMBING DOCUMENTS THAT WILL BE ISSUED AS PART OF BID PACK #3.
- COORDINATE WITH THE CONSTRUCTION MANAGER FOR ADDITIONAL INSTRUCTIONS AND SCHEDULES.
- SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.



REGISTRATION SEAL



CONSULTANT



PROJECT TITLE

New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

FOUNDATION & GROUND FLOOR PLAN - ZONE B

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:

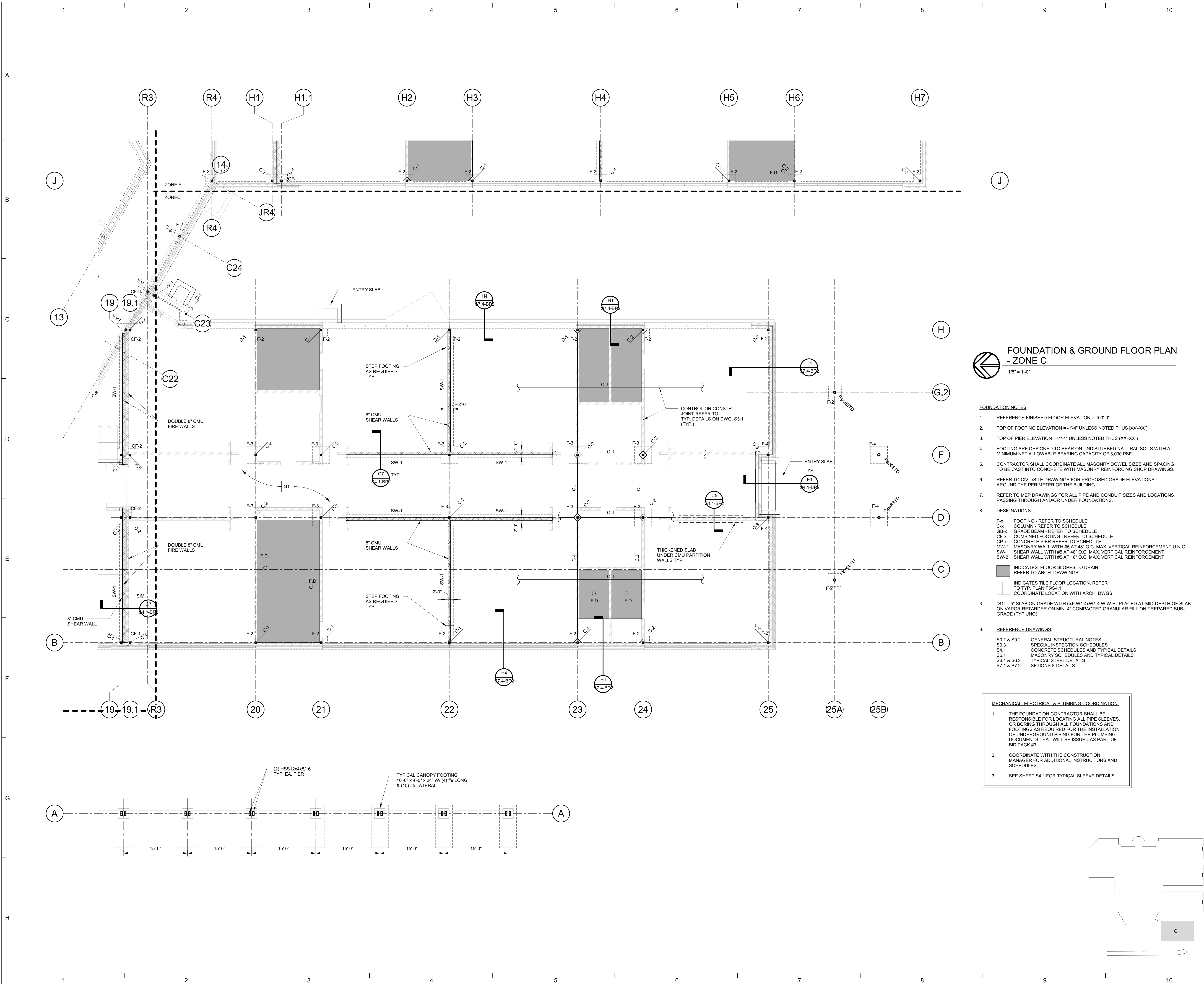
DRAWN: HB
CHECKED: WP
APPROVED: AN

PROJECT NO.

19040

DRAWING NO.

S1.2-BP2



FOUNDATION & GROUND FLOOR PLAN
- ZONE C

1/8" = 1'-0"

FOUNDATION NOTES:

- REFERENCE FINISHED FLOOR ELEVATION = 100'-0"
- TOP OF FOOTING ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- TOP OF PIER ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
- FOOTING ARE DESIGNED TO BEAR ON UNDISTURBED NATURAL SOILS WITH A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 3,000 PSF.
- CONTRACTOR SHALL COORDINATE ALL MASONRY DOVEL SIZES AND SPACING TO BE CAST INTO CONCRETE WITH MASONRY REINFORCING SHOP DRAWINGS.
- REFER TO CIVIL/SITE DRAWINGS FOR PROPOSED GRADE ELEVATIONS AROUND THE PERIMETER OF THE BUILDING.
- REFER TO MEP DRAWINGS FOR ALL PIPE AND CONDUIT SIZES AND LOCATIONS PASSING THROUGH AND/OR UNDER FOUNDATIONS.
- DESIGNATIONS:
F-x FOOTING - REFER TO SCHEDULE
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GB-x GRADE BEAM - REFER TO SCHEDULE
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SW-1 SHEAR WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT
SW-2 SHEAR WALL WITH #5 AT 16" O.C. MAX. VERTICAL REINFORCEMENT
- INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
- INDICATES TILE FLOOR LOCATION. REFER TO TYP. PLAN F5/S4.1. COORDINATE LOCATION WITH ARCH. DWGS.
- "S1" = 5" SLAB ON GRADE WITH 6x6-W1.4xW1.4 W.W.F. PLACED AT MID-DEPTH OF SLAB ON VAPOR RETARDER ON MIN. 4" COMPACTED GRANULAR FILL ON PREPARED SUB-GRADE (TYP UNO)
- REFERENCE DRAWINGS:
S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS

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- SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.



REGISTRATION SEAL



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PROJECT TITLE

New High Point
School
Washtenaw
Intermediate
School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

FOUNDATION &
GROUND FLOOR PLAN -
ZONE C

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:

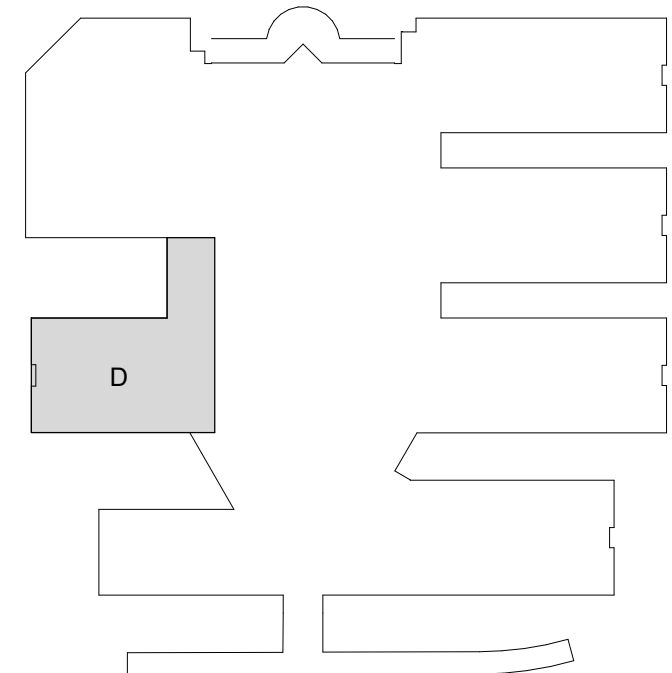
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APPROVED: AK

PROJECT NO.



19040

DRAWING NO.

S1.3-BP2

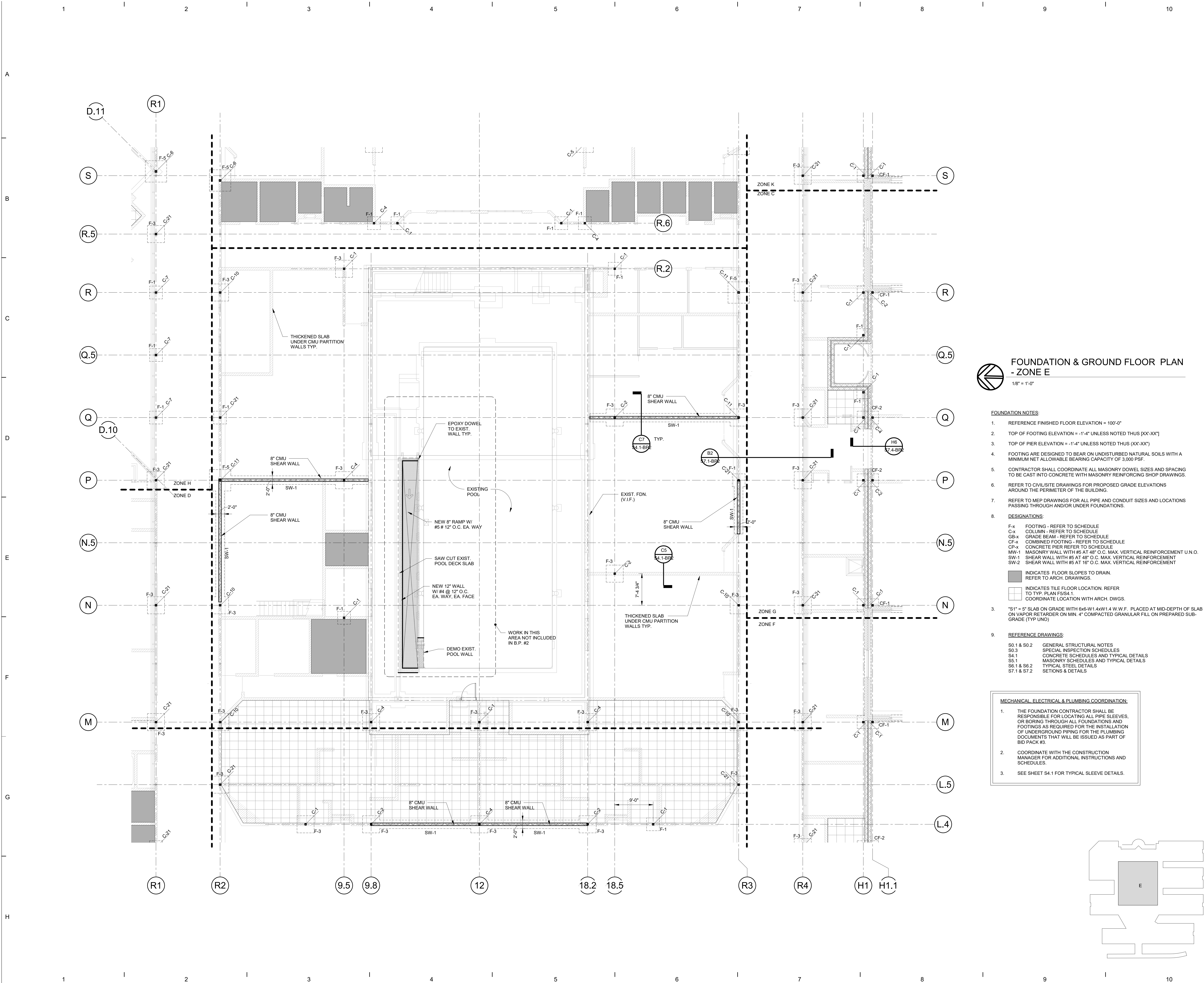


FOUNDATION NOTES:

-  INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
-  INDICATES TILE FLOOR LOCATION. REFER TO TYP. PLAN F5/S4.1. COORDINATE LOCATION WITH ARCH. DRAWINGS.

1. THE FOUNDATION CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL PIPE SLEEVES, OR BORING THROUGH ALL FOUNDATIONS AND FOOTINGS AS REQUIRED FOR THE INSTALLATION OF UNDERGROUND PIPING FOR THE PLUMBING DOCUMENTS THAT WILL BE ISSUED AS PART OF BID PACK #3.
2. COORDINATE WITH THE CONSTRUCTION MANAGER FOR ADDITIONAL INSTRUCTIONS AND SCHEDULES.
3. SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.

S1.4-BP2

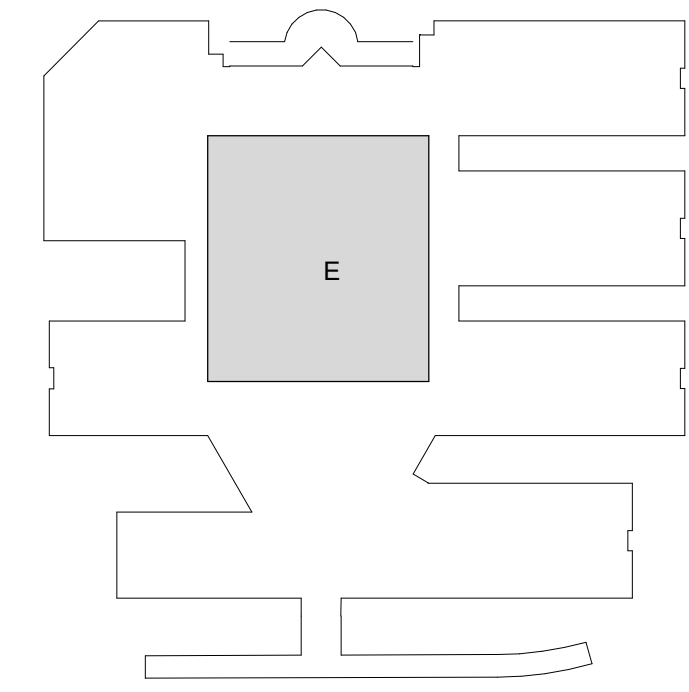


**FOUNDATION & GROUND FLOOR PLAN
- ZONE E**
1/8" = 1'-0"

- FOUNDATION NOTES:**
- REFERENCE FINISHED FLOOR ELEVATION = 100'-0"
 - TOP OF FOOTING ELEVATION = -1'-4" UNLESS NOTED THUS (XX-XX")
 - TOP OF PIER ELEVATION = -1'-4" UNLESS NOTED THUS (XX-XX")
 - FOOTING ARE DESIGNED TO BEAR ON UNDISTURBED NATURAL SOILS WITH A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 3,000 PSF.
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 - REFER TO CIVIL/SITE DRAWINGS FOR PROPOSED GRADE ELEVATIONS AROUND THE PERIMETER OF THE BUILDING.
 - REFER TO MEP DRAWINGS FOR ALL PIPE AND CONDUIT SIZES AND LOCATIONS PASSING THROUGH AND/OR UNDER FOUNDATIONS.
 - DESIGNATIONS:**
F-x FOOTING - REFER TO SCHEDULE
C-x COLUMN - REFER TO SCHEDULE
GB-x GRADE BEAM - REFER TO SCHEDULE
CF-x COMBINED FOOTING - REFER TO SCHEDULE
CP-x CONCRETE PIER REFER TO SCHEDULE
MW-1 MASONRY WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT U.N.O.
SW-1 SHEAR WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT
SW-2 SHEAR WALL WITH #5 AT 16" O.C. MAX. VERTICAL REINFORCEMENT
 - INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
INDICATES TILE FLOOR LOCATION. REFER TO TYP. PLAN FS/S4.1. COORDINATE LOCATION WITH ARCH. DWGS.
3. "S1" = 5" SLAB ON GRADE WITH 6x6-W1.4xW1.4 W.W.F. PLACED AT MID-DEPTH OF SLAB ON VAPOR RETARDER ON MIN. 4" COMPACTED GRANULAR FILL ON PREPARED SUB-GRADE (TYP. UNO)
9. **REFERENCE DRAWINGS:**
S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS

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- SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.



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Mitchell Mouat architects
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CONSULTANT

DESAI/NASR
CONSULTING ENGINEERS
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West Bloomfield, MI 48322
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info@desainasr.com

PROJECT TITLE
**New High Point School
Washtenaw Intermediate
School District**
1735 South Wagner Road
Ann Arbor, Michigan

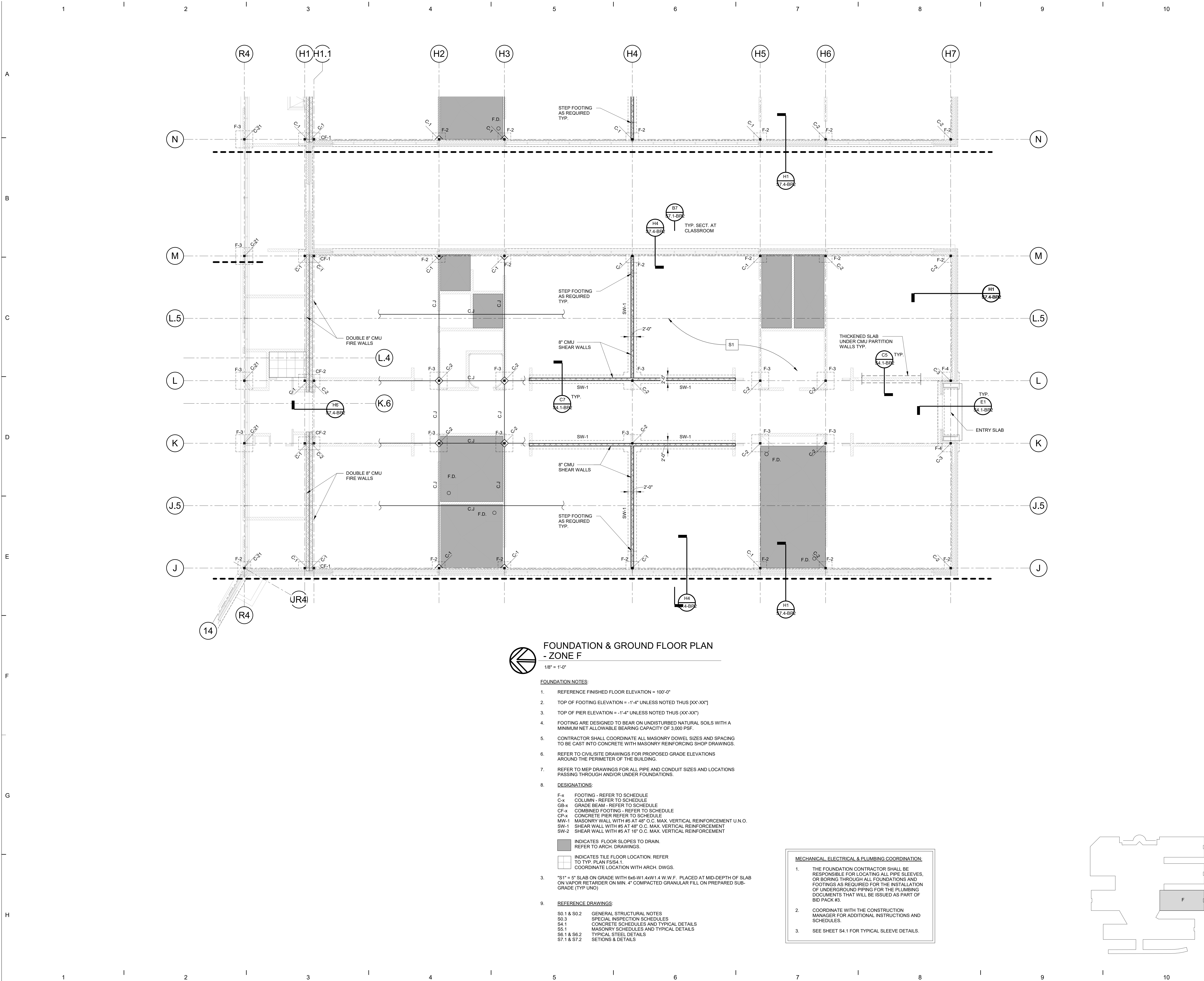
DRAWING TITLE
**FOUNDATION &
GROUND FLOOR PLAN -
ZONE E**

ISSUE DATES

03/16/20	FOR CONSTRUCTION - BID PACK #2
DATE:	ISSUED FOR:
DRAWN	MB
CHECKED	MP
APPROVED	AK

PROJECT NO.
19040

DRAWING NO.
S1.5-BP2



1191 WEST SQUARE LAKE ROAD
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West Bloomfield, MI 48322
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PROJECT TITLE

New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

FOUNDATION & GROUND FLOOR PLAN - ZONE F

ISSUE DATES

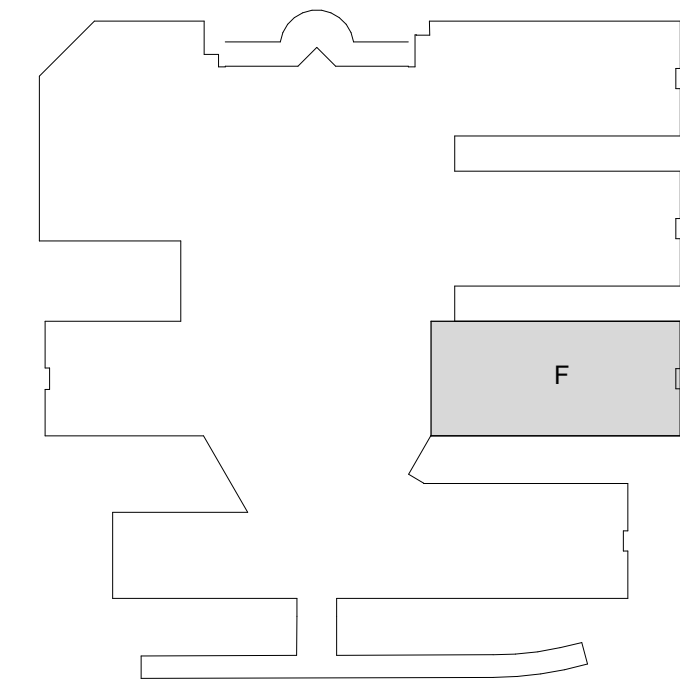
03/16/20	FOR CONSTRUCTION - BID PACK #2
DATE:	ISSUED FOR:
DRAWN	HB
CHECKED	WP
APPROVED	AK

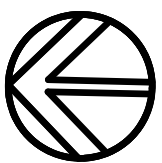
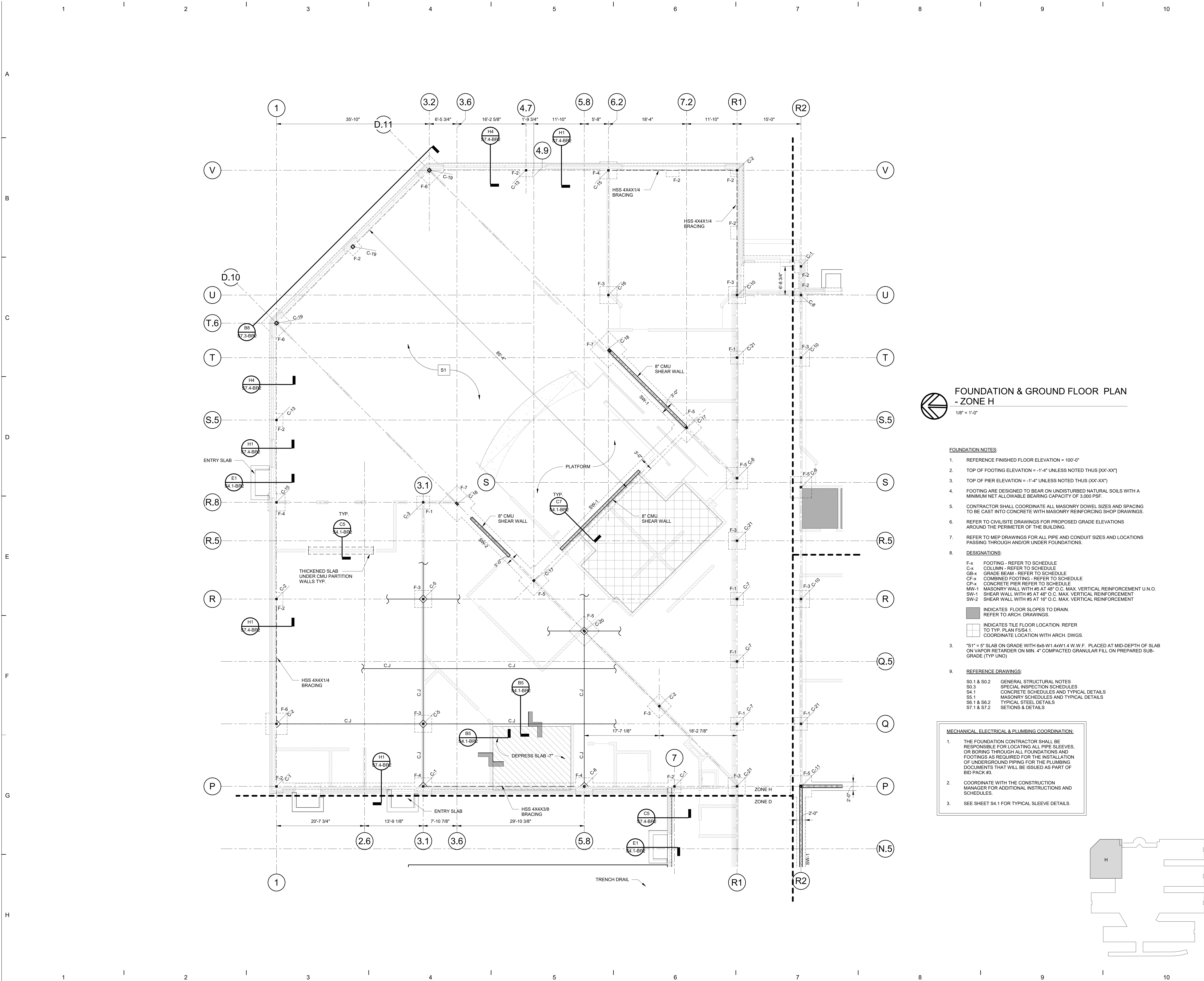
PROJECT NO.

19040

DRAWING NO.

S1.6-BP2





FOUNDATION & GROUND FLOOR PLAN
- ZONE H

1/8" = 1'-0"

FOUNDATION NOTES:

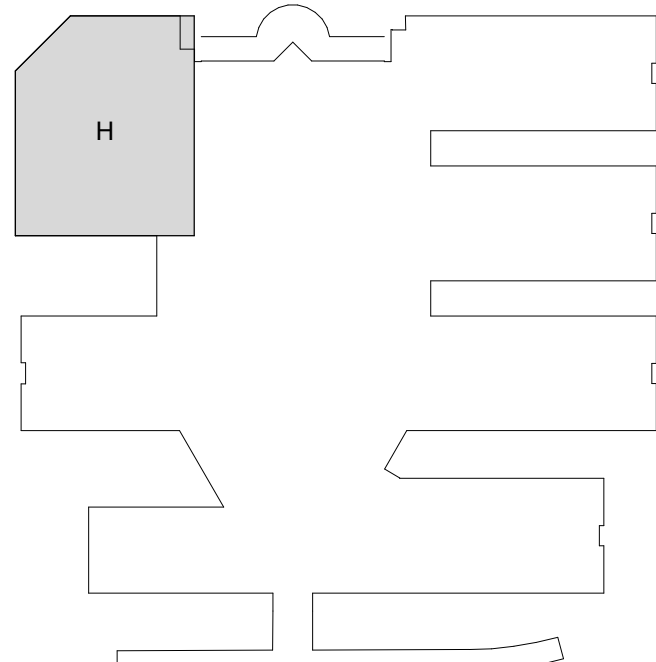
- REFERENCE FINISHED FLOOR ELEVATION = 100'-0"
 - TOP OF FOOTING ELEVATION = -1'-4" UNLESS NOTED THUS [XX'-XX"]
 - TOP OF PIER ELEVATION = -1'-4" UNLESS NOTED THUS (XX'-XX")
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 - MW-1 MASONRY WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT U.N.O.
 - SW-1 SHEAR WALL WITH #5 AT 48" O.C. MAX. VERTICAL REINFORCEMENT
 - SW-2 SHEAR WALL WITH #5 AT 16" O.C. MAX. VERTICAL REINFORCEMENT

- INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
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- REFERENCE DRAWINGS:
 - S0.1 & S0.2 GENERAL STRUCTURAL NOTES
 - S0.3 SPECIAL INSPECTION SCHEDULES
 - S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
 - SS.1 MASONRY SCHEDULES AND TYPICAL DETAILS
 - SS.1 & SS.2 TYPICAL STEEL DETAILS
 - S7.1 & S7.2 SETIONS & DETAILS

MECHANICAL, ELECTRICAL & PLUMBING COORDINATION:

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Mouat
architects**

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ANN ARBOR, MI 48106

REGISTRATION SEAL

JAYANT
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PROJECT TITLE

**New High Point
School**

**Washtenaw
Intermediate
School District**

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

**FOUNDATION &
GROUND FLOOR PLAN -
ZONE H**

ISSUE DATES

DATE	ISSUED FOR:
03/16/20	FOR CONSTRUCTION - BID PACK #2

DRAWN: HB

CHECKED: WP

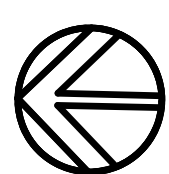
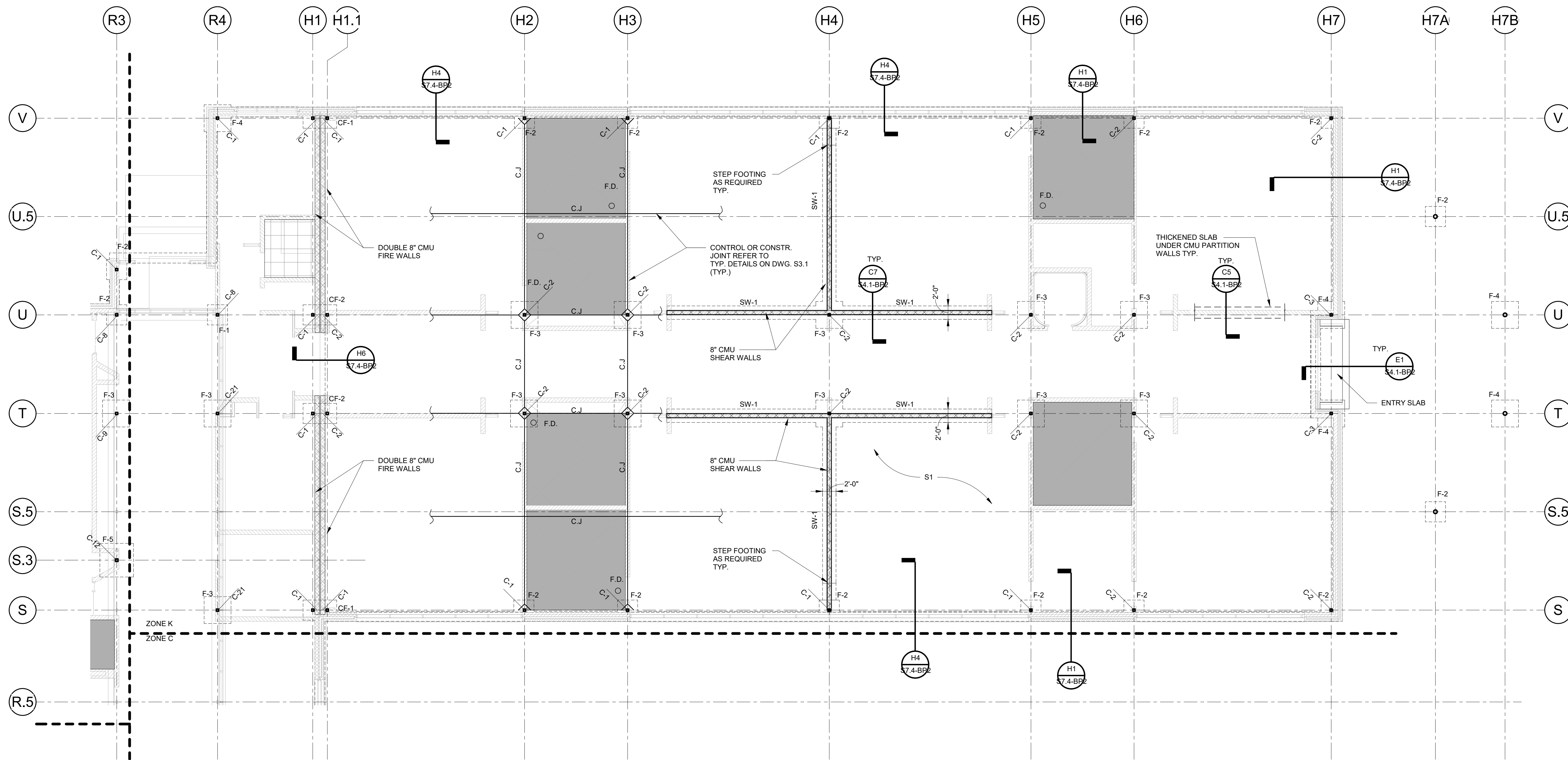
APPROVED: AN

PROJECT NO.

19040

DRAWING NO.

S1.8-BP2



FOUNDATION & GROUND FLOOR PLAN - ZONE K

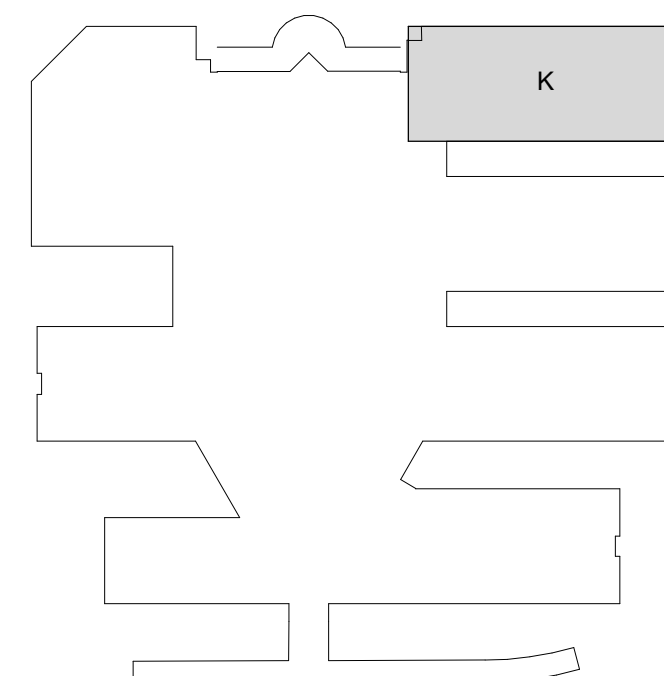
1/8" = 1'-0"

FOUNDATION NOTES:

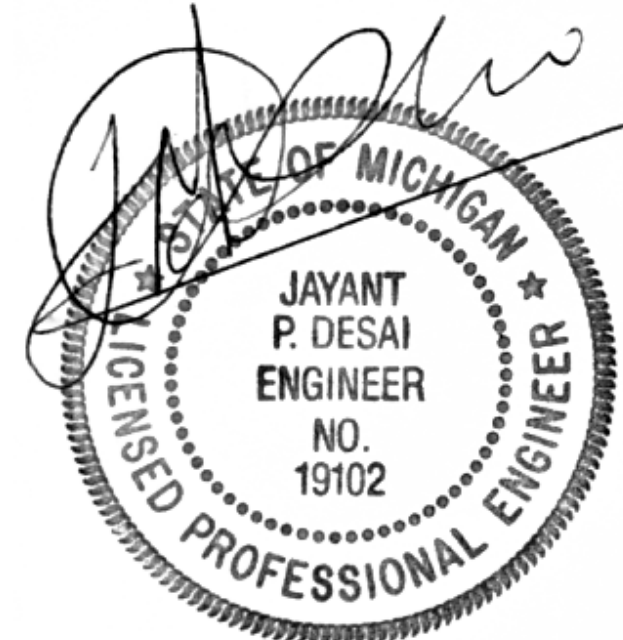
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- INDICATES FLOOR SLOPES TO DRAIN. REFER TO ARCH. DRAWINGS.
- INDICATES TILE FLOOR LOCATION. REFER TO TYP. PLAN F5/S4.1. COORDINATE LOCATION WITH ARCH. DWGS.
- "S1" = 5" SLAB ON GRADE WITH 6x6-W1.4xW1.4 W.W.F. PLACED AT MID-DEPTH OF SLAB ON VAPOR RETARDER ON MIN. 4" COMPACTED GRANULAR FILL ON PREPARED SUB-GRADE (TYP UNO).
- REFERENCE DRAWINGS:
 - S0.1 & S0.2 GENERAL STRUCTURAL NOTES
 - S0.3 SPECIAL INSPECTION SCHEDULES
 - S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
 - S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
 - S6.1 & S6.2 TYPICAL STEEL DETAILS
 - S7.1 & S7.2 SECTIONS & DETAILS

MECHANICAL, ELECTRICAL & PLUMBING COORDINATION:

- THE FOUNDATION CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL PIPE SLEEVES, OR BORING THROUGH ALL FOUNDATIONS AND FOOTINGS AS REQUIRED FOR THE INSTALLATION OF UNDERGROUND PIPING FOR THE PLUMBING DOCUMENTS THAT WILL BE ISSUED AS PART OF BID PACK #3.
- COORDINATE WITH THE CONSTRUCTION MANAGER FOR ADDITIONAL INSTRUCTIONS AND SCHEDULES.
- SEE SHEET S4.1 FOR TYPICAL SLEEVE DETAILS.



REGISTRATION SEAL



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PROJECT TITLE

New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

FOUNDATION & GROUND FLOOR PLAN - ZONE K

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:

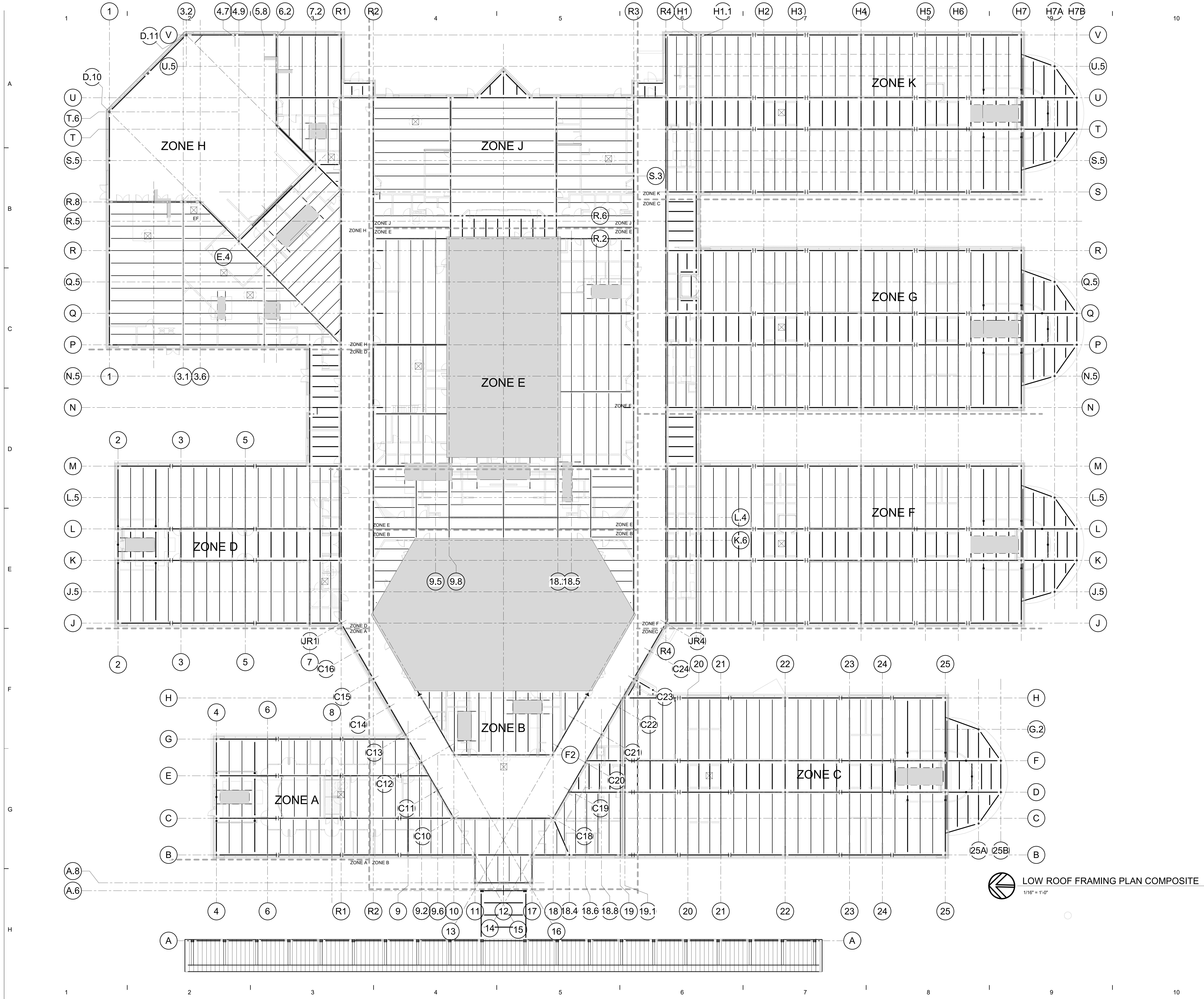
DRAWN HB
CHECKED MP
APPROVED AN

PROJECT NO.

19040

DRAWING NO.

S1.10-BP2





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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

LOW ROOF FRAMING PLAN COMPOSITE

ISSUE DATES

DATE	ISSUED FOR
03/16/20	FOR CONSTRUCTION - BID PACK #2

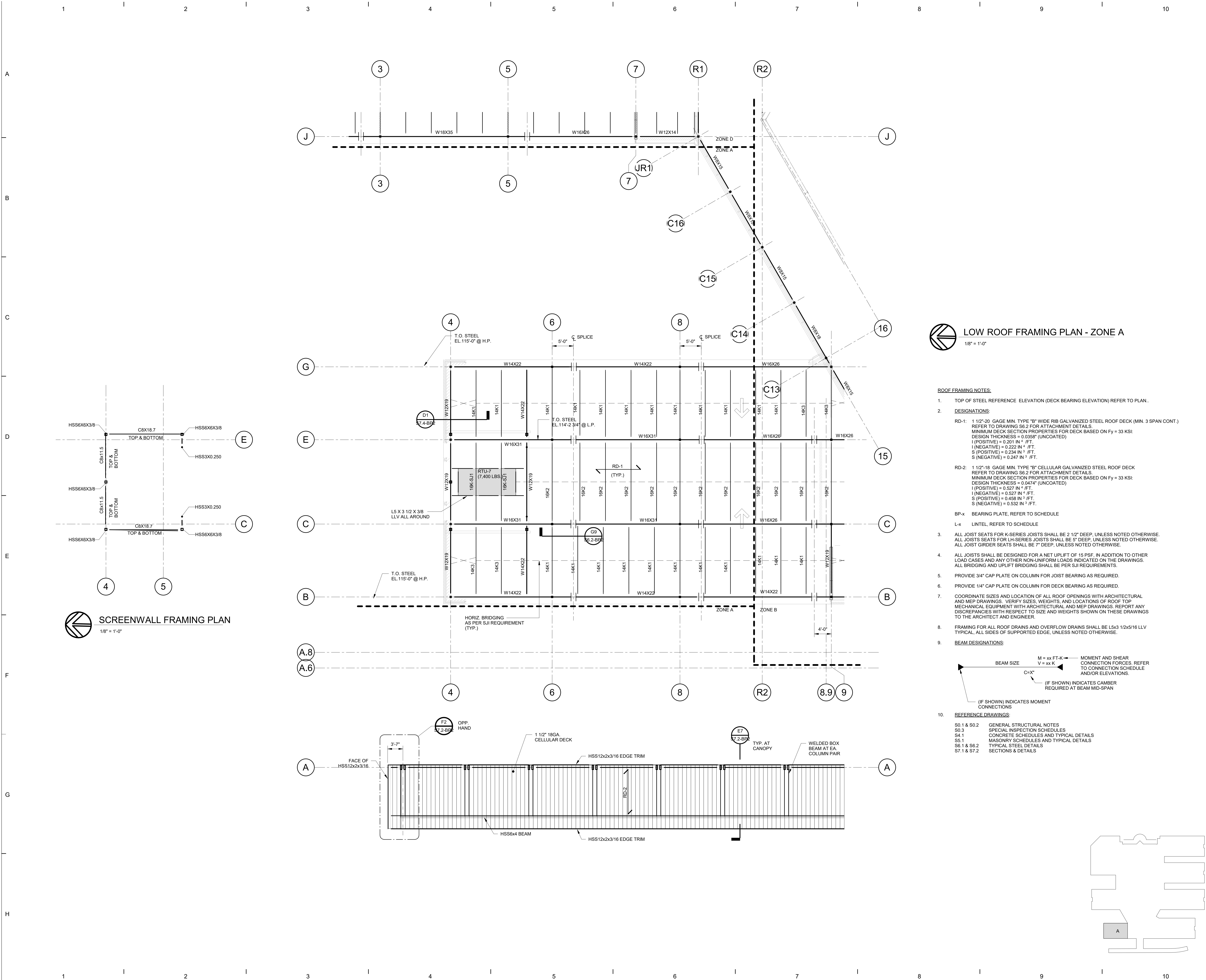
DRAWN: *MB*
CHECKED: *WP*
APPROVED: *AK*

PROJECT NO.

19040

DRAWING NO.

S2.0-BP2



LOW ROOF FRAMING PLAN - ZONE A
1/8" = 1'-0"

- ROOF FRAMING NOTES:**
- TOP OF STEEL REFERENCE ELEVATION (DECK BEARING ELEVATION) REFER TO PLAN.
 - DESIGNATIONS:**
RD-1: 1 1/2"-20 GAGE MIN. TYPE "B" WIDE RIB GALVANIZED STEEL ROOF DECK (MIN. 3 SPAN CONT.) REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS.
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON $F_y = 33$ KSI:
DESIGN THICKNESS = 0.0358" (UNCOATED)
I (POSITIVE) = 0.201 IN⁴ /FT.
I (NEGATIVE) = 0.222 IN⁴ /FT.
S (POSITIVE) = 0.234 IN³ /FT.
S (NEGATIVE) = 0.247 IN³ /FT.
RD-2: 1 1/2"-18 GAGE MIN. TYPE "B" CELLULAR GALVANIZED STEEL ROOF DECK REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS.
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON $F_y = 33$ KSI:
DESIGN THICKNESS = 0.0474" (UNCOATED)
I (POSITIVE) = 0.527 IN⁴ /FT.
I (NEGATIVE) = 0.527 IN⁴ /FT.
S (POSITIVE) = 0.458 IN³ /FT.
S (NEGATIVE) = 0.532 IN³ /FT.
BP-x BEARING PLATE, REFER TO SCHEDULE
L-x LINTEL, REFER TO SCHEDULE
 - ALL JOIST SEATS FOR K-SERIES JOISTS SHALL BE 2 1/2" DEEP, UNLESS NOTED OTHERWISE. ALL JOISTS SEATS FOR LH-SERIES JOISTS SHALL BE 5" DEEP, UNLESS NOTED OTHERWISE. ALL JOIST GIRDER SEATS SHALL BE 7" DEEP, UNLESS NOTED OTHERWISE.
 - ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF. IN ADDITION TO OTHER LOAD CASES AND ANY OTHER NON-UNIFORM LOADS INDICATED ON THE DRAWINGS. ALL BRIDGING AND UPLIFT BRIDGING SHALL BE PER SJI REQUIREMENTS.
 - PROVIDE 3/4" CAP PLATE ON COLUMN FOR JOIST BEARING AS REQUIRED.
 - PROVIDE 1/4" CAP PLATE ON COLUMN FOR DECK BEARING AS REQUIRED.
 - COORDINATE SIZES AND LOCATION OF ALL ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. VERIFY SIZES, WEIGHTS, AND LOCATIONS OF ROOF TOP MECHANICAL EQUIPMENT WITH ARCHITECTURAL AND MEP DRAWINGS. REPORT ANY DISCREPANCIES WITH RESPECT TO SIZE AND WEIGHTS SHOWN ON THESE DRAWINGS TO THE ARCHITECT AND ENGINEER.
 - FRAMING FOR ALL ROOF DRAINS AND OVERFLOW DRAINS SHALL BE L5x3 1/2x5/16 LLV TYPICAL. ALL SIDES OF SUPPORTED EDGE, UNLESS NOTED OTHERWISE.
 - BEAM DESIGNATIONS:**
 $M = xx$ FT-K
 $V = xx$ K
C=x" (IF SHOWN) INDICATES CAMBER REQUIRED AT BEAM MID-SPAN
(IF SHOWN) INDICATES MOMENT CONNECTIONS
 - REFERENCE DRAWINGS:**
S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS

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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

LOW ROOF FRAMING PLAN - ZONE A

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

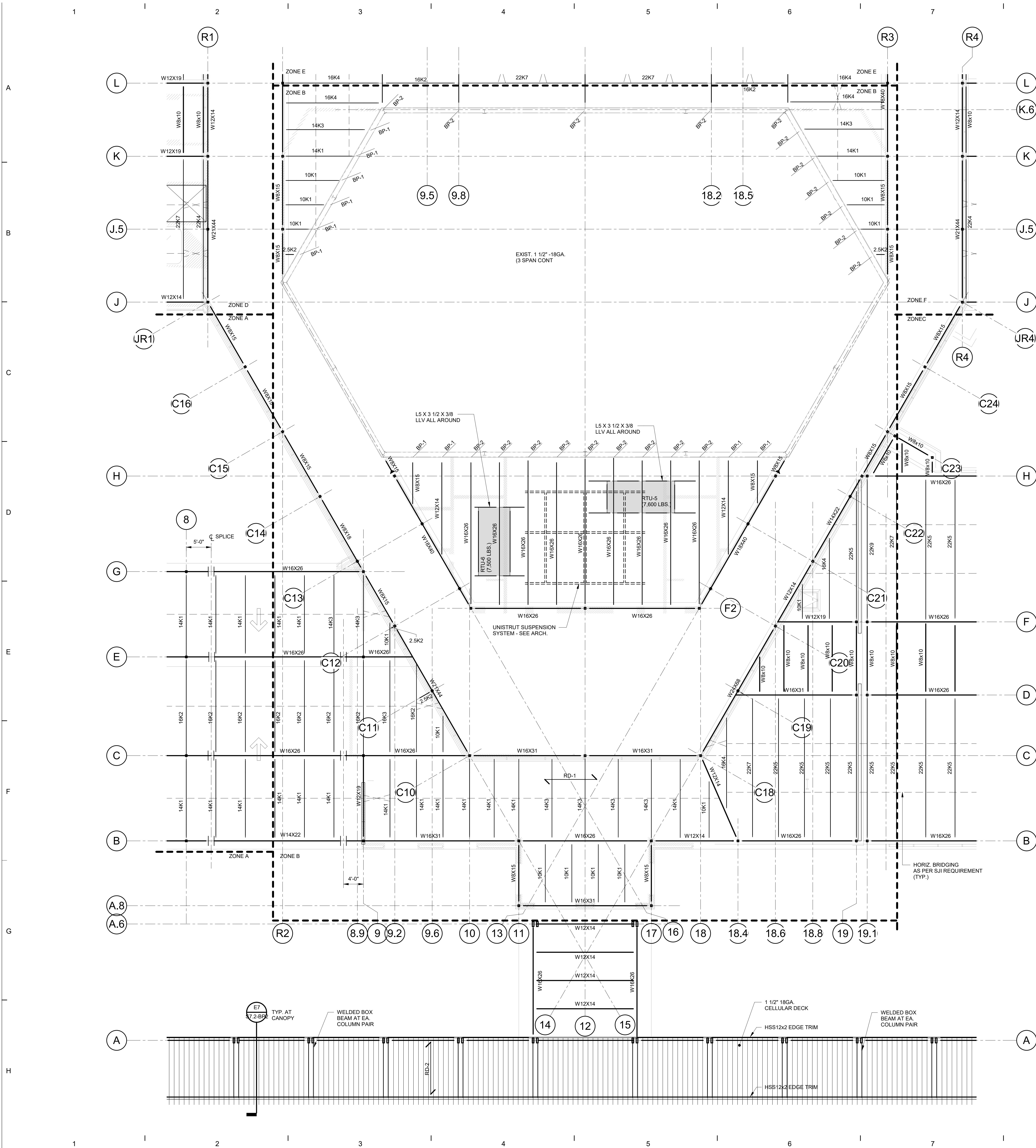
DRAWN	HB
CHECKED	WP
APPROVED	AK

PROJECT NO.

19040

DRAWING NO.

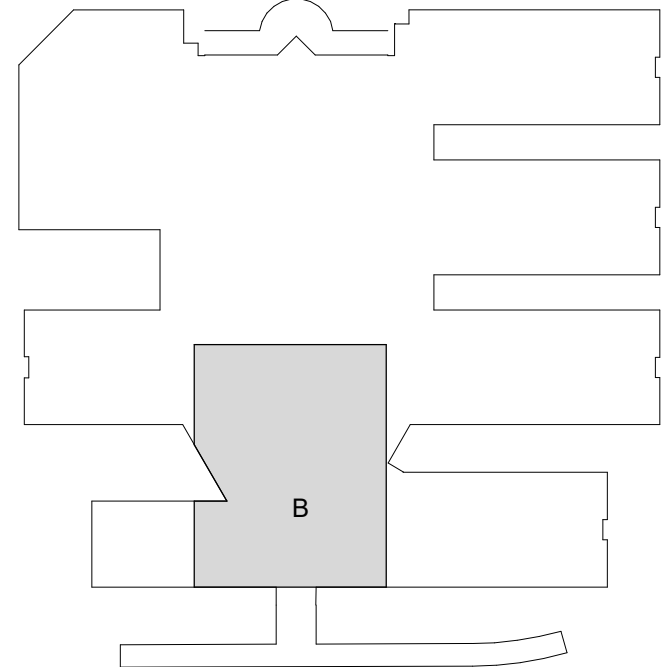
S2.1-BP2



LOW ROOF FRAMING PLAN - ZONE B
1/8" = 1'-0"

- ROOF FRAMING NOTES:**
- TOP OF STEEL REFERENCE ELEVATION (DECK BEARING ELEVATION) REFER TO PLAN.
 - DESIGNATIONS:**
RD-1: 1 1/2"-20 GAGE MIN. TYPE "B" WIDE RIB GALVANIZED STEEL ROOF DECK (MIN. 3 SPAN CONT.) REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS. MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI:
DESIGN THICKNESS = 0.0358" (UNCOATED)
I (POSITIVE) = 0.201 IN⁴ / FT.
I (NEGATIVE) = 0.222 IN⁴ / FT.
S (POSITIVE) = 0.234 IN³ / FT.
S (NEGATIVE) = 0.247 IN³ / FT.
RD-2: 1 1/2"-18 GAGE MIN. TYPE "B" CELLULAR GALVANIZED STEEL ROOF DECK REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS. MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI:
DESIGN THICKNESS = 0.0474" (UNCOATED)
I (POSITIVE) = 0.527 IN⁴ / FT.
I (NEGATIVE) = 0.527 IN⁴ / FT.
S (POSITIVE) = 0.458 IN³ / FT.
S (NEGATIVE) = 0.532 IN³ / FT.
BP-x BEARING PLATE, REFER TO SCHEDULE
L-x LINTEL, REFER TO SCHEDULE
 - ALL JOIST SEATS FOR K-SERIES JOISTS SHALL BE 2 1/2" DEEP, UNLESS NOTED OTHERWISE. ALL JOIST SEATS FOR LH-SERIES JOISTS SHALL BE 5" DEEP, UNLESS NOTED OTHERWISE. ALL JOIST GIRDER SEATS SHALL BE 7" DEEP, UNLESS NOTED OTHERWISE.
 - ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF, IN ADDITION TO OTHER LOAD CASES AND ANY OTHER NON-UNIFORM LOADS INDICATED ON THE DRAWINGS. ALL BRIDGING AND UPLIFT BRIDGING SHALL BE PER SJI REQUIREMENTS.
 - PROVIDE 3/4" CAP PLATE ON COLUMN FOR JOIST BEARING AS REQUIRED.
 - PROVIDE 1/4" CAP PLATE ON COLUMN FOR DECK BEARING AS REQUIRED.
 - COORDINATE SIZES AND LOCATION OF ALL ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. VERIFY SIZES, WEIGHTS, AND LOCATIONS OF ROOF TOP MECHANICAL EQUIPMENT WITH ARCHITECTURAL AND MEP DRAWINGS. REPORT ANY DISCREPANCIES WITH RESPECT TO SIZE AND WEIGHTS SHOWN ON THESE DRAWINGS TO THE ARCHITECT AND ENGINEER.
 - FRAMING FOR ALL ROOF DRAINS AND OVERFLOW DRAINS SHALL BE L5x3 1/2x5/16 LLV TYPICAL, ALL SIDES OF SUPPORTED EDGE, UNLESS NOTED OTHERWISE.
 - BEAM DESIGNATIONS:**

BEAM SIZE: M = xx FT-K, V = xx K
C=x" (IF SHOWN) INDICATES CAMBER REQUIRED AT BEAM MID-SPAN
(IF SHOWN) INDICATES MOMENT CONNECTIONS
 - REFERENCE DRAWINGS:**
S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS



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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

LOW ROOF FRAMING PLAN - ZONE B

ISSUE DATES

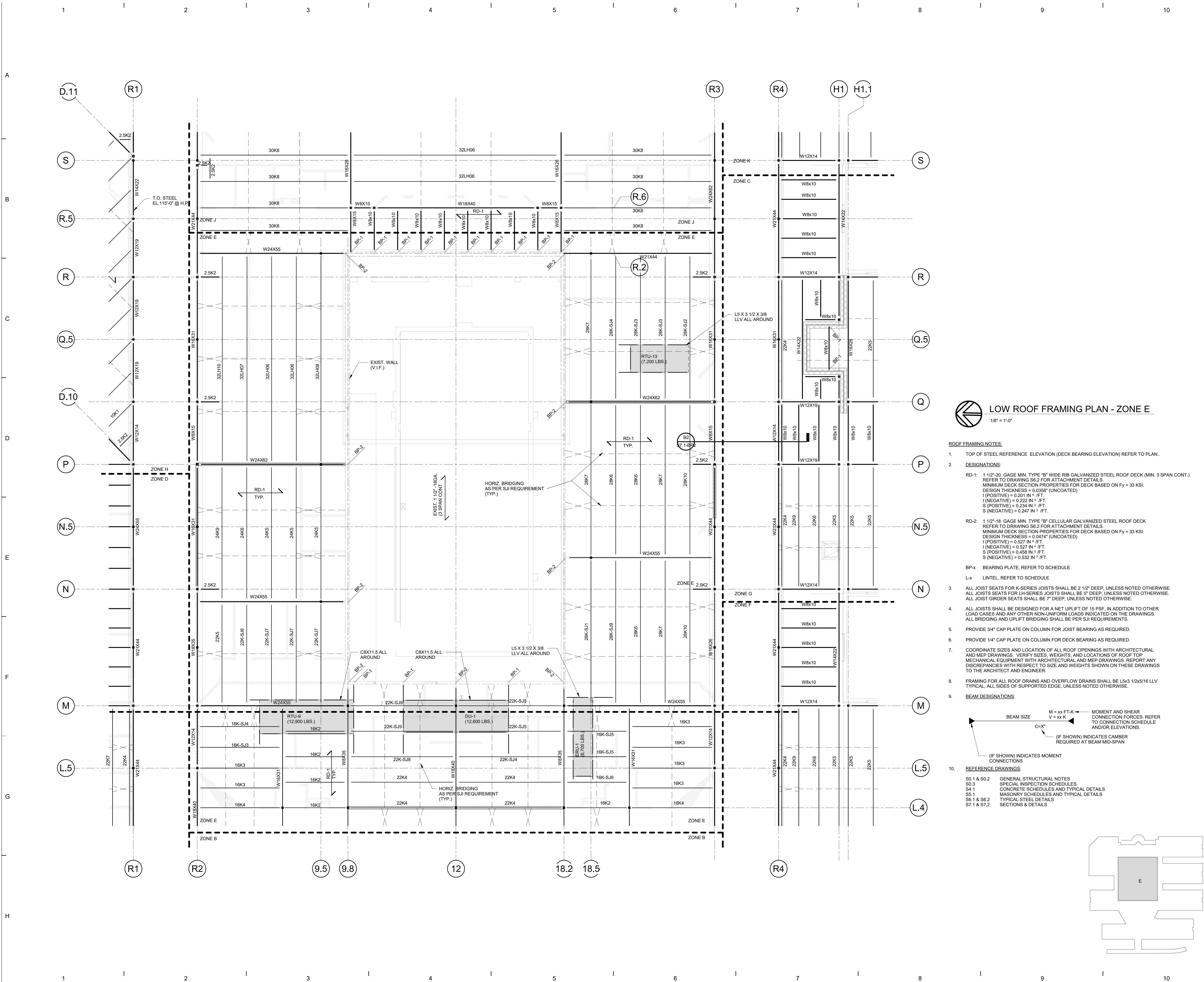
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DATE:	ISSUED FOR:
DRAWN: HB	
CHECKED: WP	
APPROVED: AN	

PROJECT NO.

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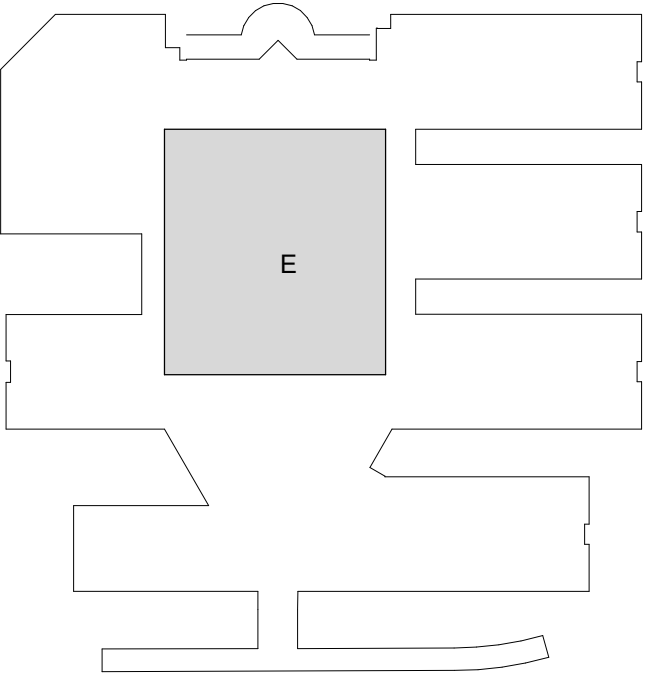
DRAWING NO.

S2.2-BP2



LOW ROOF FRAMING PLAN - ZONE E

1. TOP OF STEEL REFERENCE ELEVATION (DECK BEARING ELEVATION) REFER TO PLAN.
2. DESIGNATIONS:
- RD-1: 1 1/2"-20 GAGE MIN. TYPE "B" WIDE RIB GALVANIZED STEEL ROOF DECK (MIN. 3 SPAN CONT.) REFER TO DRAWING SB-2 FOR ATTACHMENT DETAILS. MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI: DESIGN THICKNESS = 0.0358" (UNCOATED) I (POSITIVE) = 0.201 IN⁴ / FT. I (NEGATIVE) = 0.222 IN⁴ / FT. S (POSITIVE) = 0.234 IN³ / FT. S (NEGATIVE) = 0.247 IN³ / FT.
- RD-2: 1 1/2"-18 GAGE MIN. TYPE "B" CELLULAR GALVANIZED STEEL ROOF DECK REFER TO DRAWING SB-2 FOR ATTACHMENT DETAILS. MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI: DESIGN THICKNESS = 0.0474" (UNCOATED) I (POSITIVE) = 0.327 IN⁴ / FT. I (NEGATIVE) = 0.527 IN⁴ / FT. S (POSITIVE) = 0.458 IN³ / FT. S (NEGATIVE) = 0.532 IN³ / FT.
- BP-x BEARING PLATE, REFER TO SCHEDULE
- L-x LINTEL, REFER TO SCHEDULE
3. ALL JOIST SEATS FOR K-SERIES JOISTS SHALL BE 2 1/2" DEEP, UNLESS NOTED OTHERWISE. ALL JOISTS SEATS FOR LH-SERIES JOISTS SHALL BE 5" DEEP, UNLESS NOTED OTHERWISE. ALL JOIST GIRDER SEATS SHALL BE 7" DEEP, UNLESS NOTED OTHERWISE.
4. ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF, IN ADDITION TO OTHER LOAD CASES AND ANY OTHER NON-UNIFORM LOADS INDICATED ON THE DRAWINGS. ALL BRIDGING AND UPLIFT BRIDGING SHALL BE PER SJI REQUIREMENTS.
5. PROVIDE 3/4" CAP PLATE ON COLUMN FOR JOIST BEARING AS REQUIRED.
6. PROVIDE 1/4" CAP PLATE ON COLUMN FOR DECK BEARING AS REQUIRED.
7. COORDINATE SIZES AND LOCATION OF ALL ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. VERIFY SIZES, WEIGHTS, AND LOCATIONS OF ROOF TOP MECHANICAL EQUIPMENT WITH ARCHITECTURAL AND MEP DRAWINGS. REPORT ANY DISCREPANCIES WITH RESPECT TO SIZE AND WEIGHTS SHOWN ON THESE DRAWINGS TO THE ARCHITECT AND ENGINEER.
8. FRAMING FOR ALL ROOF DRAINS AND OVERFLOW DRAINS SHALL BE L5x3 1/2x5/16 LLV TYPICAL, ALL SIDES OF SUPPORTED EDGE, UNLESS NOTED OTHERWISE.
9. BEAM DESIGNATIONS:
- BEAM SIZE M = xx FT-K V = xx K C=x" (IF SHOWN) INDICATES CAMBER REQUIRED AT BEAM MID-SPAN (IF SHOWN) INDICATES MOMENT CONNECTIONS
10. REFERENCE DRAWINGS:
- S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS



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PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
LOW ROOF FRAMING PLAN - ZONE E

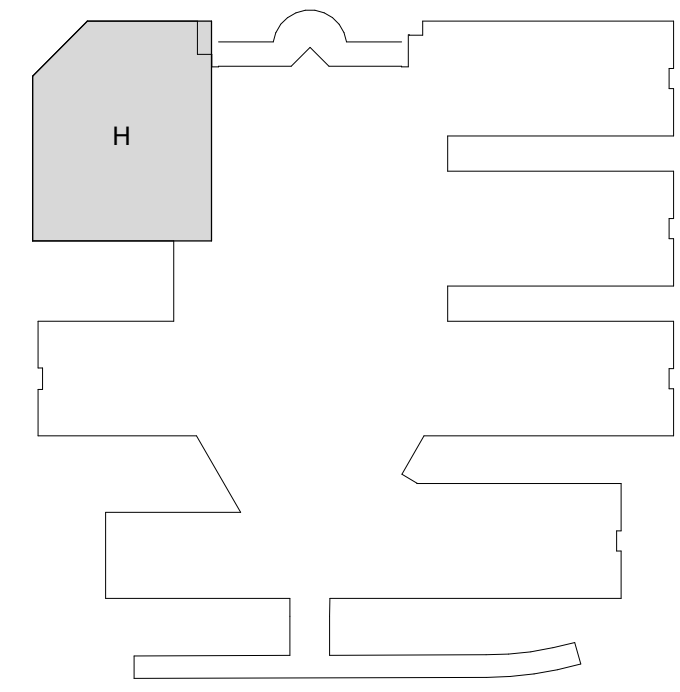
ISSUE DATES

DATE	FOR CONSTRUCTION - BID PACK #2
03/16/20	ISSUED FOR:

DRAWN: HB
CHECKED: MP
APPROVED: AN

PROJECT NO.
19040

DRAWING NO.
S2.5-BP2



1. TOP OF STEEL REFERENCE ELEVATION (DECK BEARING ELEVATION) REFER TO PLAN.

2. DESIGNATIONS:

RD-1: 1 1/2"-20 GAGE MIN. TYPE "B" WIDE RIB GALVANIZED STEEL ROOF DECK (MIN. 3 SPAN CONT.)
REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS.
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON $F_y = 33$ KSI:
DESIGN THICKNESS = 0.038" (UNFOCATED)
I (POSITIVE) = 0.201 IN⁴ /FT.
I (NEGATIVE) = 0.222 IN⁴ /FT.
S (POSITIVE) = 0.234 IN³ /FT.
S (NEGATIVE) = 0.247 IN³ /FT.

RD-2: 1 1/2"-18 GAGE MIN. TYPE "B" CELLULAR GALVANIZED STEEL ROOF DECK
REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS.
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON $F_y = 33$ KSI:
DESIGN THICKNESS = 0.047" (UNFOCATED)
I (POSITIVE) = 0.527 IN⁴ /FT.
I (NEGATIVE) = 0.527 IN⁴ /FT.
S (POSITIVE) = 0.498 IN³ /FT.
S (NEGATIVE) = 0.532 IN³ /FT.

BP-x BEARING PLATE, REFER TO SCHEDULE

L-x LINTEL, REFER TO SCHEDULE

3. ALL JOIST SEATS FOR K-SERIES JOISTS SHALL BE 2 1/2" DEEP, UNLESS NOTED OTHERWISE.
ALL JOIST SEATS FOR LH-SERIES JOISTS SHALL BE 5" DEEP, UNLESS NOTED OTHERWISE.
ALL JOIST GIRDER SEATS SHALL BE 7" DEEP, UNLESS NOTED OTHERWISE.

4. ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF. IN ADDITION TO OTHER LOAD CASES AND ANY OTHER NON-UNIFORM LOADS INDICATED ON THE DRAWINGS, ALL BRIDGING AND UPLIFT BRIDGING SHALL BE PER SJI REQUIREMENTS.

5. PROVIDE 3/4" CAP PLATE ON COLUMN FOR JOIST BEARING AS REQUIRED.

6. PROVIDE 1/4" CAP PLATE ON COLUMN FOR DECK BEARING AS REQUIRED.

7. COORDINATE SIZES AND LOCATION OF ALL ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. VERIFY SIZES, WEIGHTS, AND LOCATIONS OF ROOF TOP MECHANICAL EQUIPMENT WITH ARCHITECTURAL AND MEP DRAWINGS. REPORT ANY DISCREPANCIES WITH RESPECT TO SIZE AND WEIGHTS SHOWN ON THESE DRAWINGS TO THE ARCHITECT AND ENGINEER.

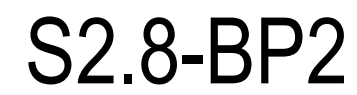
8. FRAMING FOR ALL ROOF DRAINS AND OVERFLOW DRAINS SHALL BE L5x3 1/2x5/16 LLV TYPICAL. ALL SIDES OF SUPPORTED EDGE, UNLESS NOTED OTHERWISE.

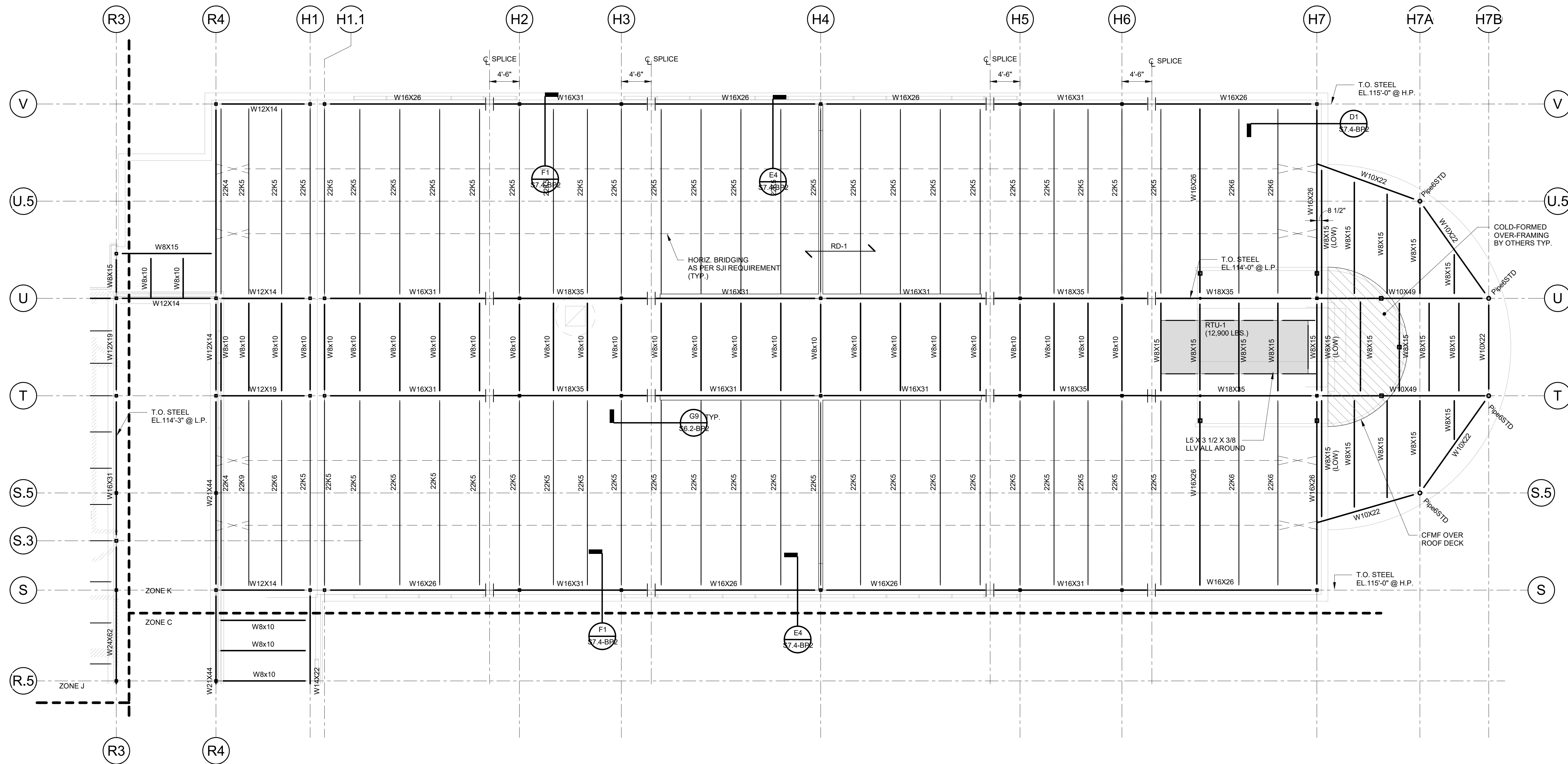
9. BEAM DESIGNATIONS:

$M = xx \text{ FT-K}$ — MOMENT AND SHEAR CONNECTION FORCES. REFER TO CONNECTION SCHEDULE AND/OR ELEVATIONS.
 $V = xx \text{ K}$
 $C=x''$ — (IF SHOWN) INDICATES CAMBER REQUIRED AT BEAM MID-SPAN
(IF SHOWN) INDICATES MOMENT CONNECTIONS

10. REFERENCE DRAWINGS:

S0.1 & S0.2	GENERAL STRUCTURAL NOTES
S0.3	SPECIAL INSPECTION SCHEDULES
S4.1	CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1	MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2	TYPICAL STEEL DETAILS
S7.1 & S7.2	SECTIONS & DETAILS





LOW ROOF FRAMING PLAN - ZONE K
1/8" = 1'-0"

ROOF FRAMING NOTES:

1. TOP OF STEEL REFERENCE ELEVATION (DECK BEARING ELEVATION) REFER TO PLAN.

DESIGNATIONS:

RD-1: 1 1/2"-20 GAGE MIN. TYPE "B" WIDE RIB GALVANIZED STEEL ROOF DECK (MIN. 3 SPAN CONT.)
REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI:
DESIGN THICKNESS = 0.0388" (UNCOATED)
I (POSITIVE) = 0.201 IN⁴ /FT.
I (NEGATIVE) = 0.222 IN⁴ /FT.
S (POSITIVE) = 0.234 IN³ /FT.
S (NEGATIVE) = 0.247 IN³ /FT.

RD-2: 1 1/2"-18 GAGE MIN. TYPE "B" CELLULAR GALVANIZED STEEL ROOF DECK
REFER TO DRAWING S6.2 FOR ATTACHMENT DETAILS
MINIMUM DECK SECTION PROPERTIES FOR DECK BASED ON Fy = 33 KSI:
DESIGN THICKNESS = 0.0474" (UNCOATED)
I (POSITIVE) = 0.527 IN⁴ /FT.
I (NEGATIVE) = 0.527 IN⁴ /FT.
S (POSITIVE) = 0.458 IN³ /FT.
S (NEGATIVE) = 0.532 IN³ /FT.

BP-x BEARING PLATE, REFER TO SCHEDULE

L-x LINTEL, REFER TO SCHEDULE

3. ALL JOIST SEATS FOR K-SERIES JOISTS SHALL BE 2 1/2" DEEP, UNLESS NOTED OTHERWISE.
ALL JOIST SEATS FOR LH-SERIES JOISTS SHALL BE 5" DEEP, UNLESS NOTED OTHERWISE.
ALL JOIST GIRDER SEATS SHALL BE 7" DEEP, UNLESS NOTED OTHERWISE.

4. ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF, IN ADDITION TO OTHER
LOAD CASES AND ANY OTHER NON-UNIFORM LOADS INDICATED ON THE DRAWINGS.
ALL BRIDGING AND UPLIFT BRIDGING SHALL BE PER SJI REQUIREMENTS.

5. PROVIDE 3/4" CAP PLATE ON COLUMN FOR JOIST BEARING AS REQUIRED.

6. PROVIDE 1/4" CAP PLATE ON COLUMN FOR DECK BEARING AS REQUIRED.

7. COORDINATE SIZES AND LOCATION OF ALL ROOF OPENINGS WITH ARCHITECTURAL
AND MEP DRAWINGS. VERIFY SIZES, WEIGHTS, AND LOCATIONS OF ROOF TOP
MECHANICAL EQUIPMENT WITH ARCHITECTURAL AND MEP DRAWINGS. REPORT ANY
DISCREPANCIES WITH RESPECT TO SIZE AND WEIGHTS SHOWN ON THESE DRAWINGS
TO THE ARCHITECT AND ENGINEER.

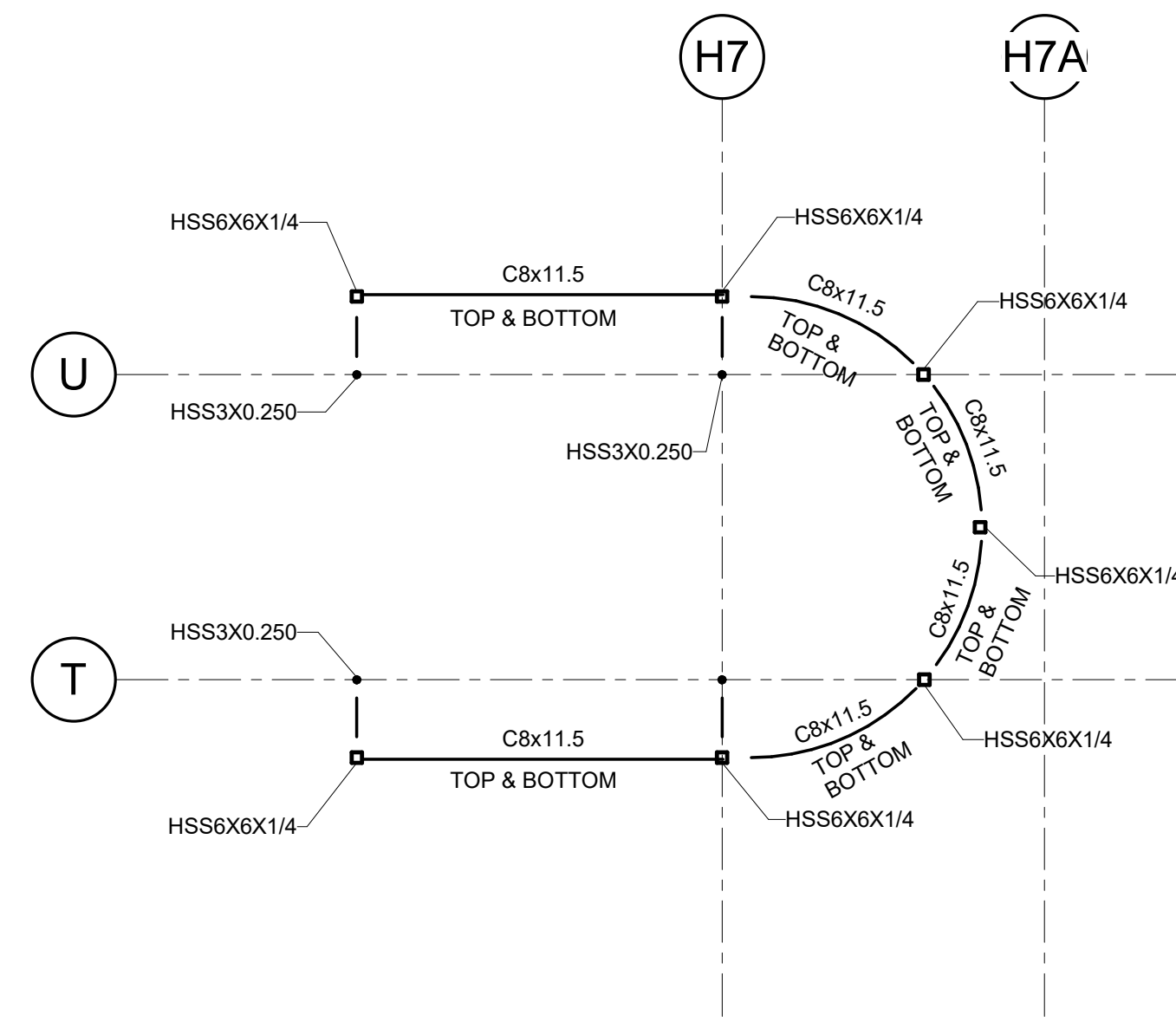
8. FRAMING FOR ALL ROOF DRAINS AND OVERFLOW DRAINS SHALL BE L5X3 1/2X5/16 LLV
TYPICAL, ALL SIDES OF SUPPORTED EDGE, UNLESS NOTED OTHERWISE.

BEAM DESIGNATIONS:

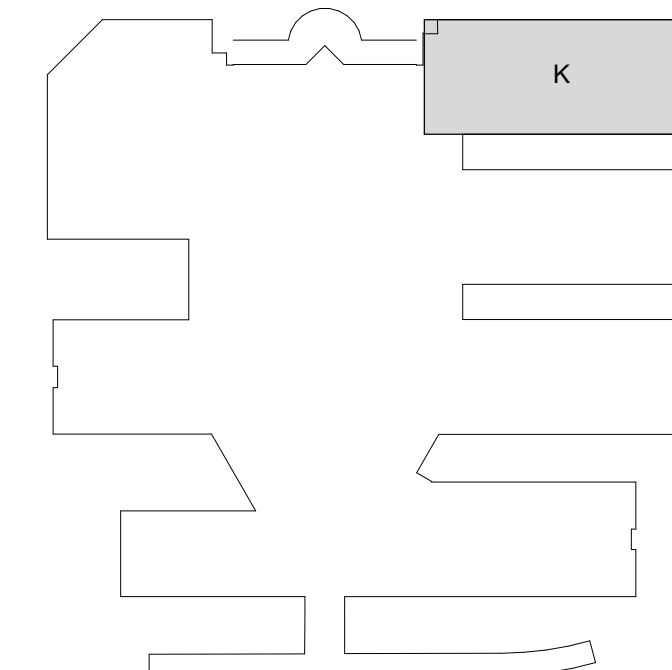
BEAM SIZE
M = xx FT-K
V = xx K
C=xx"
(IF SHOWN) INDICATES CAMBER
REQUIRED AT BEAM MID-SPAN

REFERENCE DRAWINGS:

S0.1 & S0.2 GENERAL STRUCTURAL NOTES
S0.3 SPECIAL INSPECTION SCHEDULES
S4.1 CONCRETE SCHEDULES AND TYPICAL DETAILS
S5.1 MASONRY SCHEDULES AND TYPICAL DETAILS
S6.1 & S6.2 TYPICAL STEEL DETAILS
S7.1 & S7.2 SECTIONS & DETAILS



SCREENWALL FRAMING PLAN
1/8" = 1'-0"



REGISTRATION SEAL



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PROJECT TITLE

New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

LOW ROOF FRAMING PLAN - ZONE K

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:

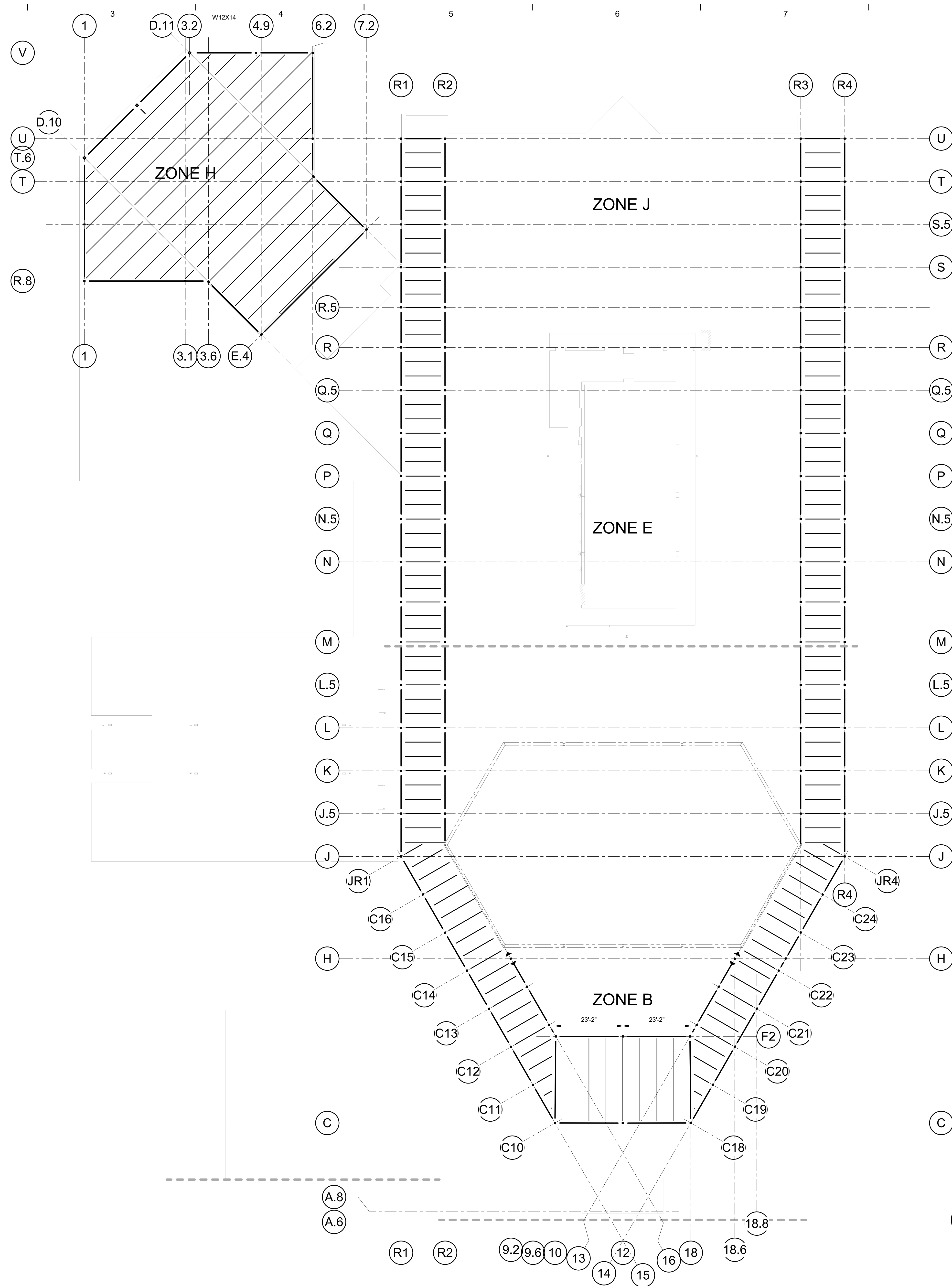
DRAWN: HB
CHECKED: WP
APPROVED: AK

PROJECT NO.

19040

DRAWING NO.

S2.10-BP2



HIGH ROOF FRAMING PLAN
COMPOSITE
1/16" = 1'-0"



REGISTRATION SEAL



CONSULTANT



CONSULTING ENGINEERS

6765 Daly Road
West Bloomfield, MI 48322
T/ 248.932.2010 • F/ 248.932.3088
info@desainasr.com

PROJECT TITLE

**New High Point
School**

**Washtenaw
Intermediate
School District**

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

**HIGH ROOF FRAMING
PLAN COMPOSITE**

ISSUE DATES

03/16/20 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:

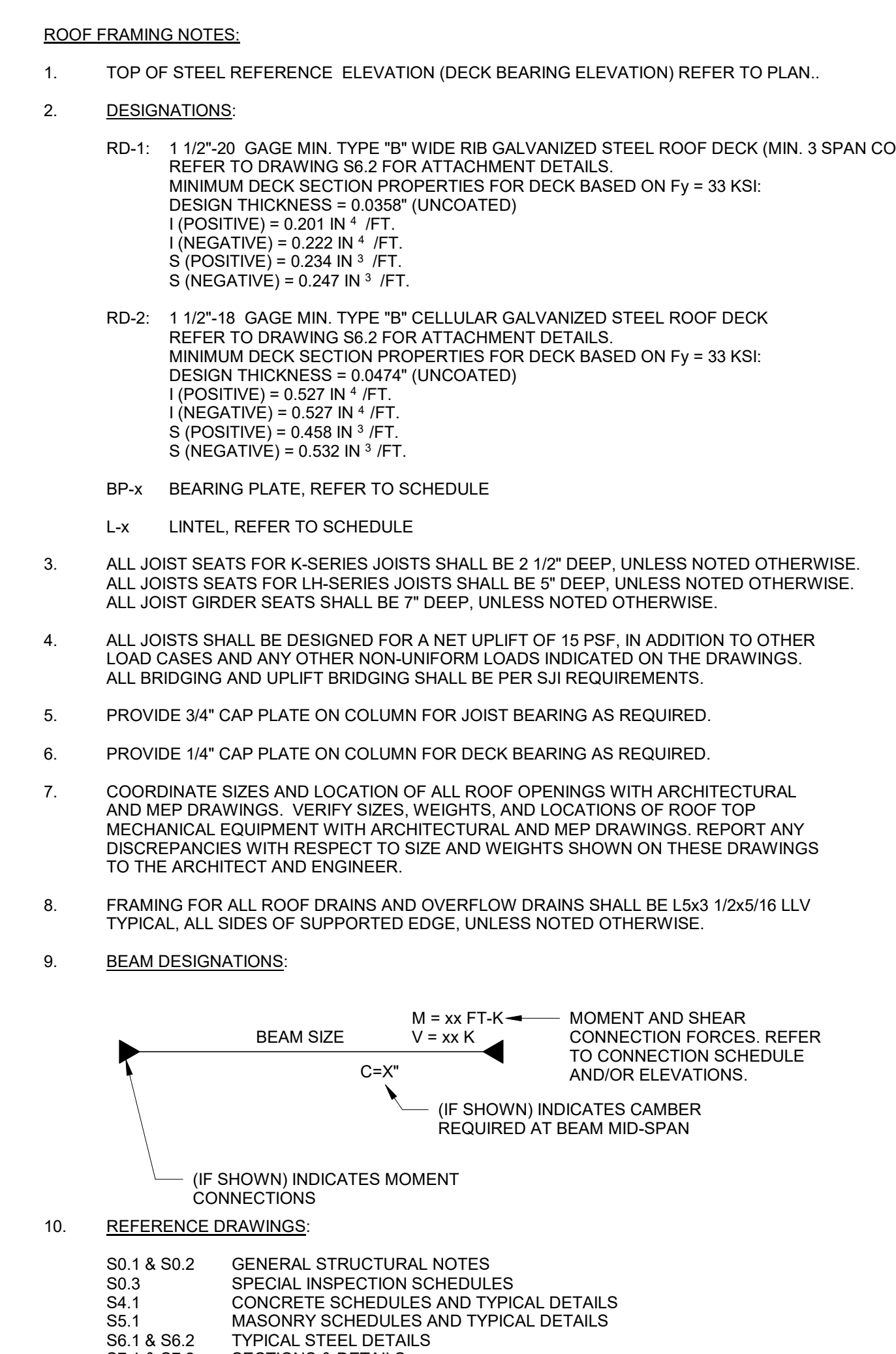
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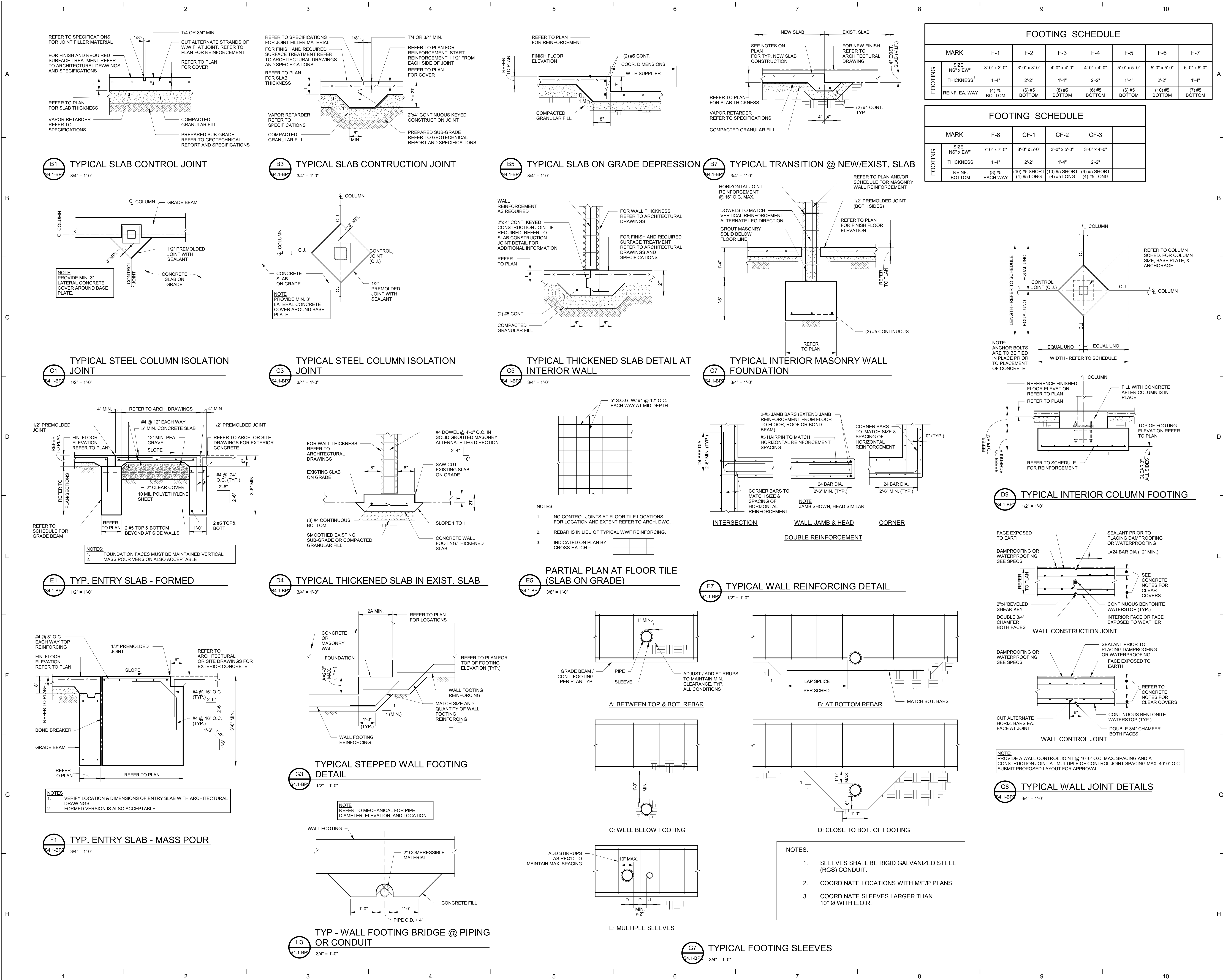
PROJECT NO.

19040

DRAWING NO.

S3.0-BP2

[illegible]



SPECIAL JOIST SCHEDULE			
MARK	SIZE	LOADING DIAGRAM	REMARKS
A	16K - SJ1		MECH. UNIT LOAD P1=2300 LBS
	16K - SJ2		MECH. UNIT LOAD P1=1000 LBS
	16K - SJ3		MECH. UNIT LOAD P1=600 LBS
B	16K - SJ4		MECH. UNIT LOAD P1=1000 LBS
	16K - SJ5		MECH. UNIT LOAD P1=1700 LBS
	16K - SJ6		MECH. UNIT LOAD P1=1100 LBS
C	22K - SJ1		MECH. UNIT LOAD P1=1000 LBS
	22K - SJ2		MECH. UNIT LOAD P1=1000 LBS
	22K - SJ3		MECH. UNIT LOAD P1=1650 LBS
D	22K - SJ4		MECH. UNIT LOAD P1=600 LBS
	22K - SJ5		MECH. UNIT LOAD P1=2700 LBS
	22K - SJ6		MECH. UNIT LOAD P1=400 LBS
E	22K - SJ7		MECH. UNIT LOAD P1=2500 LBS
	22K - SJ8		MECH. UNIT LOAD P1=100 LBS P2=600 LBS
	22K - SJ9		MECH. UNIT LOAD P1=450 LBS P2=2700 LBS
F	26K - SJ1		MECH. UNIT LOAD P1=200 LBS
	26K - SJ2		MECH. UNIT LOAD P1=650 LBS
	26K - SJ3		MECH. UNIT LOAD P1=1000 LBS
G	26K - SJ4		MECH. UNIT LOAD P1=1600 LBS

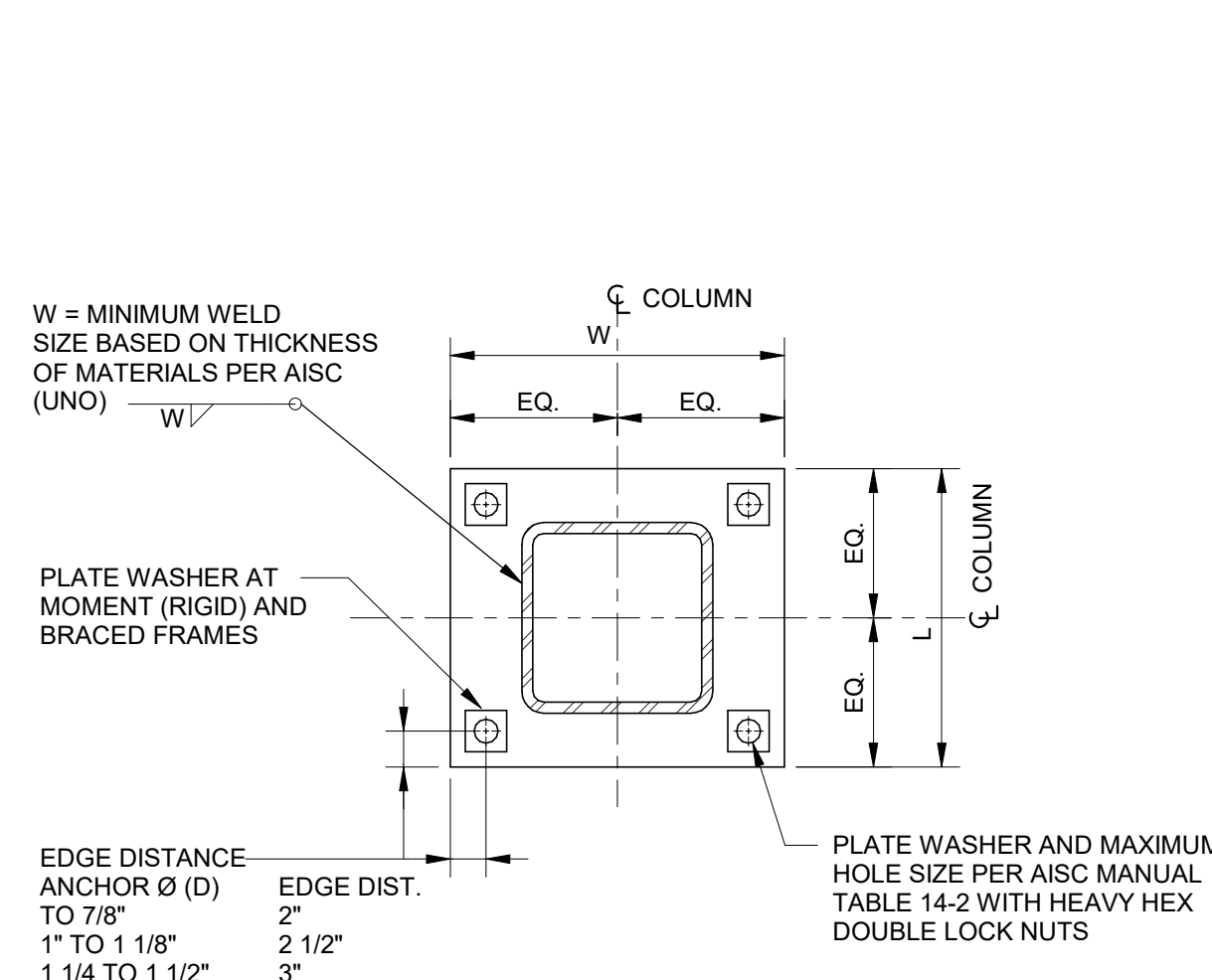
SPECIAL JOIST SCHEDULE			
MARK	SIZE	LOADING DIAGRAM	REMARKS
26K - SJ5			MECH. UNIT LOAD $P_1=1800$ LBS
28K - SJ1			MECH. UNIT LOAD $P_1=600$ LBS
28K - SJ2			MECH. UNIT LOAD $P_1=1100$ LBS
28K - SJ3			MECH. UNIT LOAD $P_1=2100$ LBS
28K - SJ4			MECH. UNIT LOAD $P_1=400$ LBS
28K - SJ5			MECH. UNIT LOAD $P_1=2000$ LBS
28K - SJ6			MECH. UNIT LOAD $P_1=4250$ LBS
28K - SJ7			MECH. UNIT LOAD $P_1=5800$ LBS
28K - SJ8			MECH. UNIT LOAD $P_1=1000$ LBS
28K - SJ9			MECH. UNIT LOAD $P_1=100$ LBS

[illegible]

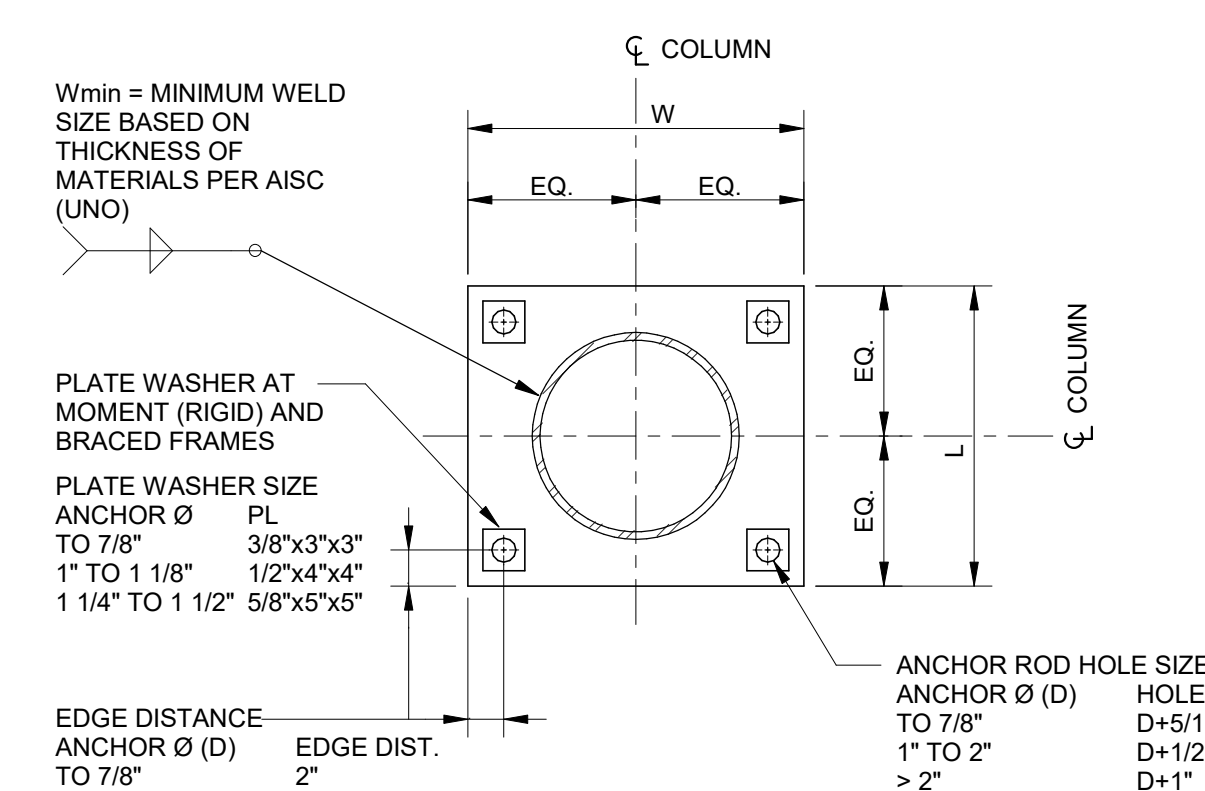
STEEL COLUMN SCHEDULE												
MARK		C-13	C-14	C-15	C-16	C-17	C-18	C-19	C-20	C-21		
SUPPORTING												
REF. FIN. T.O.S. AUDITORIUM												
REF. FIN. T.O.S. MONITOR												
REF. FIN. T.O.S. LOW ROOF												
REF. FIN. 1ST. FL.												
BASE PLATE	TYPE	A	A	A	A	A	A	A	A	A		
	SIZE L" x W" x T"	10" x 10" x 3/4"	10" x 10" x 3/4"	10" x 10" x 3/4"	10" x 10" x 3/4"	12" x 12" x 3/4"	12" x 12" x 1"	14" x 14" x 1"	12" x 12" x 1"	10" x 10" x 3/4"		
	ANCHOR BOLTS	(4) 3/4" DIA.	(4) 3/4" DIA.	(4) 3/4" DIA.	(4) 3/4" DIA.	(4) 3/4" DIA.	(4) 3/4" DIA.	(4) 1" DIA.	(4) 3/4" DIA.	(4) 3/4" DIA.		
	ANCHOR BOLT PROJECTION	7"	7"	7"	7"	7"	7"	7"	7"	7"		
REMARKS		12" EMBED.	12" EMBED.	12" EMBED.	12" EMBED.	12" EMBED.	12" EMBED.	16" EMBED.	12" EMBED.	12" EMBED.		

BEARING PLATE SCHEDULE					
MARK	BEARING PLATE SIZE (L" x W" x T")	ANCHOR BOLTS			REMARKS
		NO. & SIZE	EMBEDMENT LENGTH	PROJ.	
BP-1	7"x8"x3/8"		6"		
BP-2	7"x8"x1/2"		6"		
BP-3	7"x8"x5/8"		6"		
BP-4	8"x10"x1/2"		6"		
BP-5	8"x10"x5/8"		6"		

NOTE:
EDGE OF BEARING PLATE SHALL BE LOCATED NOT MORE THAN 1/2" FROM THE FACE OF THE WALL ON THE SIDE OF BEAM OR JOIST SPAN.

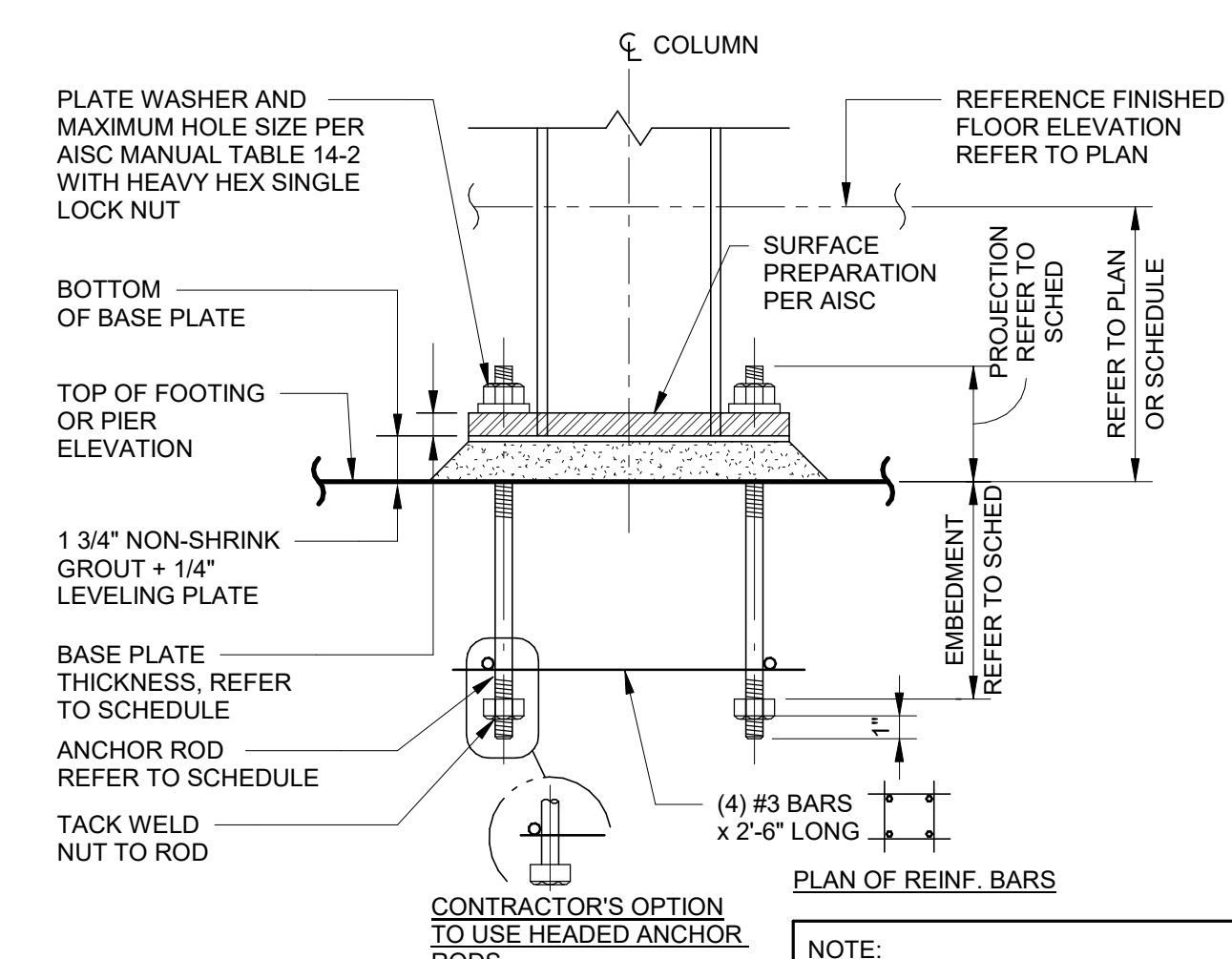


D9 TYPICAL HSS TUBE COLUMN BASE PLATE



1" TO 1 1/8" 2 1/2"
1 1/4 TO 1 1/2" 3"

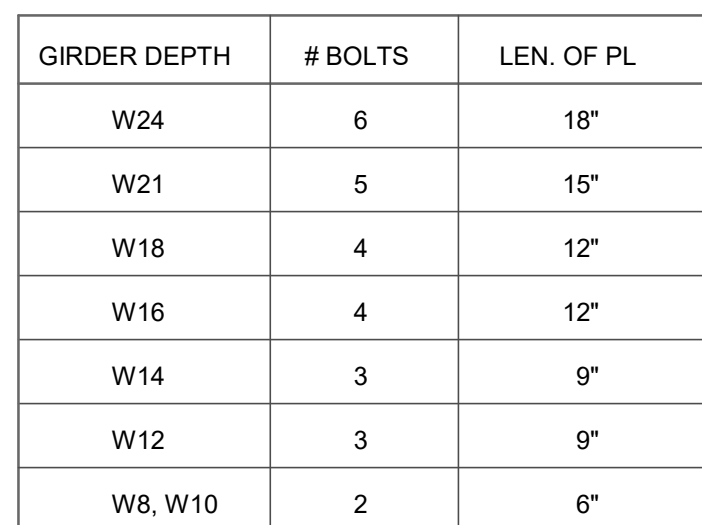
 TYPICAL HSS ROUND COLUMN BASE
PLATE



ANCHOR RODS MUST BE INSTALLED WITHIN AISC TOLERANCES BEFORE PLACEMENT OF CONCRETE

H9 ANCHOR ROD DETAIL WITH LEVELING PLATE - BASE PLATE TYPE "X"

A	<div style="text-align: center;"><p>TMP ARCHITECTURE INC.</p></div> <p>1191 WEST SQUARE LAKE ROAD BLOOMFIELD HILLS • MICHIGAN 48302 PH : 248.338.4561 FX : 248.338.0223 E/M : INFO@TMAP-ARCHITECTURE.COM</p>																				
B	<div style="text-align: center;">Mitchell Mouat architects</div> <p>1135 BLOOMFIELD RD., SUITE 100 BLOOMFIELD HILLS MI 48302 TEL: 248.363.2070 FAX: 248.363.2862 EMAIL: info@mitchellmouat.com</p>																				
C	<p>REGISTRATION SEAL</p> <div style="text-align: center;"></div> <p>CONSULTANT</p> <div style="text-align: center;"><p>DESAI / NASR CONSULTING ENGINEERS</p></div> <p>6765 Daily Road West Bloomfield, MI 48322 T/ 248.932.2010 • F/ 248.932.3088 info@desainasr.com</p>																				
D	<p>PROJECT TITLE</p> <p>New High Point School Washtenaw Intermediate School District 1735 South Wagner Road Ann Arbor, Michigan</p>																				
E	<p>DRAWING TITLE</p> <p>TYPICAL STEEL DETAILS</p>																				
F	<p>ISSUE DATES</p> <table border="1"><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr><tr><td> </td><td> </td></tr></table>																				
G	<table border="1"><tr><td>03/16/20</td><td>FOR CONSTRUCTION - BID PACK #2</td></tr><tr><td>DATE:</td><td>ISSUED FOR:</td></tr><tr><td>DRAWN</td><td>N/R</td></tr><tr><td>CHECKED</td><td>W/P</td></tr><tr><td>APPROVED</td><td>A/N</td></tr></table>	03/16/20	FOR CONSTRUCTION - BID PACK #2	DATE:	ISSUED FOR:	DRAWN	N/R	CHECKED	W/P	APPROVED	A/N										
03/16/20	FOR CONSTRUCTION - BID PACK #2																				
DATE:	ISSUED FOR:																				
DRAWN	N/R																				
CHECKED	W/P																				
APPROVED	A/N																				
H	<p>PROJECT NO.</p> <p>19040</p> <p>DRAWING NO.</p> <p>S6.1-BP2</p>																				



NOTES:

1. THESE DETAILS LIMITED TO JOIST & BEAMS WITH $t_w \leq 7/16"$
2. SEE 14/S 003 FOR GIRDER CONNECTIONS TO HSS COLUMNS
3. SEE 14/S 401 FOR HIGH-LOAD GIRDER CONNECTIONS TO HSS COLUMNS WHERE NOTED ON PLAN
4. ALL BOLTS $3/4" \text{ } \phi \text{ } A325N \text{ U.O.N.}$
5. SHEAR PLATE THICKNESS "t" TO BE t_w OR $5/16" \text{ MIN.}, 7/16" \text{ MAX.}$



CONSULTANT

DESAI / NASR

CONSULTING ENGINEERS

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PROJECT TITLE

New High Point School

Washtenaw
Intermediate
School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
TYPICAL STEEL
DETAILS

ISSUE DATES

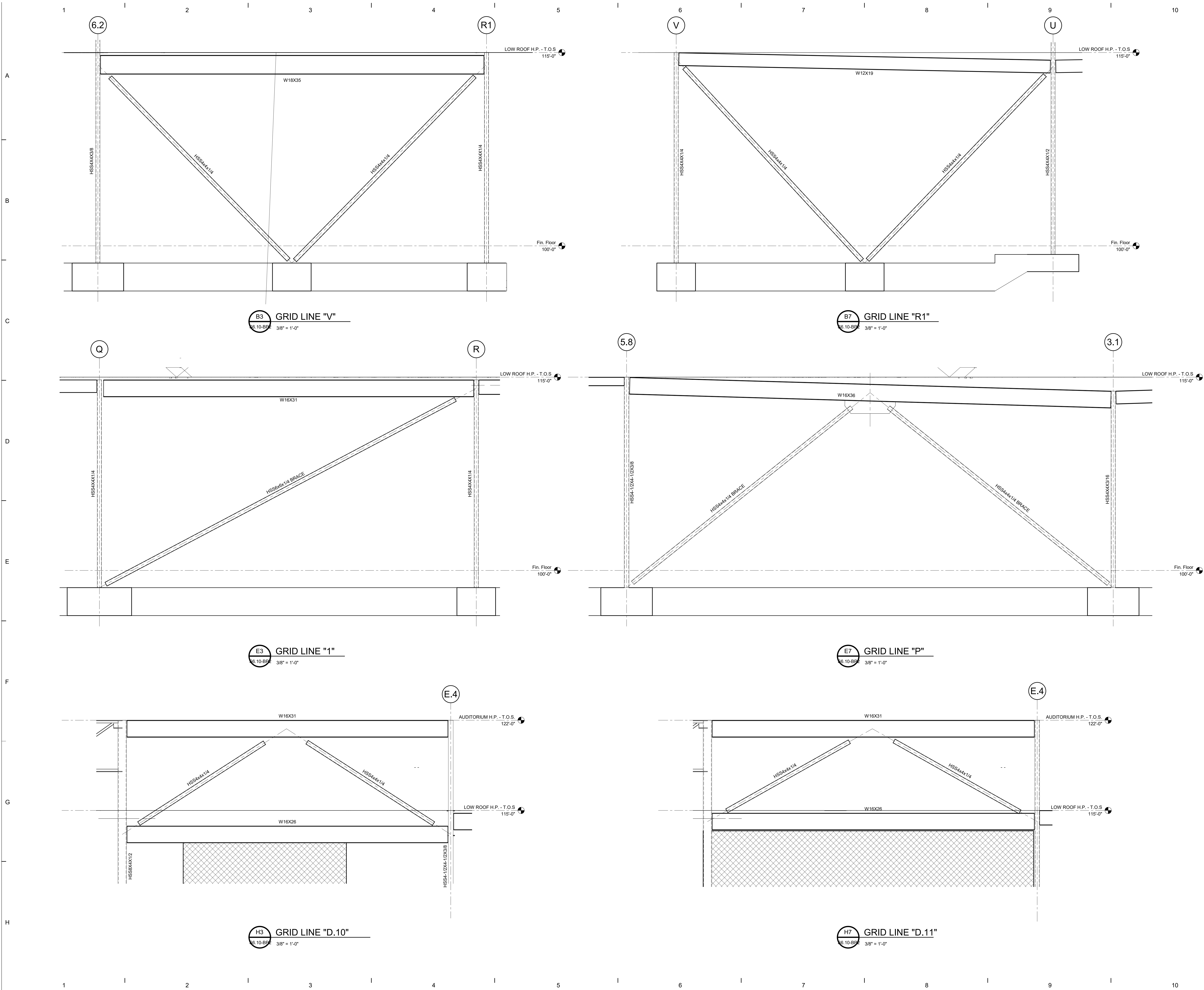
03/16/20	FOR CONSTRUCTION - BID PACK #2
DATE:	ISSUED FOR:

PROJECT NO.

19040

DRAWING NO

S6.2-BP2



ARCHITECTURE

TMP ARCHITECTURE INC

1191 WEST SQUARE LAKE ROAD

BLOOMFIELD HILLS - MICHIGAN 48302

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EM - INFO - TMP-ARCHITECTURE.COM

Mitchell

Mouat

architects

1122 2ND ST. 1ST FLOOR BLOOMFIELD HILLS, MI 48304

REGISTRATION SEAL

STATE OF MICHIGAN

JAYANT P. DESAI

ENGINEER

NO. 19102

LICENSED PROFESSIONAL ENGINEER

CONSULTANT

DESAI / NASR

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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road

Ann Arbor, Michigan

DRAWING TITLE

STEEL FRAME ELEVATIONS

ISSUE DATES

03/16/20

FOR CONSTRUCTION - BID PACK #2

DATE:

ISSUED FOR:

DRAWN

MB

CHECKED

WP

APPROVED

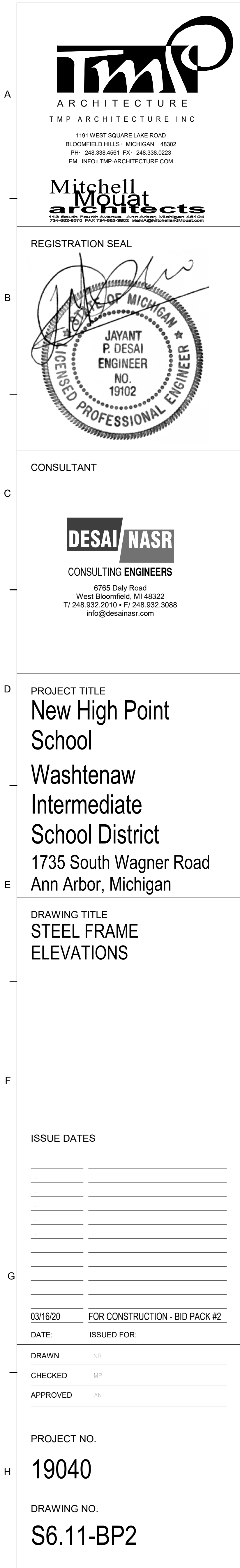
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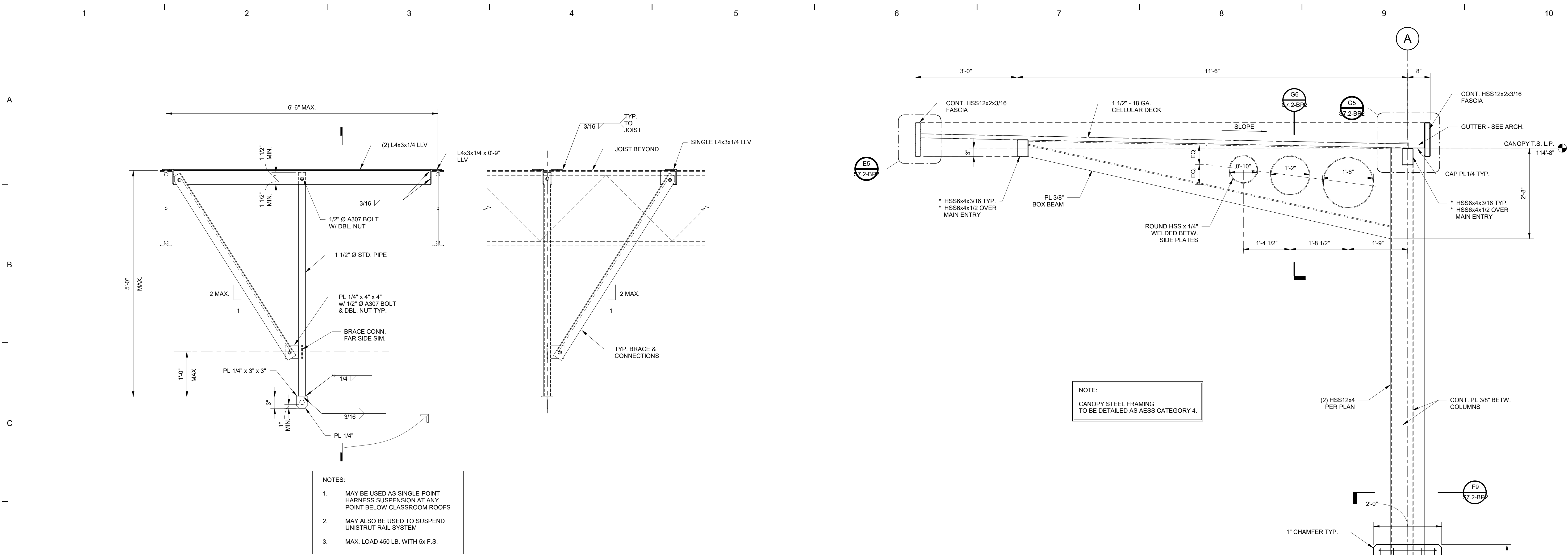
PROJECT NO.

19040

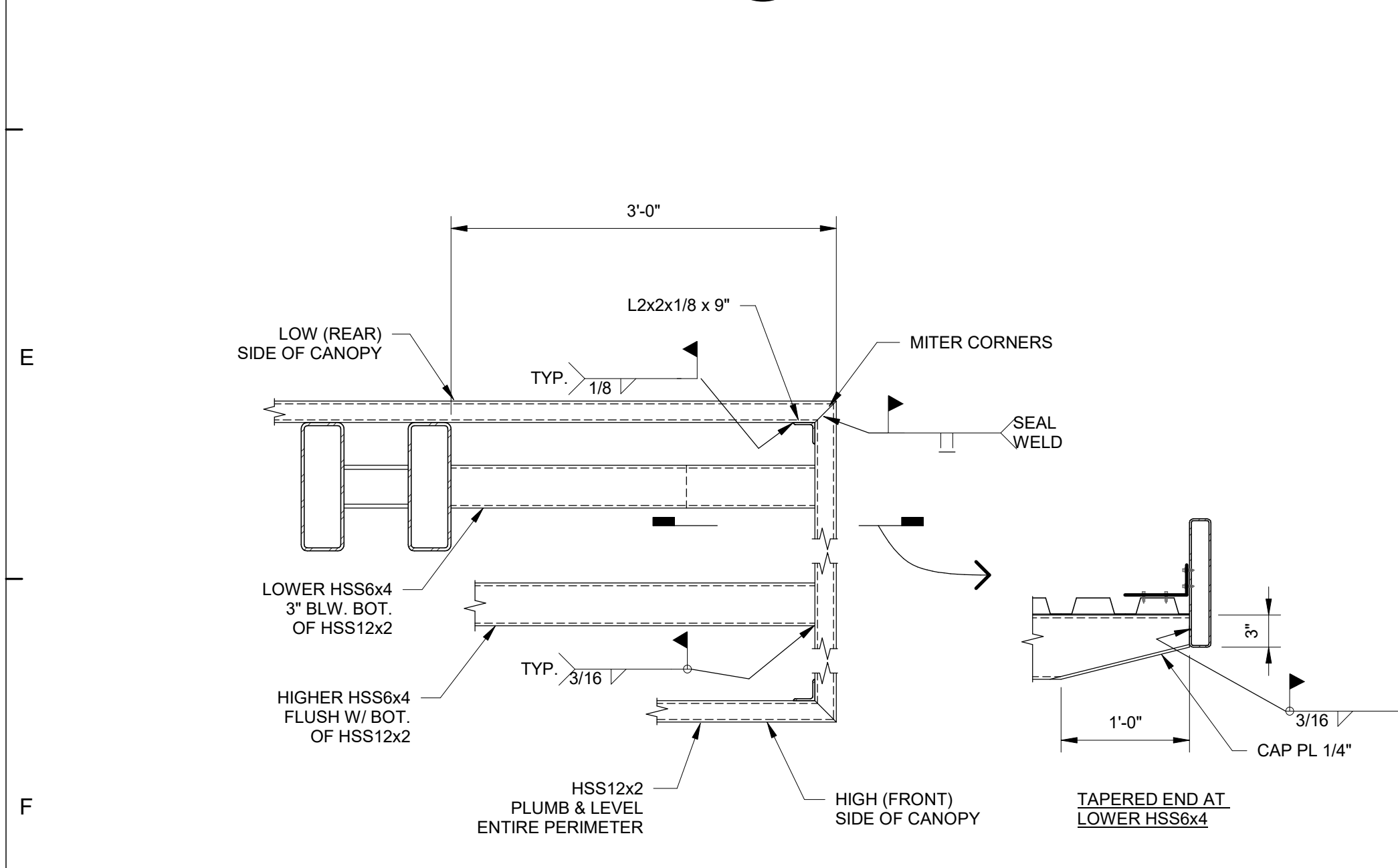
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S6.10-BP2

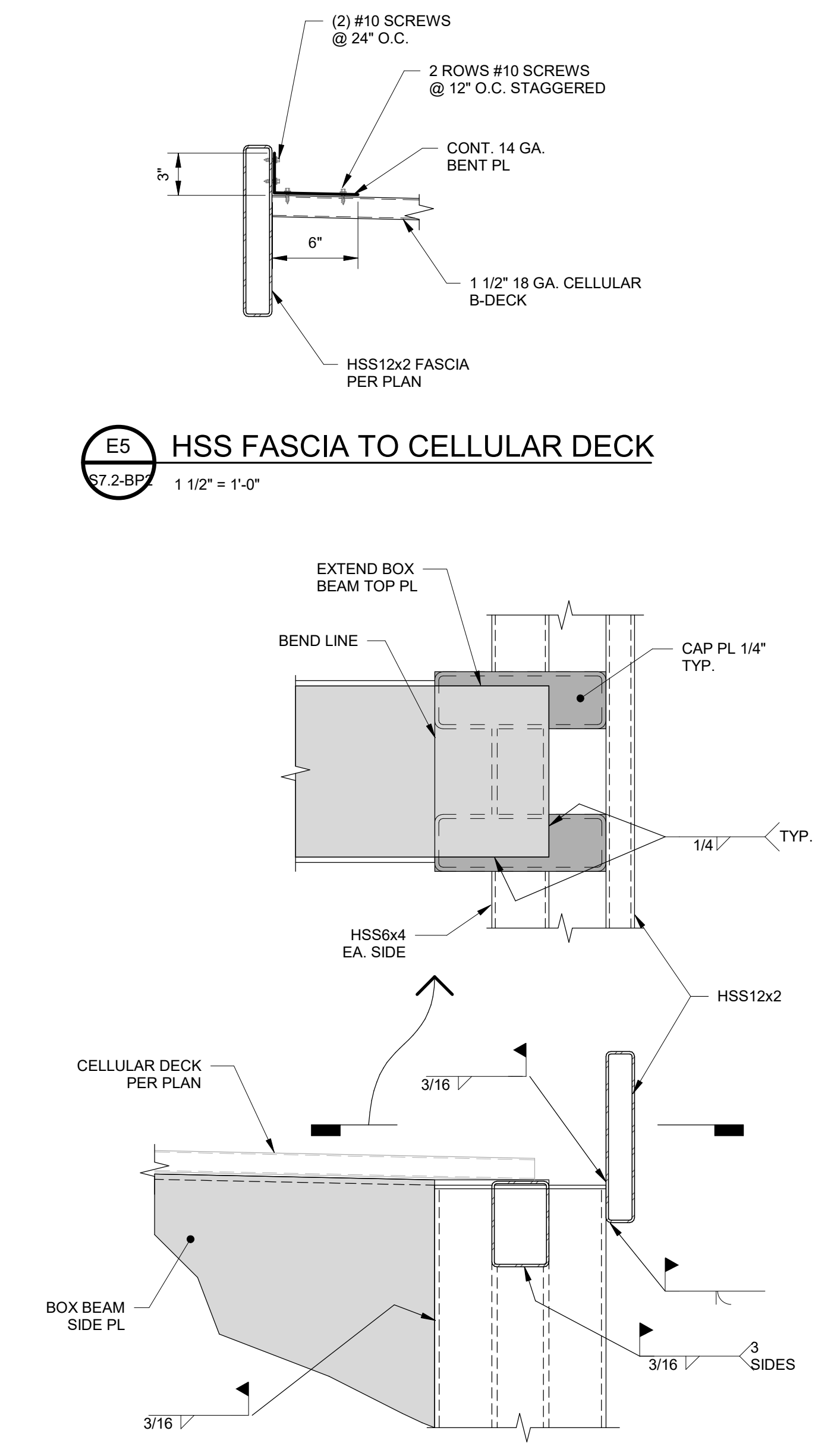
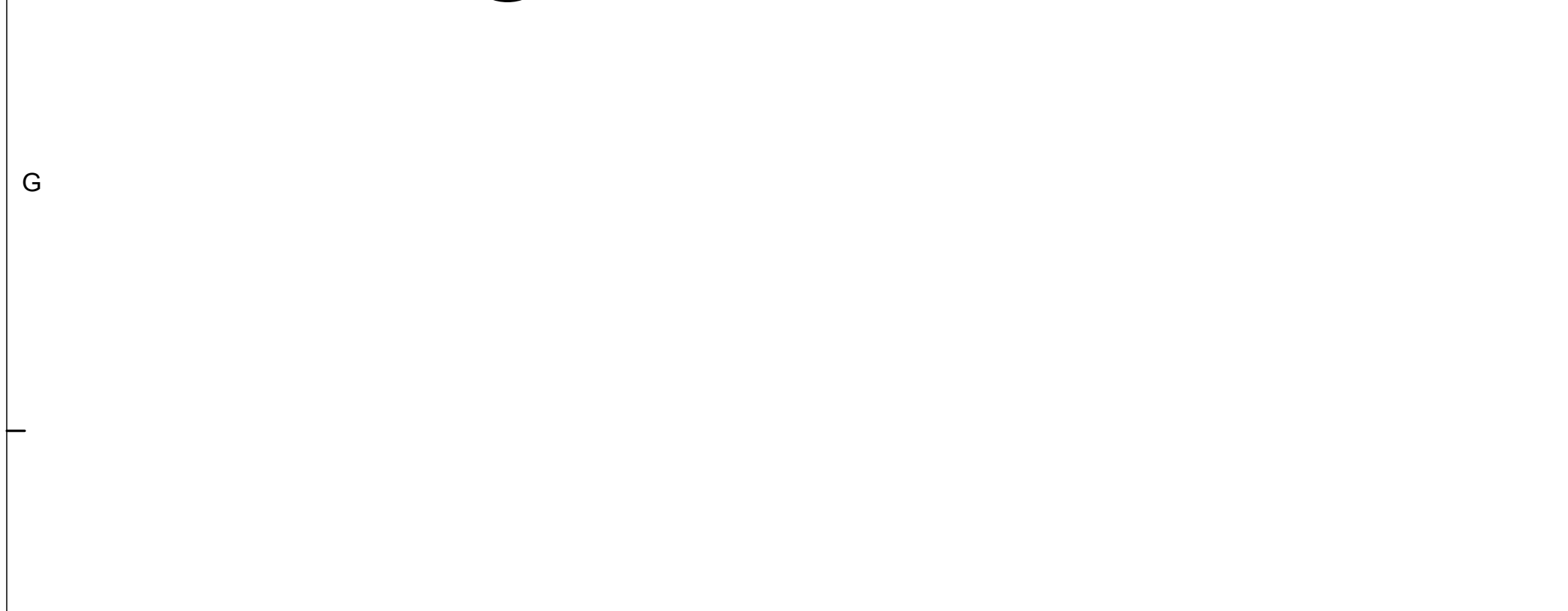




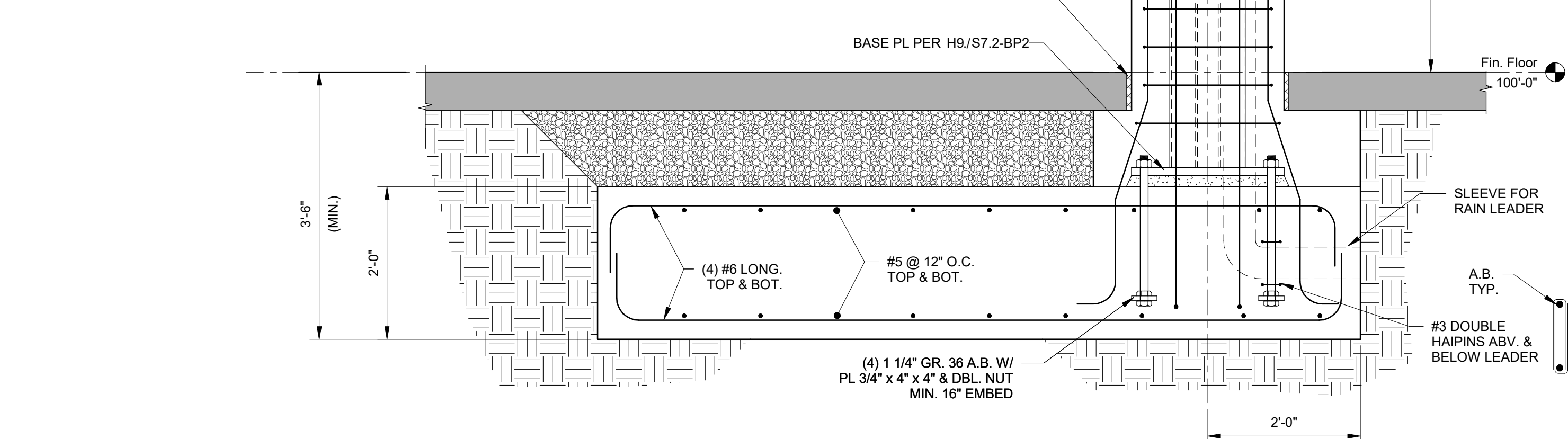
D2 HARNESS SUSPENSION SYSTEM
1" = 1'-0"



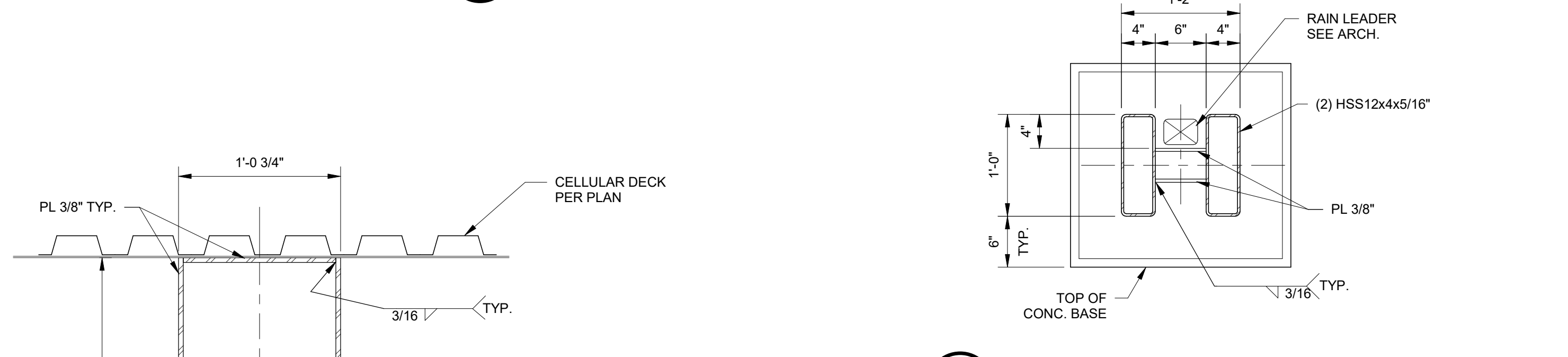
F2 CANOPY END DETAILS
1" = 1'-0"



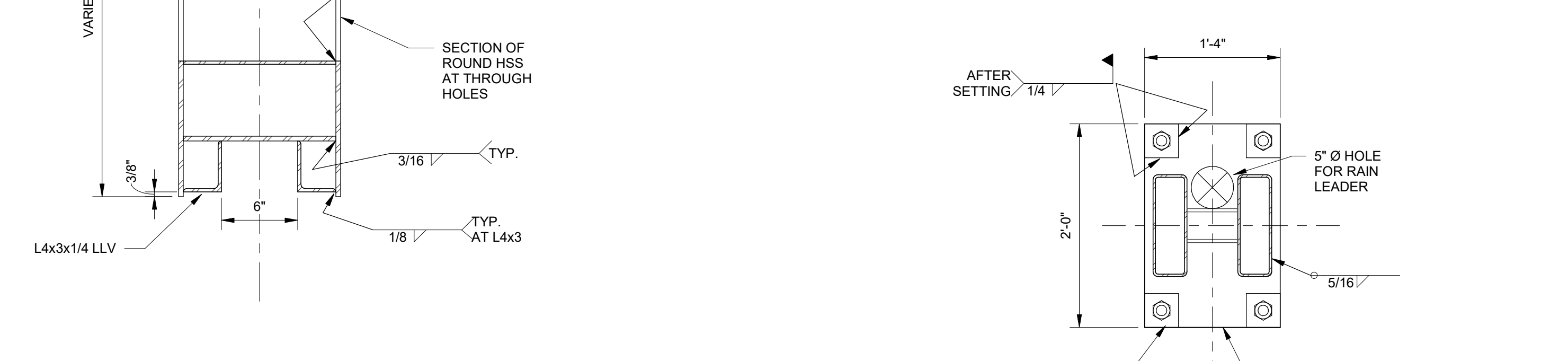
G5 CANOPY TOP CONNECTIONS
1 1/2" = 1'-0"



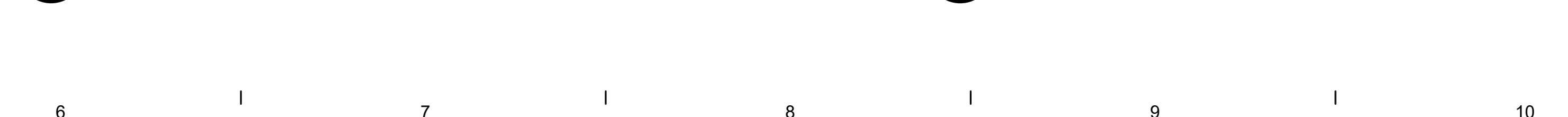
E7 TYPICAL CANOPY SECTION
3/4" = 1'-0"



F9 DROP OFF CANOPY COLUMN SECTION
1" = 1'-0"



H9 DROP OFF CANOPY BASE PLATE
1" = 1'-0"



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REGISTRATION SEAL

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PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road

Ann Arbor, Michigan

DRAWING TITLE

SECTIONS AND DETAILS

ISSUE DATES

NO.	DATE	DESCRIPTION

03/16/20 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN: H9

CHECKED: WP

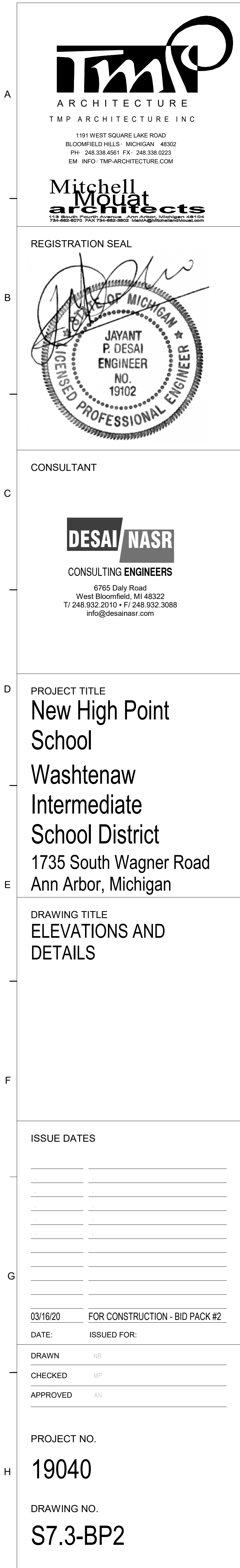
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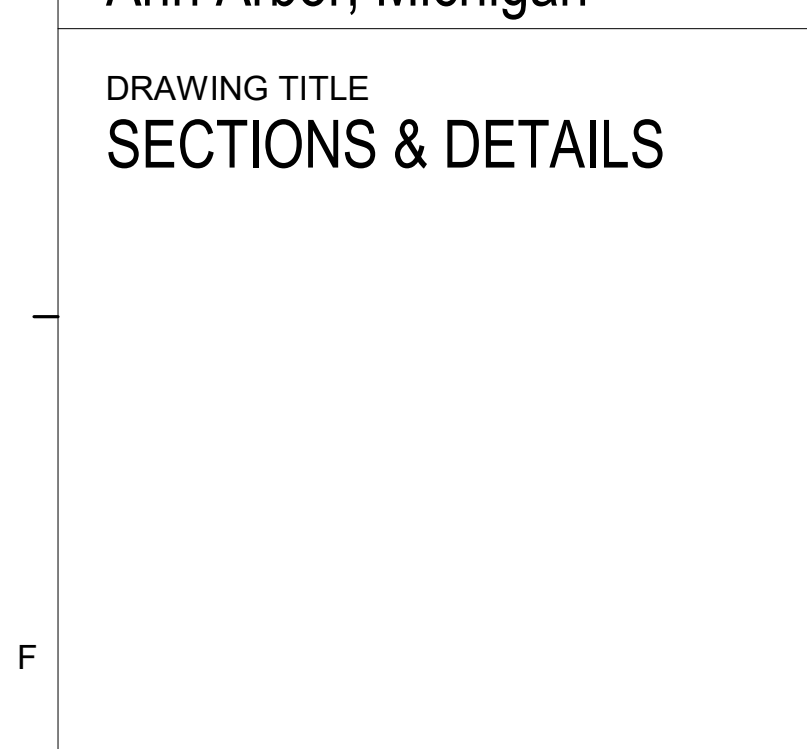
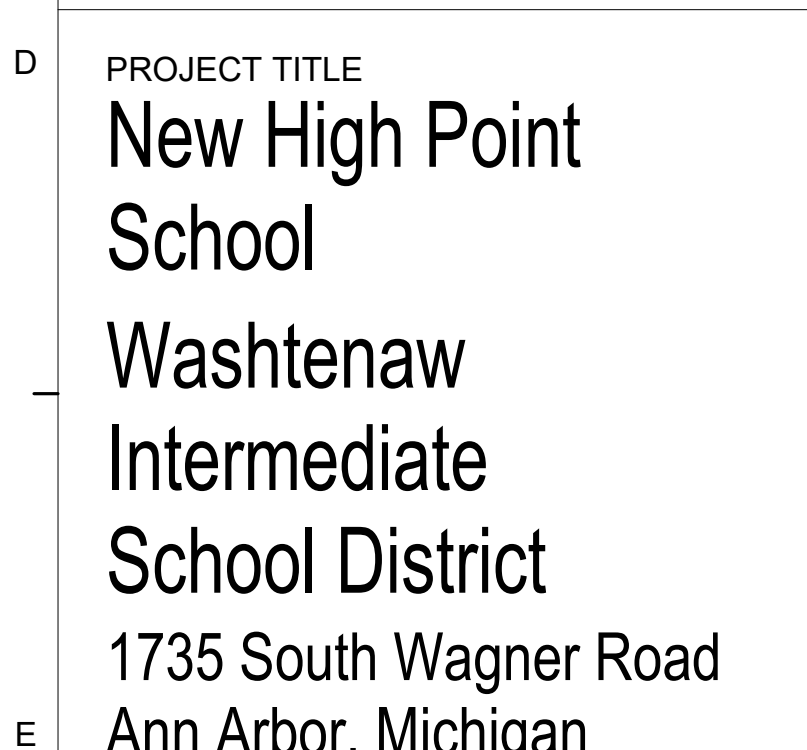
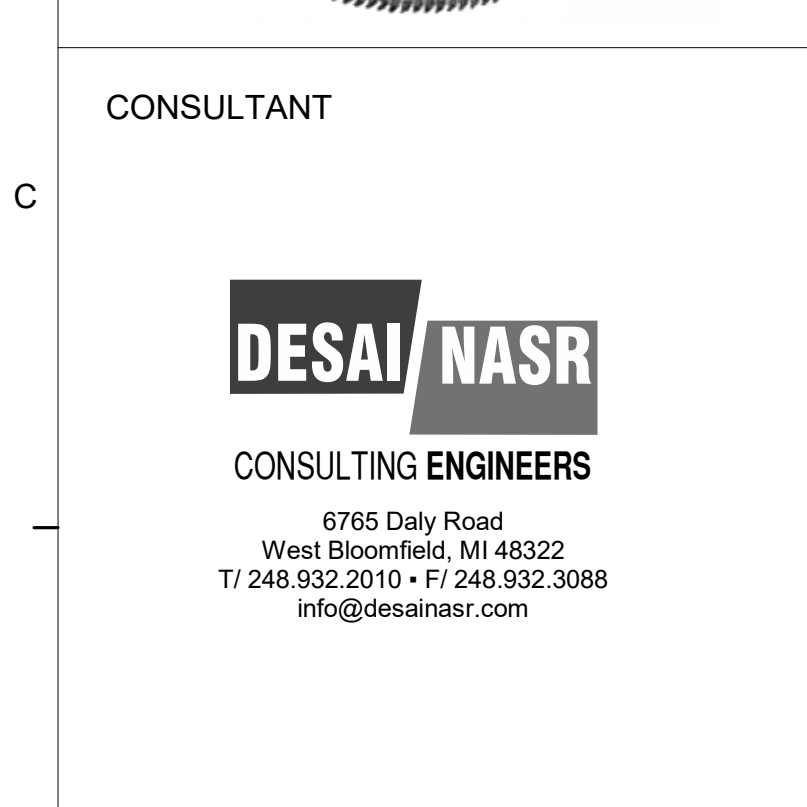
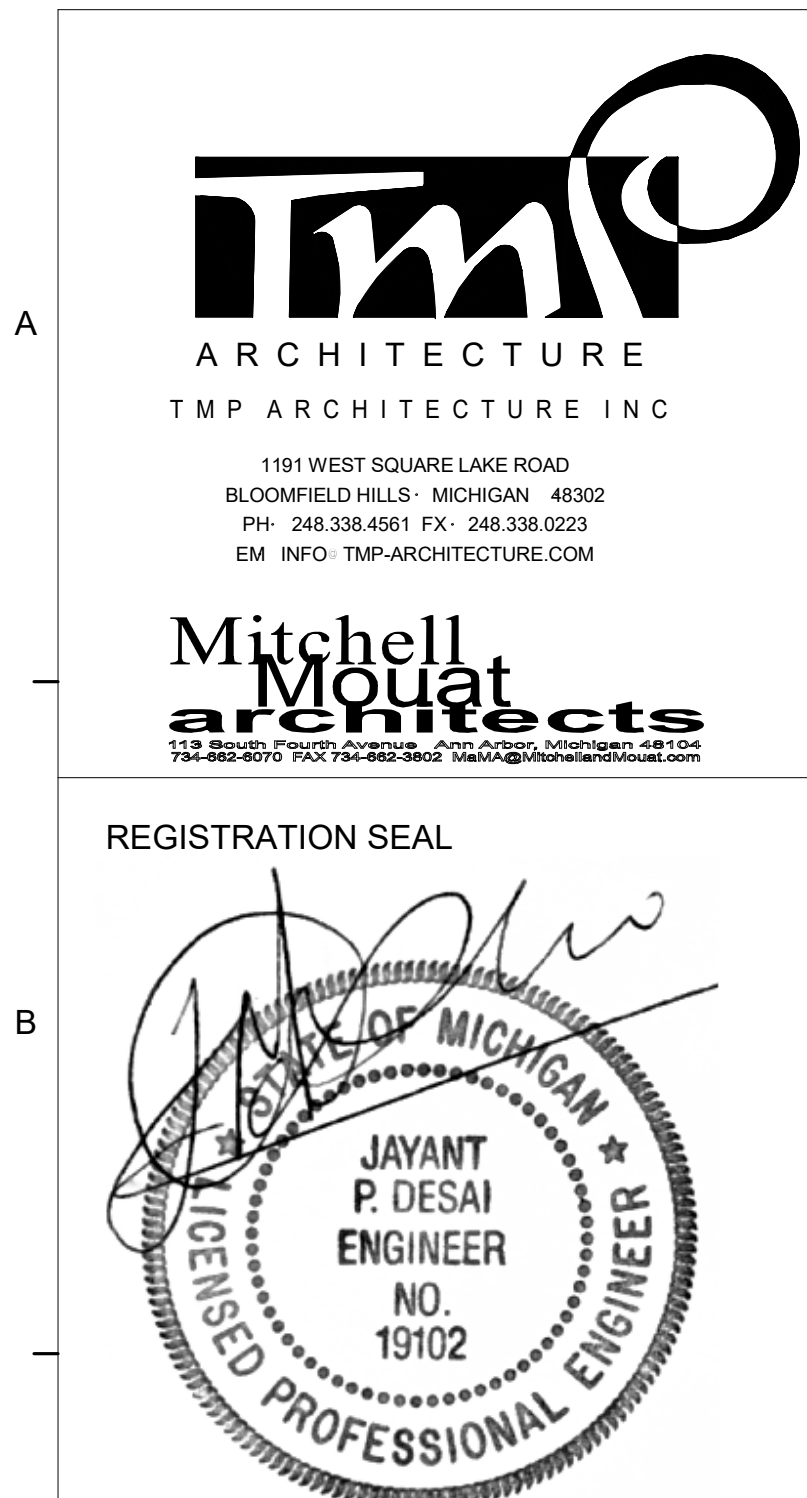
PROJECT NO.

19040

DRAWING NO.

S7.2-BP2





REGISTRATION SEAL

CONSULTANT

PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

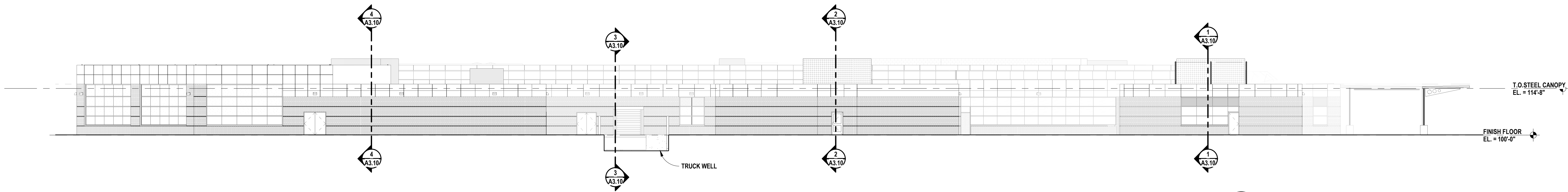
DRAWING TITLE
Composite Exterior Elevations

ISSUE DATES

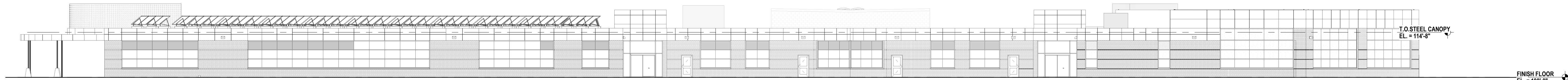
03/16/2020	FOR CONSTRUCTION - BID PACK #2
DATE:	ISSUED FOR:
DRAWN	Author
CHECKED	Checker
APPROVED	Approver

PROJECT NO.
19040
DRAWING NO.
A3.1-BP2

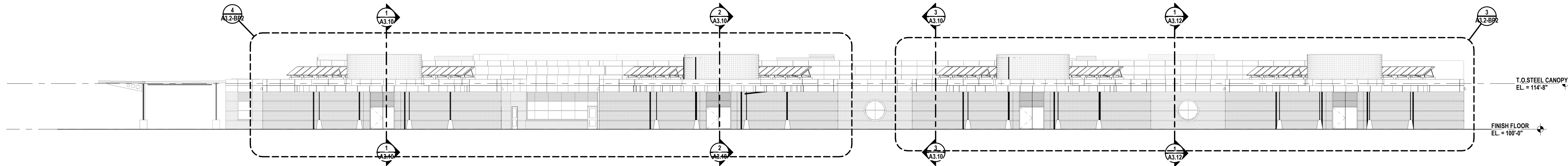
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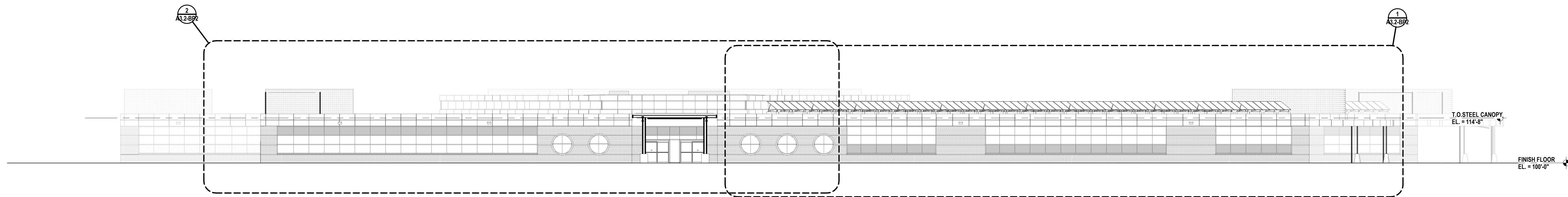
5 NORTHWEST ELEVATION - SERVICES
SCALE: 1/16" = 1'-0"



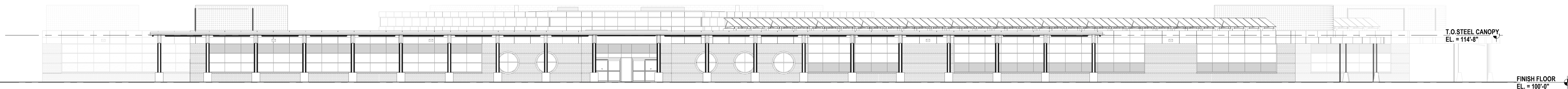
4 NORTHEAST ELEVATION - MEDIA CENTER
SCALE: 1/16" = 1'-0"



3 SOUTHEAST ELEVATION - HOUSES
SCALE: 1/16" = 1'-0"



2 SOUTHWEST ELEVATION - MAIN ENTRANCE (WITHOUT CANOPY)
SCALE: 1/16" = 1'-0"



1 SOUTHWEST ELEVATION - MAIN ENTRANCE (WITH CANOPY)
SCALE: 1/16" = 1'-0"

REGISTRATION SEAL

CONSULTANT

PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
Exterior Elevations

ISSUE DATES

03/16/2020 FOR CONSTRUCTION - BID PACK #2
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DRAWN: Author
CHECKED: Checker
APPROVED: Approver

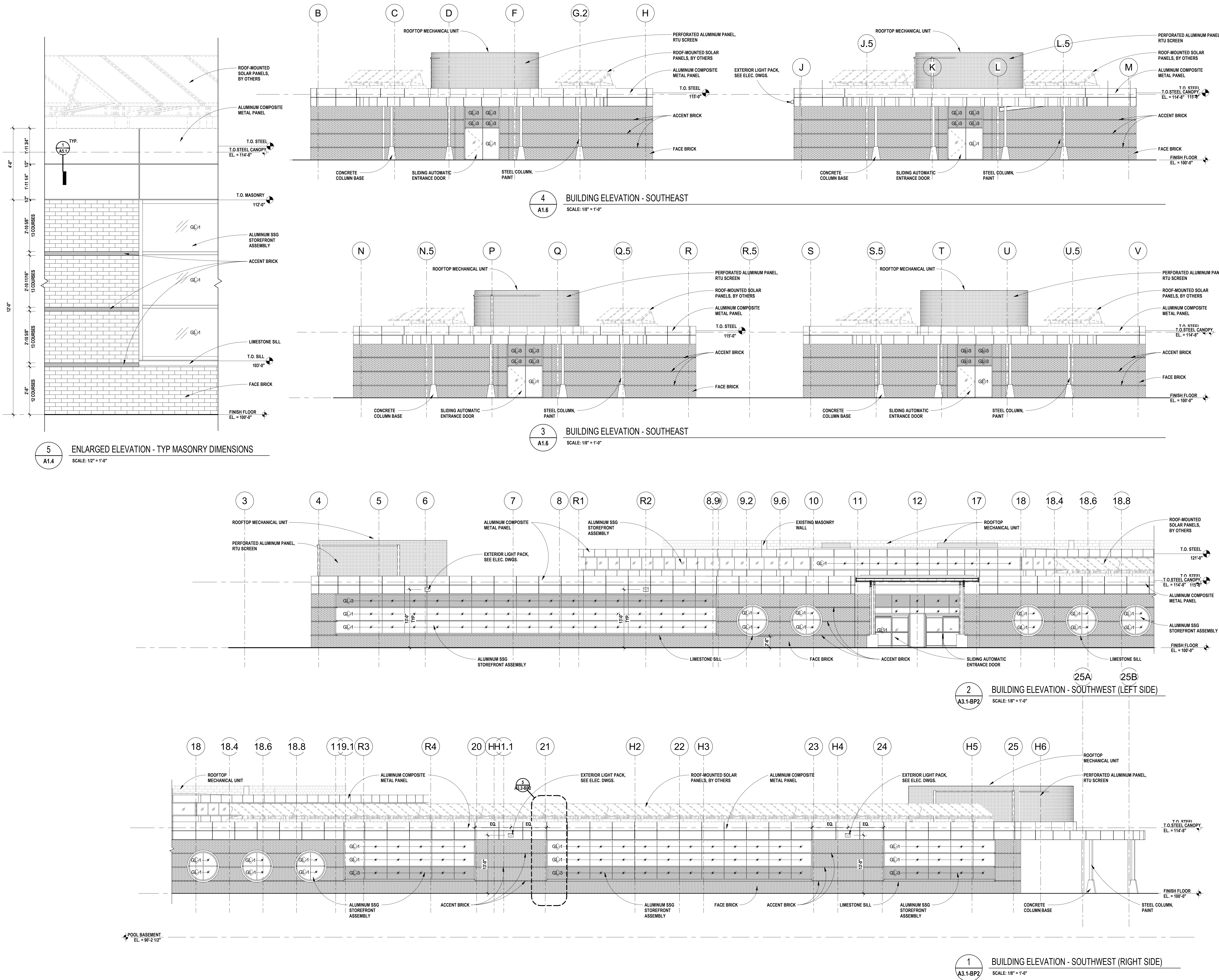
PROJECT NO.

19040

DRAWING NO.

A3.2-BP2

FOR REFERENCE ONLY



REGISTRATION SEAL

CONSULTANT

PROJECT TITLE

New High Point School

Washtenaw Intermediate School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

Exterior Elevations

ISSUE DATES

03/16/2020 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN: Author

CHECKED: Checker

APPROVED: Approver

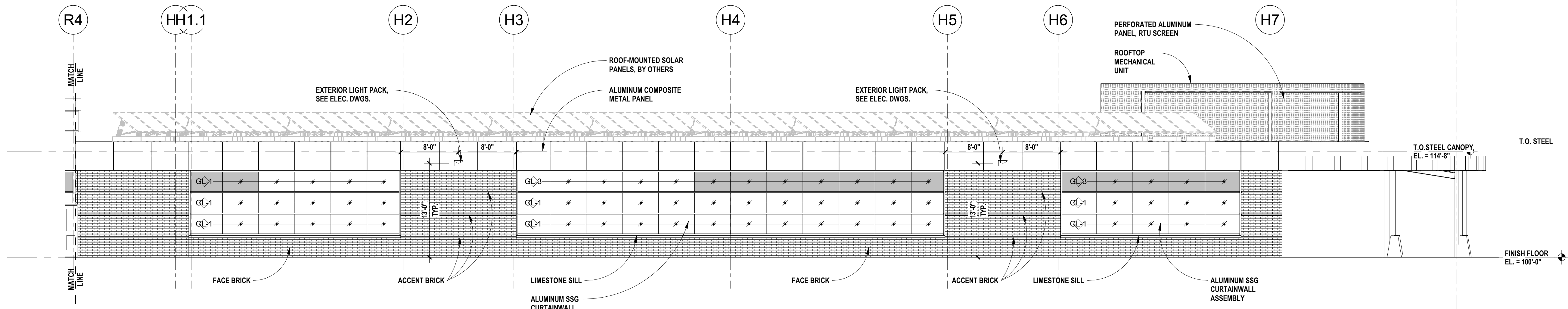
PROJECT NO.

19040

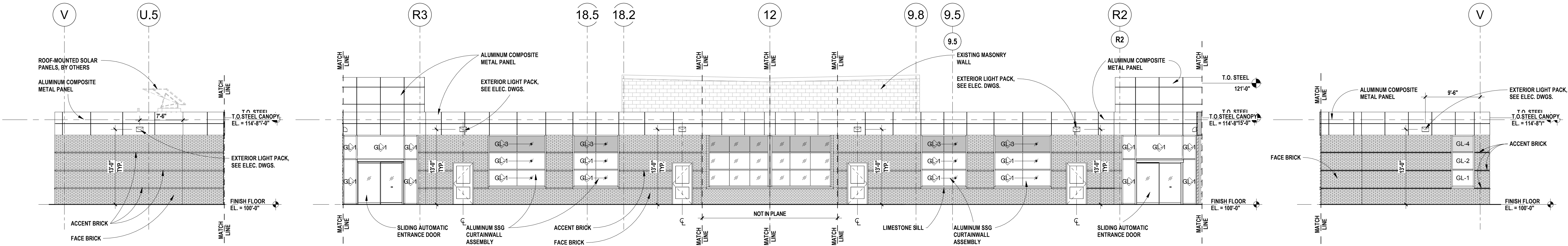
DRAWING NO.

A3.3-BP2

FOR REFERENCE ONLY



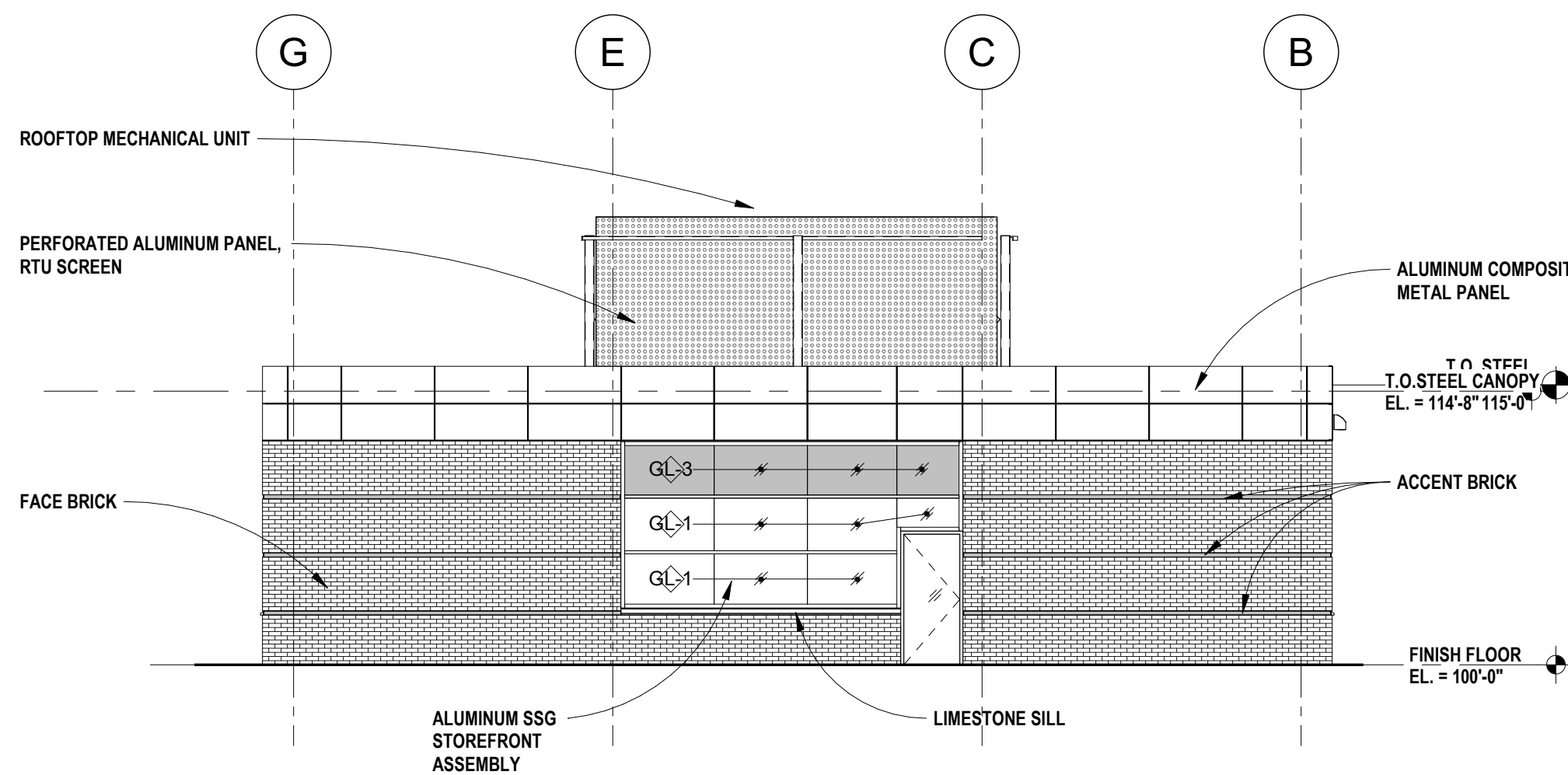
8 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"



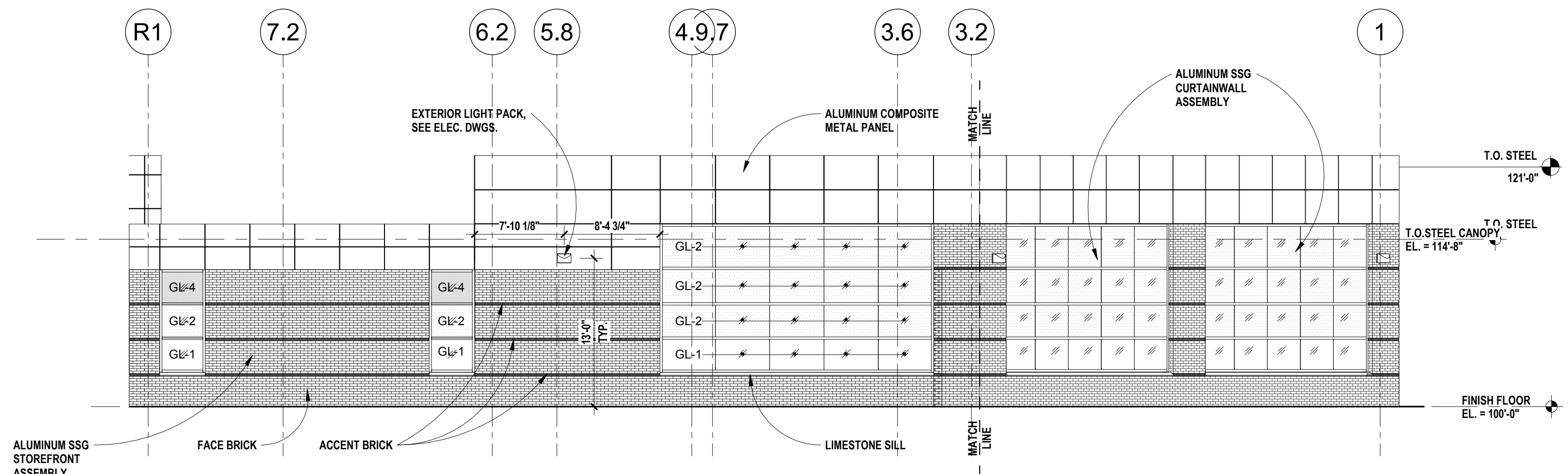
7 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"

6 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"

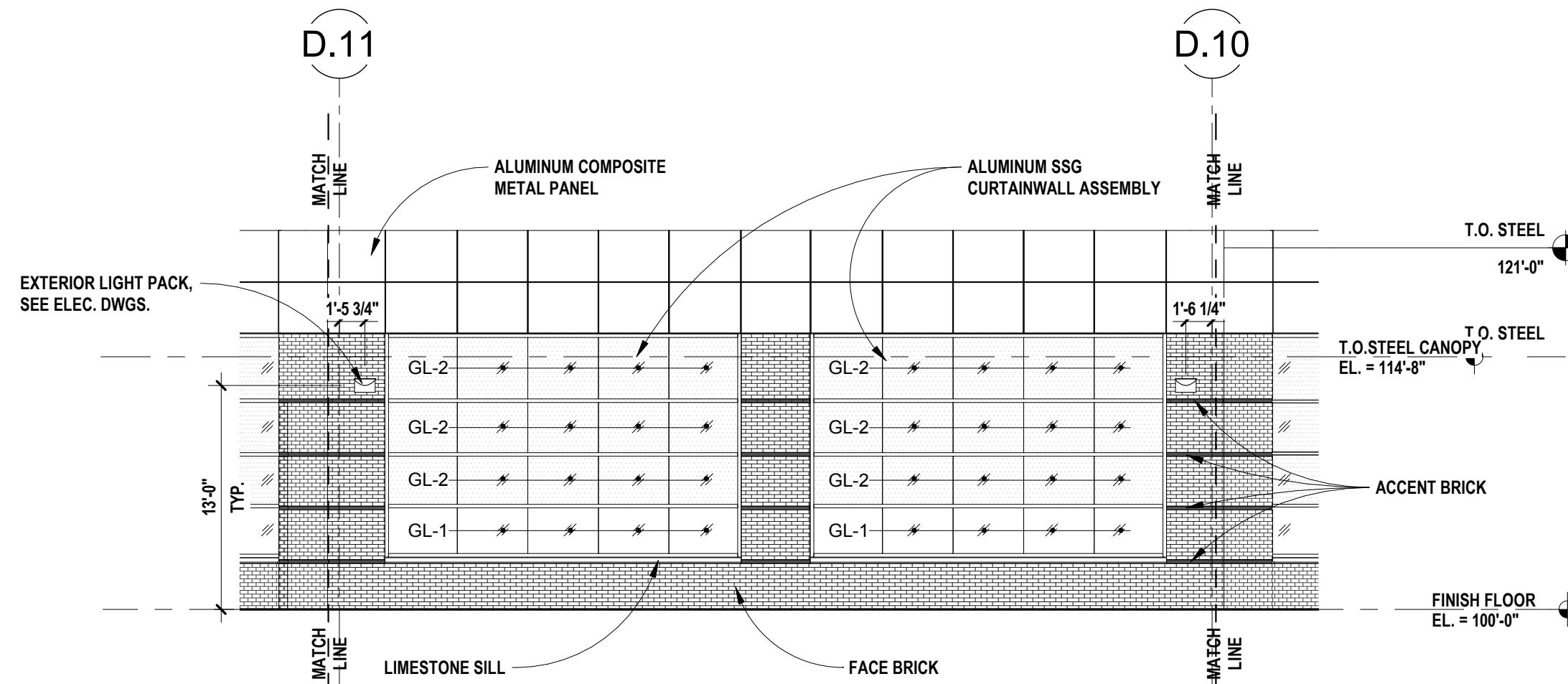
5 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"



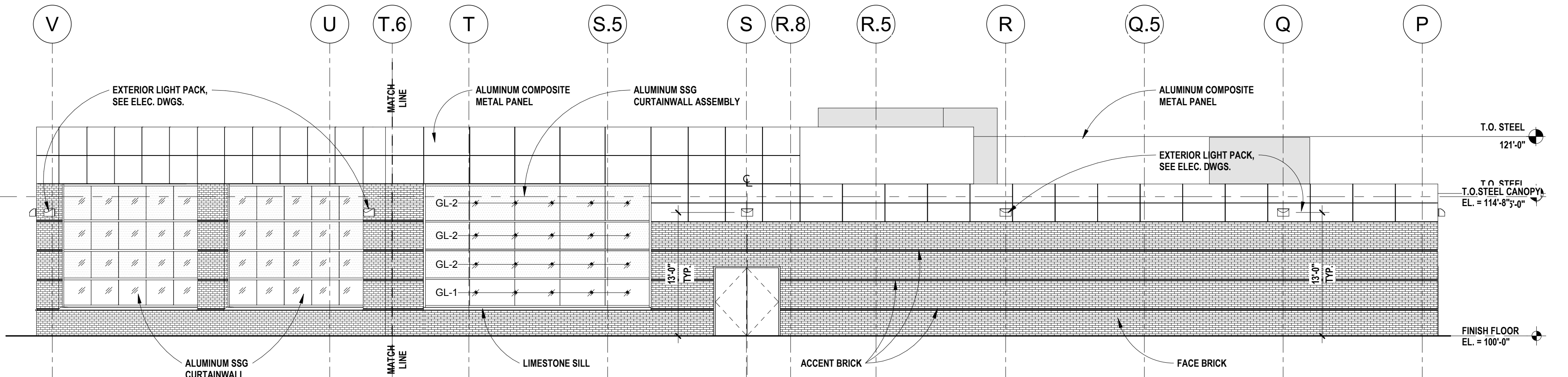
4 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"



3 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"



2 EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



1 BUILDING ELEVATION
SCALE: 1/8" = 1'-0"

REGISTRATION SEAL

CONSULTANT

PROJECT TITLE

**New High Point
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**Washtenaw
Intermediate
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1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE

Exterior Elevations

ISSUE DATES

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CHECKED: Checker
APPROVED: Approver

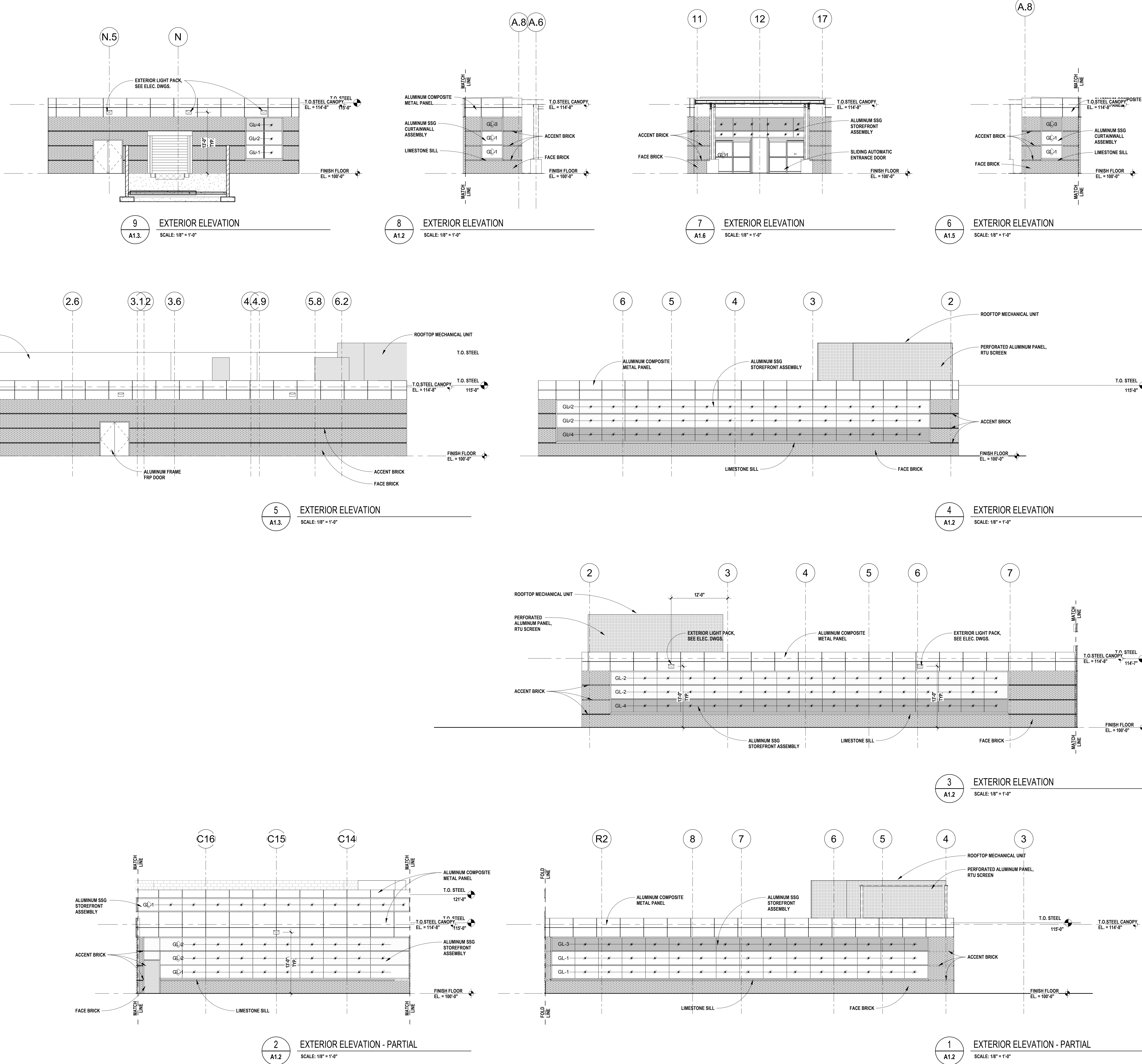
PROJECT NO.

19040

DRAWING NO.

A3.4-BP2

FOR REFERENCE ONLY



REGISTRATION SEAL

CONSULTANT

PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
Exterior Elevations

ISSUE DATES

03/16/2020 FOR CONSTRUCTION - BID PACK #2
DATE: ISSUED FOR:
DRAWN: dco
CHECKED: don
APPROVED: jjc

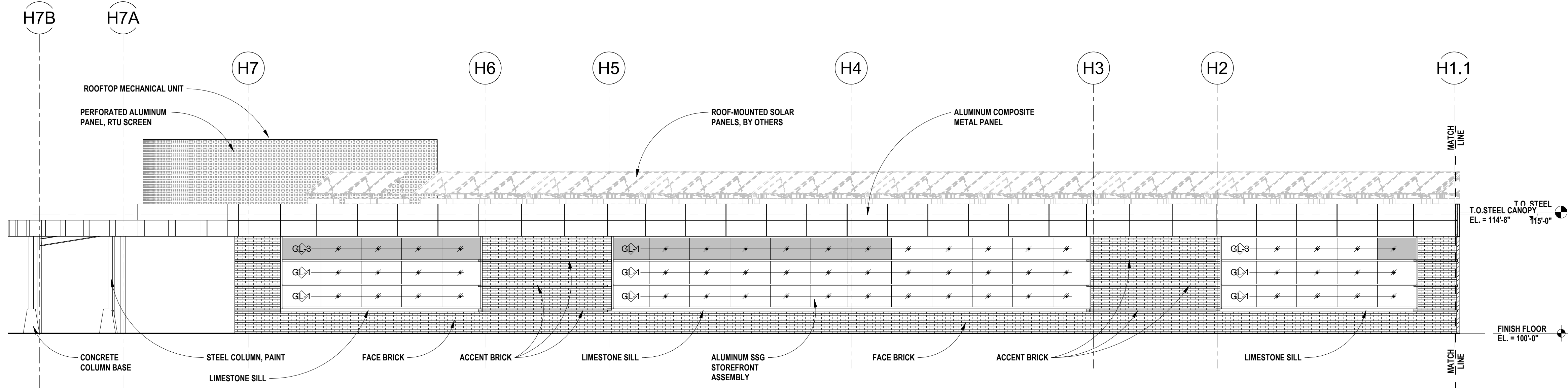
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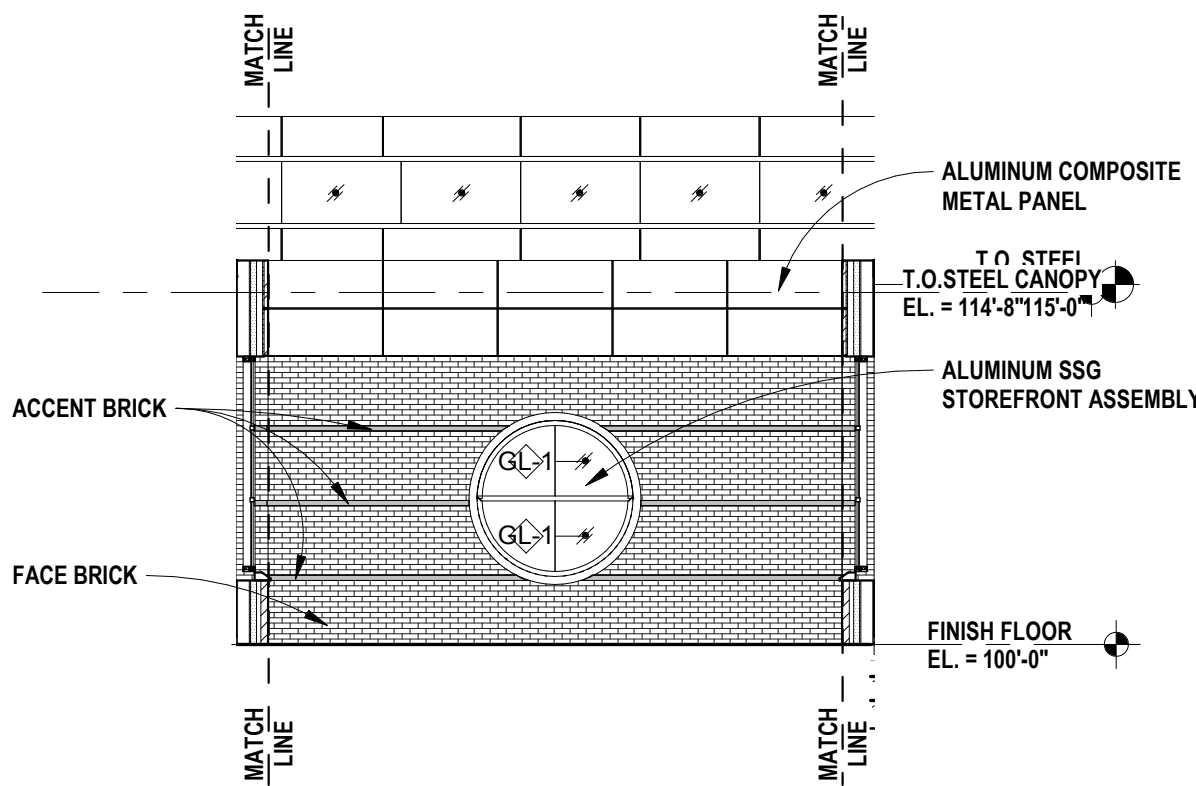
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A3.5-BP2

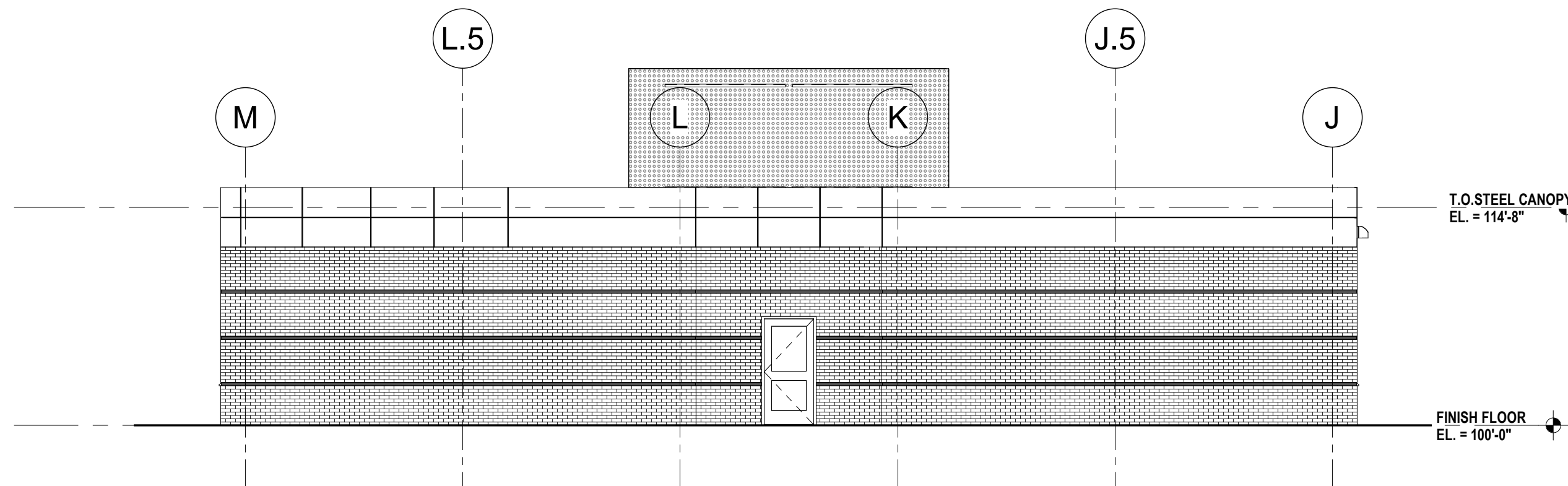
FOR REFERENCE ONLY



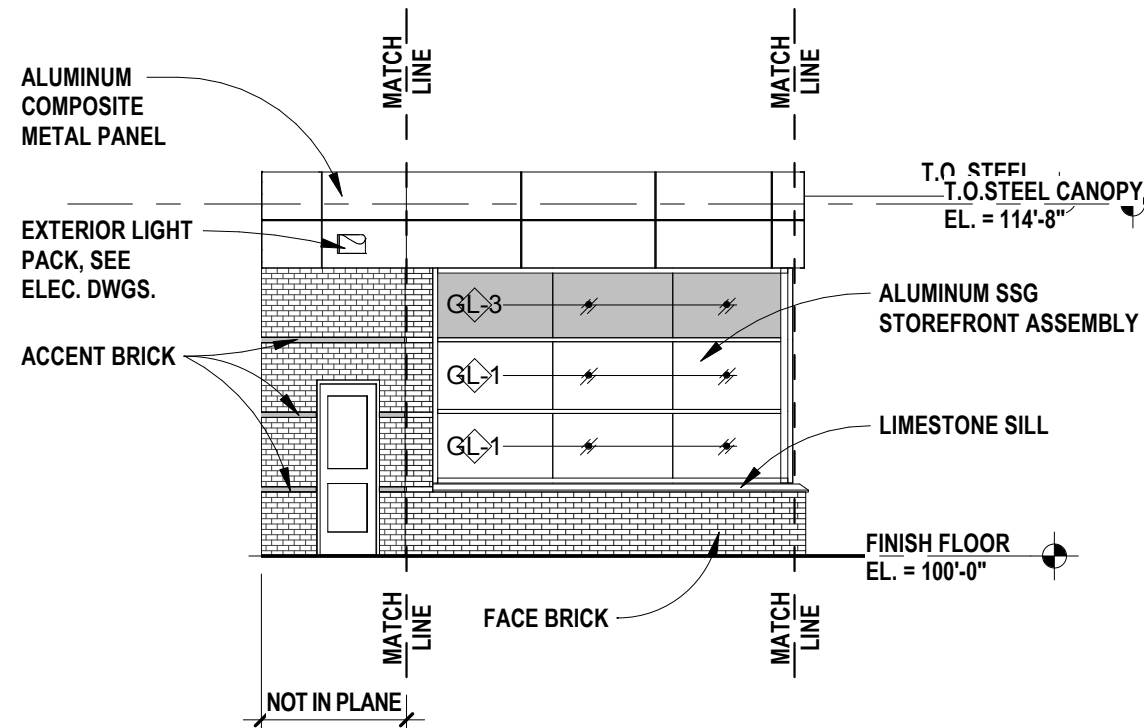
8
A1.4
EXTERIOR ELEVATION - PARTIAL
SCALE: 1/8" = 1'-0"



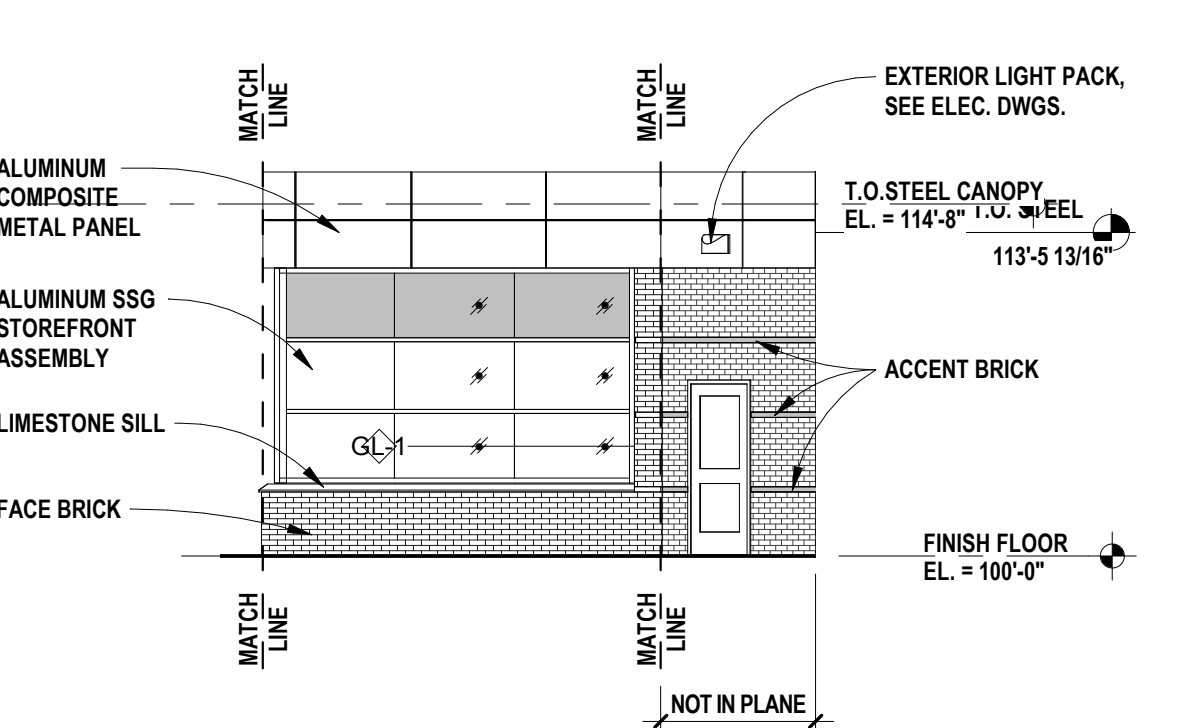
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EXTERIOR ELEVATION - PARTIAL
SCALE: 1/8" = 1'-0"



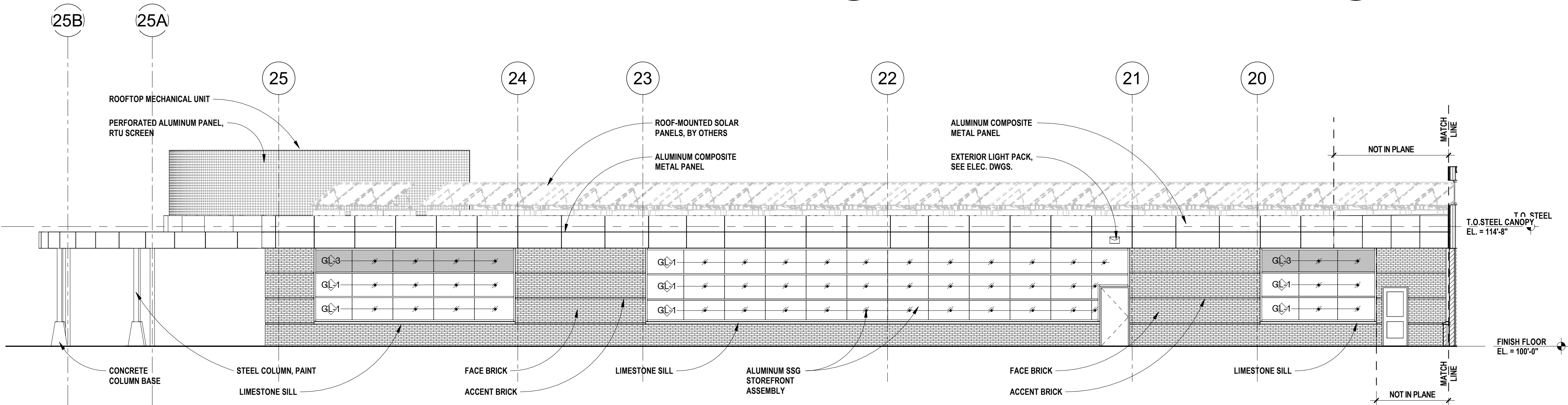
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BUILDING ELEVATION
SCALE: 1/8" = 1'-0"



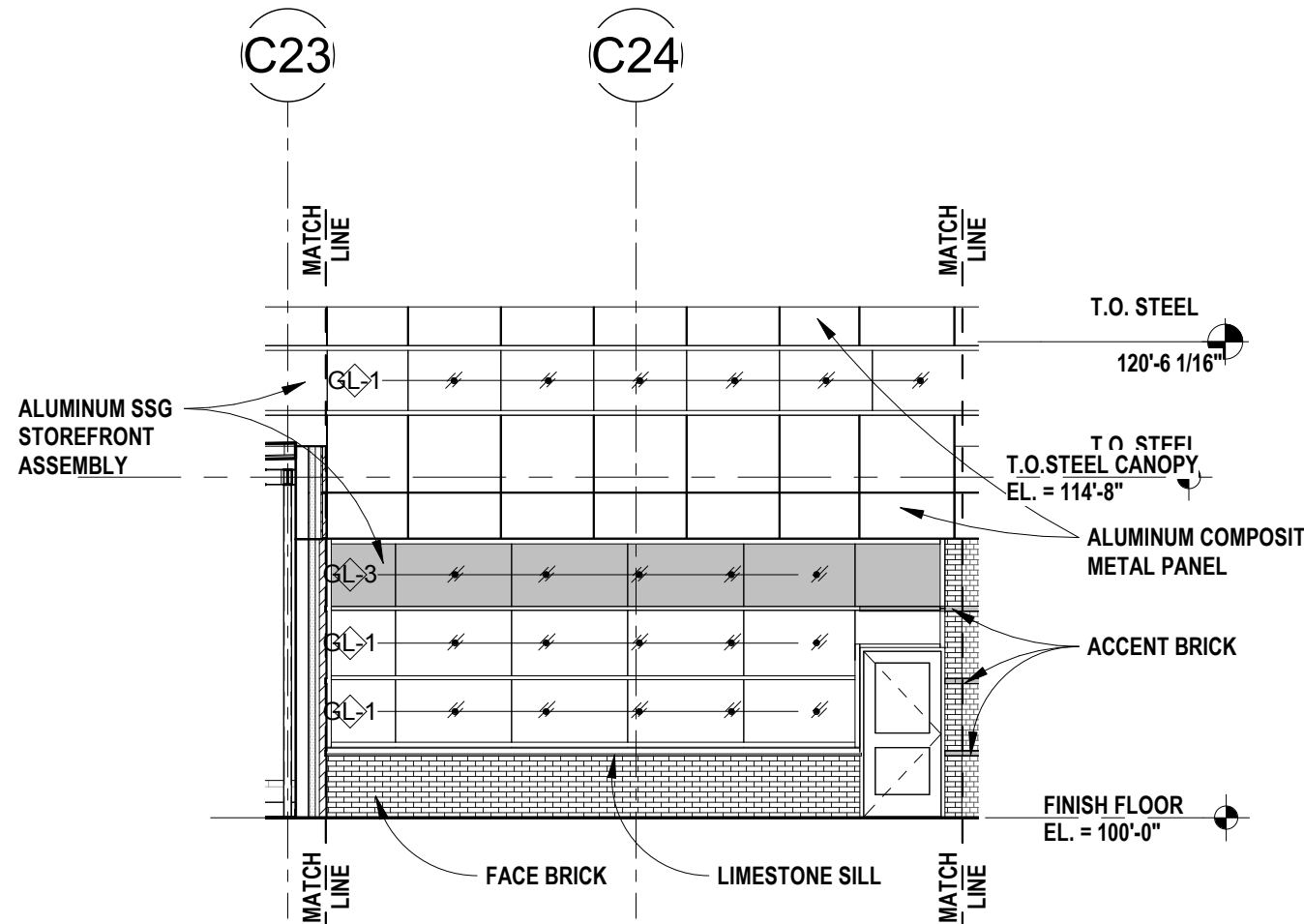
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EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



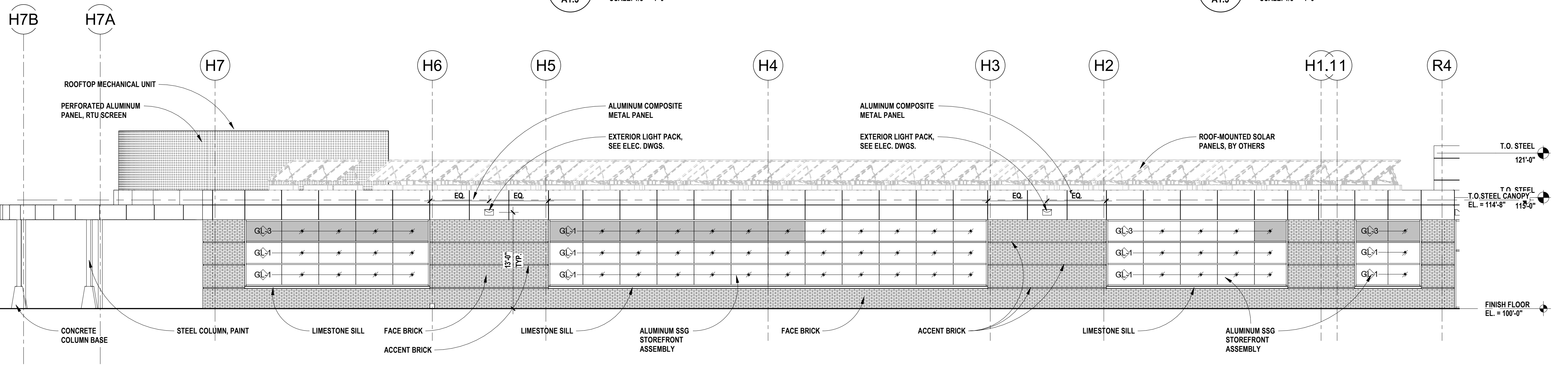
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EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



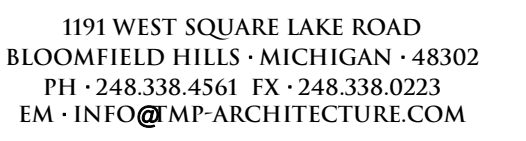
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A1.5
EXTERIOR ELEVATION - PARTIAL
SCALE: 1/8" = 1'-0"



2
A1.5
EXTERIOR ELEVATION - PARTIAL
SCALE: 1/8" = 1'-0"



1
A1.4
EXTERIOR ELEVATION
SCALE: 1/8" = 1'-0"



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CONSULTANT

New High Point
School

Washtenaw
Intermediate
School District

1735 South Wagner Road
Ann Arbor, Michigan

DRAWING TITLE
**Exterior Elevations -
Clerestory**

03/16/2020 FOR CONSTRUCTION - BID PACK #2

DATE: ISSUED FOR:

DRAWN	Author
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CHECKED Checker

APPROVED _____

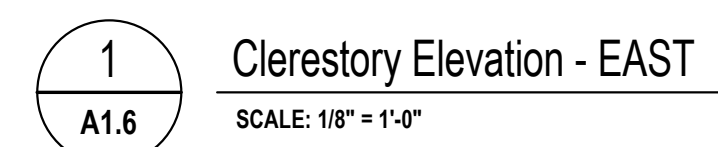
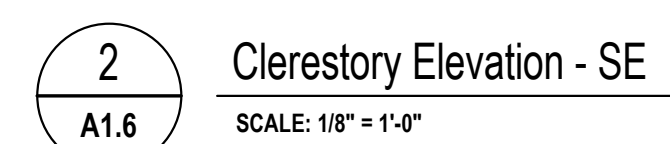
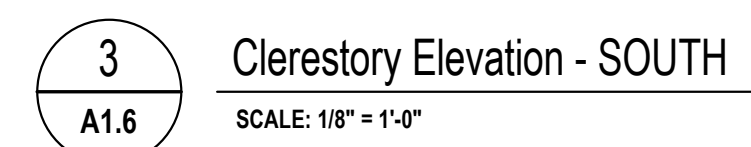
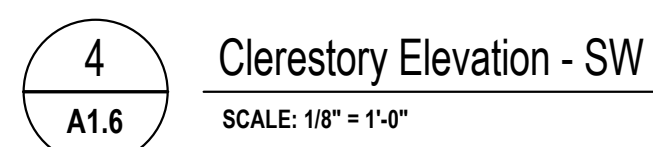
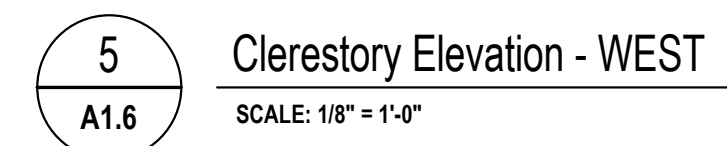
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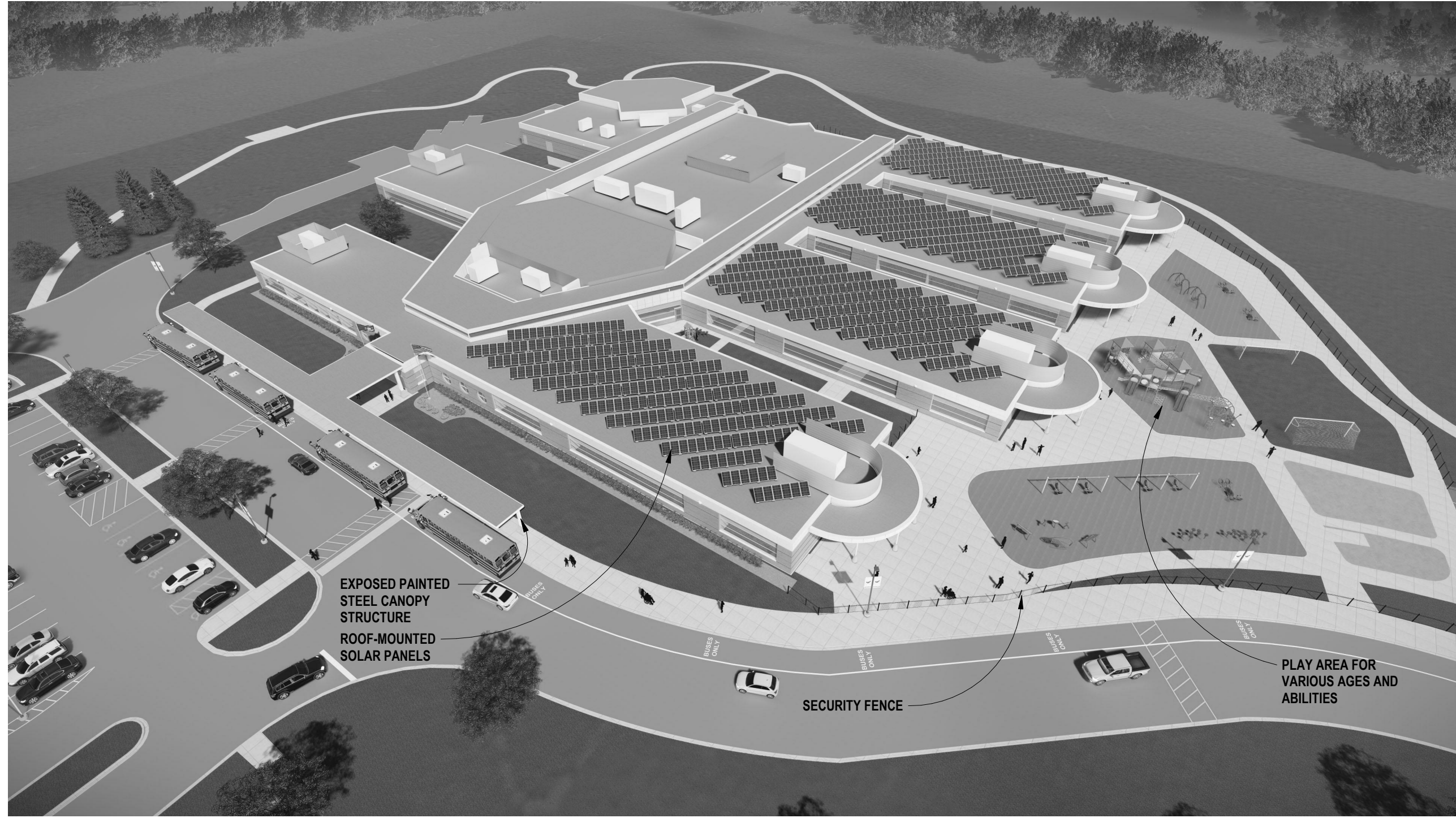
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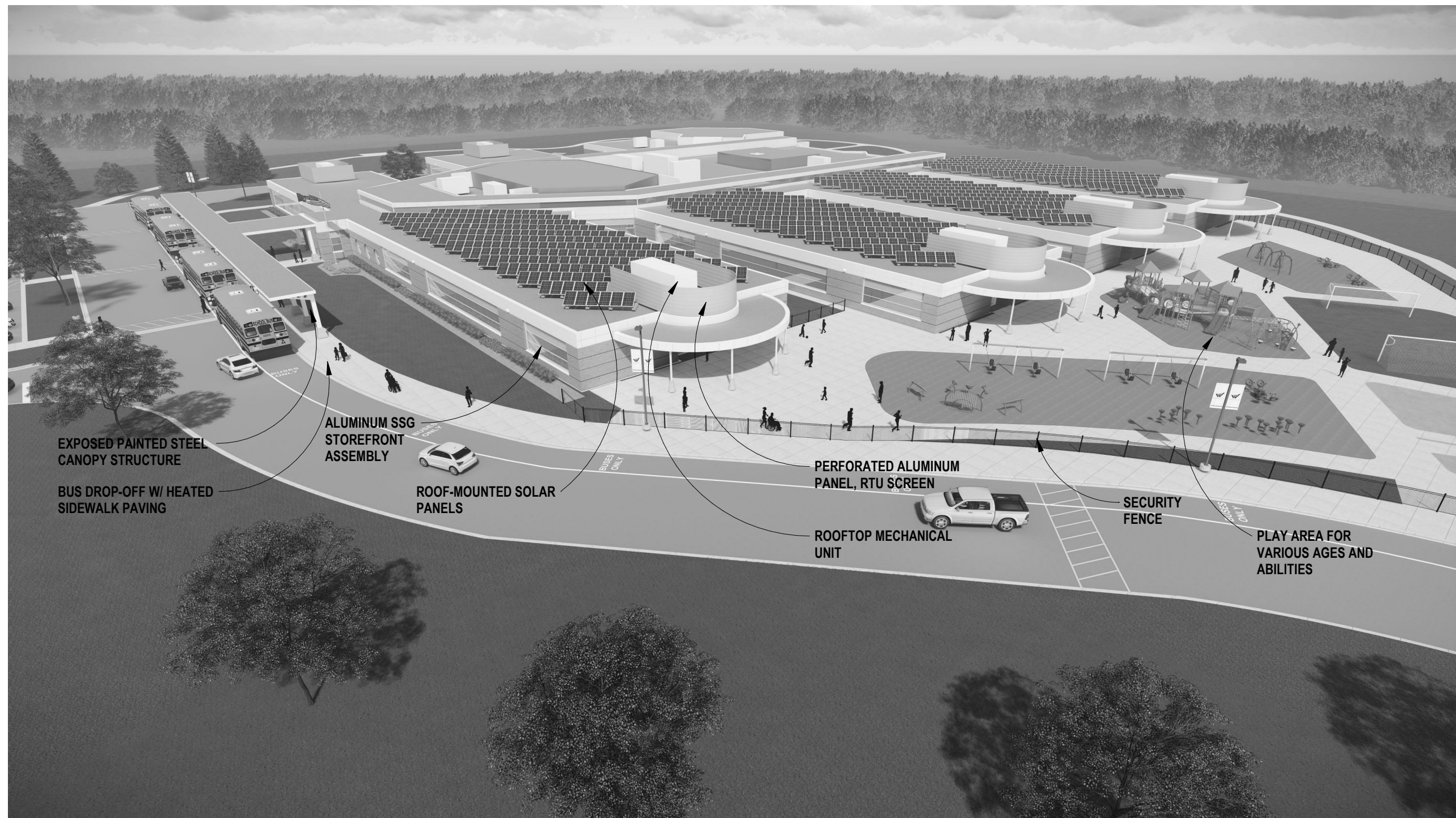




EXTERIOR - OVERALL



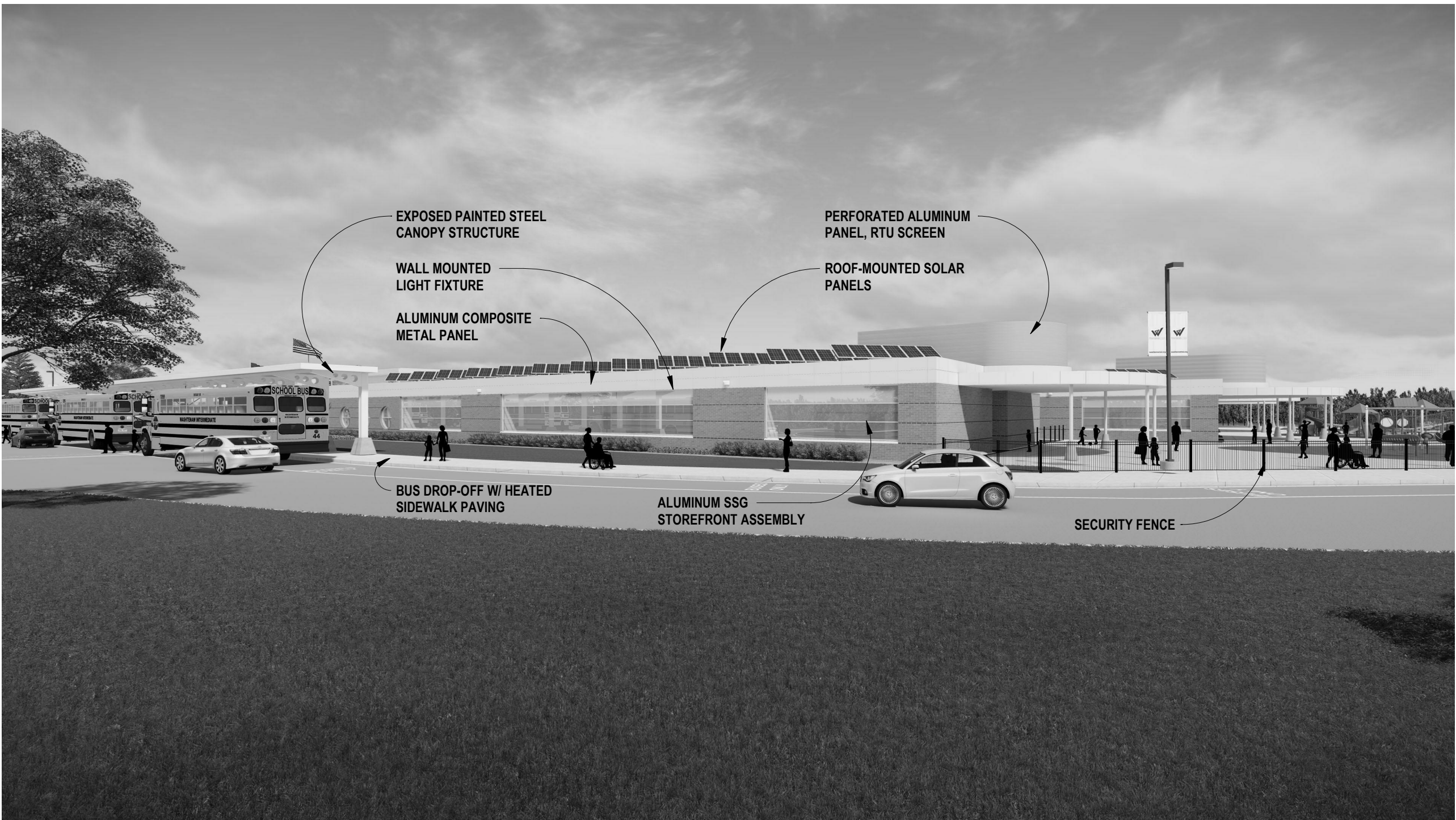
EXTERIOR - OVERALL



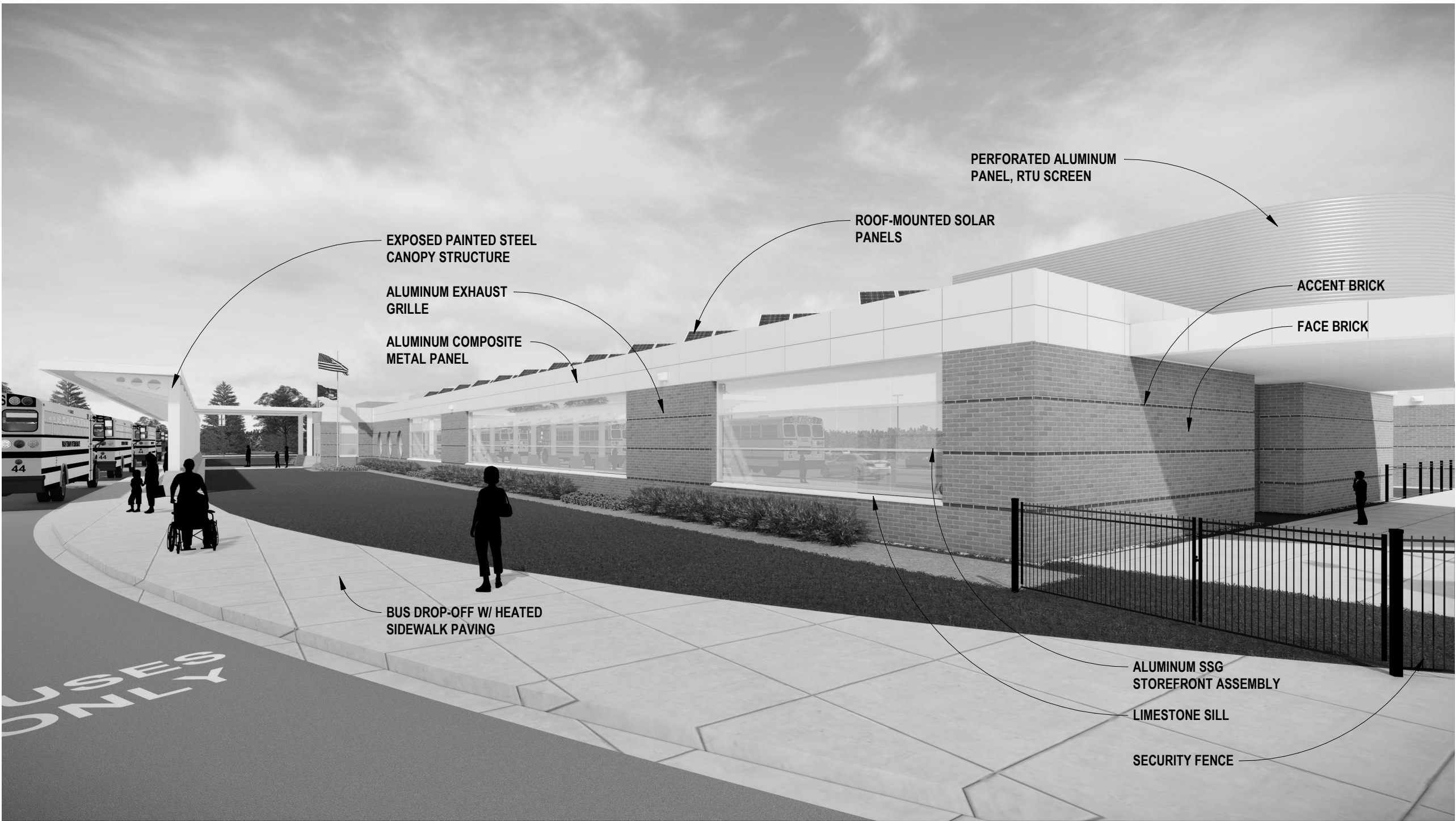
EXTERIOR - OVERALL



EXTERIOR - ENTRANCE



EXTERIOR - ENTRANCE



EXTERIOR - BUS DROP-OFF

REGISTRATION SEAL

CONSULTANT

PROJECT TITLE
New High Point School
Washtenaw Intermediate School District
1735 South Wagner Road
Ann Arbor, Michigan

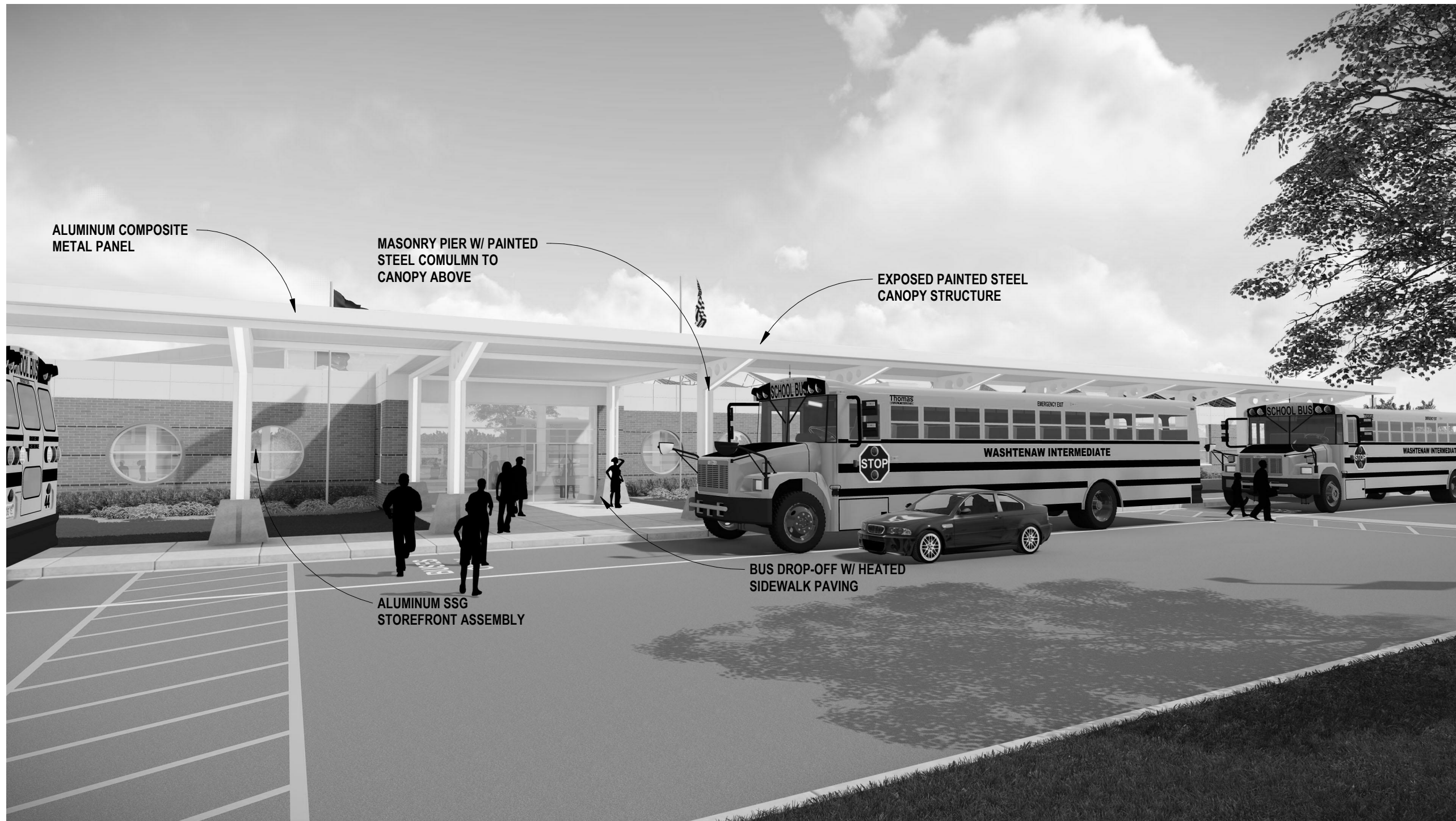
DRAWING TITLE
Renderings - Exterior

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A3.22-BP2

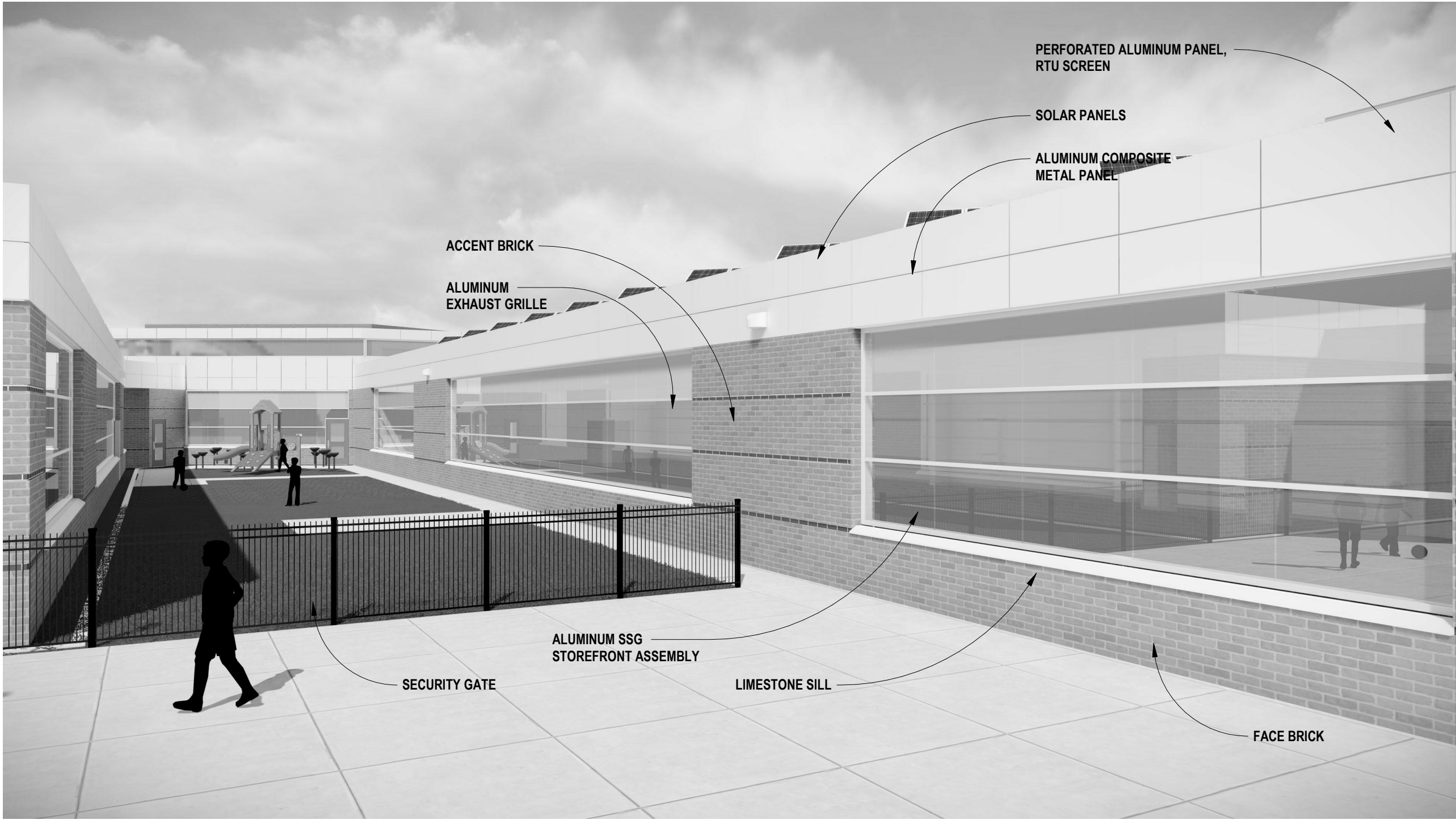
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EXTERIOR - MAIN ENTRANCE



EXTERIOR - CAFETORIUM



EXTERIOR - INTERSTITIAL SPACE



EXTERIOR - PLAYGROUND



EXTERIOR - CANOPY

REGISTRATION SEAL

CONSULTANT

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DRAWING TITLE
Renderings - Exterior

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