



FLOOR PLAN GENERAL NOTES:

- ALL DETAILS SHALL BE REVIEWED FOR SCOPE OF WORK.
- ALL PLAN DIMENSIONS ARE NOMINAL TO FACE OF WALL. WALL THICKNESSES ARE SHOWN NOMINAL. SEE WALL SECTIONS FOR ACTUAL THICKNESS.
- FIELD VERIFY ALL MILLWORK OPENINGS.
- SEE ELEC. LIGHTING PLAN FOR QUANTITY AND LOCATION OF ALL LIGHT FIXTURES.

FLOOR PLAN KEY NOTES:

- 2" SOFFIT @ PERIMETER OF BUILDING W/ EAVE VENT

GENERAL STRUCTURAL NOTES:

- Verify dimensions before commencing work. Report discrepancies to the Architect.
 - Verify openings in the framing plans with the architectural, mechanical and electrical drawings.
 - Design loads
 - Designed in accordance with Michigan Building Code 2009.
 - Roof Snow Load:
 - Ground snow load $P_g = 25 \text{ psf}$
 - Flat roof snow load, $P_f = 21 \text{ psf}$
 - Snow exposure factor, $C_e = 1.0$
 - Snow load importance factor, $I = 1.0$
 - Thermal factor, $C_t = 1.2$
 - Drifted Snow load = 25 psf
 - Wind Loads:
 - Basic wind speed, $V = 40 \text{ mph}$
 - Wind load importance factor, $I = 1.0$
 - Wind exposure B
 - Internal pressure coefficient, $C_{pi} = +/- 0.18$
- | Wall Components | Effective Wind Area (ft ²) | Positive Pressure (psf) | Negative Pressure (psf) |
|-----------------|--|-------------------------|-------------------------|
| -End Zone | 10 | 14.6 | 14.5 |
| | 20 | 19.1 | 18.2 |
| | 50 | 13.0 | 16.5 |
| | 100 | 12.4 | 15.1 |
| -Interior Zone | 10 | 14.6 | 15.8 |
| | 20 | 19.1 | 19.1 |
| | 50 | 13.0 | 14.3 |
| | 100 | 12.4 | 13.6 |
- Earthquake Design Data:
 - Seismic use group, II
 - Seismic importance factor, $I = 1.0$
 - Spectral response coefficients: $S_{ds} = 0.124$, $S_{d1} = 0.070$
 - Site Class D
 - Basic seismic - force - resisting system: Shear wall
 - Seismic design category, B

FOUNDATION NOTES:

- Foundations are designed based on soil bearing of 1000 psf. If soil of this capacity is not found at the elevation noted, enlarge or lower footings at the direction of the Architect/Engineer.
- Place structural backfill in layers not exceeding 4" loose thickness. Compact each layer to at least 95% of the maximum density per ASTM D-1557. Compacting by flooding is not permitted.
- Center footings under wall location and columns unless noted.
- Earth forms are not permitted unless specifically noted.
- Following demolition of structures and stripping of topsoil, thoroughly proof-roll slab on grade areas utilizing a fully loaded single axle dump truck. Proof rolled areas shall be inspected. Loose areas shall be completely removed and replaced with 2'-0" compacted structural fill.
- Existing foundations or floor slab encountered during site gradings and excavation shall be removed to a depth of two (2) feet below new construction. Replace with structural backfill.
- Provide bond break material between all grade slabs and vertical surfaces.
- Casings will be required for installation of drilled piers.
- Backfill and excavation per specifications.

CONCRETE NOTES:

- ACI Building Code (318); Manual of Standard Practice for Detailing (315) for the mixing, fabrication and placement of concrete, reinforcing steel, and accessories.
- Concrete Strength - (Standard) weight concrete:
 - Footings: $f_c = 3000 \text{ psi}$
 - Concrete Slabs on Grade: $f_c = 3500 \text{ psi}$
 - Concrete Entry Slabs: $f_c = 4500 \text{ psi}$
- Reinforcing - Bars: ASTM A-615 grade 60
Welded Wire Fabric: ASTM A-185
- Concrete Slabs on Grade Reinforcing: 6x6 - #14x#14 W/WF unless noted. Located in the upper 1/3 of slab thickness.
- Provide saucut control joints at approximately 20' on center each way in slabs on grade, see details. Locate joints under partitions whenever possible. Construction joints at Contractor's option.
- Depress slabs as required for floor finishes, see Architect.
- Slope floors as required to floor drains, see Architect.
- Form all concrete.
- Provide corner bars for all contiguous corners.
- Water/cement ratio limits:
 - $f_c = 3000 \text{ psi}$: 0.68 non-air entrained, 0.50 air entrained
 - $f_c = 4500 \text{ psi}$: 0.40 air-entrained
- Slump limits:
 - 3" for foundations, 4" for slabs
- Provide air entrained concrete for exterior exposures.
- Wall footing reinforcing lap length: minimum 21", 21" if laps staggered.

MASONRY NOTES:

- Work shall be performed in accordance with ACI 530 specifications.
- Mortar: ASTM C270, Type M below grade, Type M or S above grade, Type N for non load bearing above grade.
- Grout: ASTM C476, $f_c = 20000 \text{ psi}$, tested per ASTM C1014.
- Reinforcing bars shall be ASTM A-615, Grade 60, lap minimum 40 bar diameters for 5 bars and smaller, lap minimum 52 bar diameters for bars larger than #5 unless noted otherwise.
- Horizontal wall reinforcing: As specified.
- Concrete masonry units: ASTM C-40, Grade N, two core type for reinforced masonry.
- Vertical bar reinforcing: Place accurately and mechanically hold in position while grouting. Grouting shall be done in lifts not exceeding 4'-0" and mechanically consolidated in place; consolidation by rodding not acceptable.
- Provide completely grouted concrete:
 - Under cast-in-place concrete floor bearing
 - Under wall bearing
 - Under any change of wall thickness, i.e.: 8" on top of 12"
- Temporary wall bracing is the Contractor's responsibility. Conform to applicable Codes and Standards.

WOOD FRAMING:

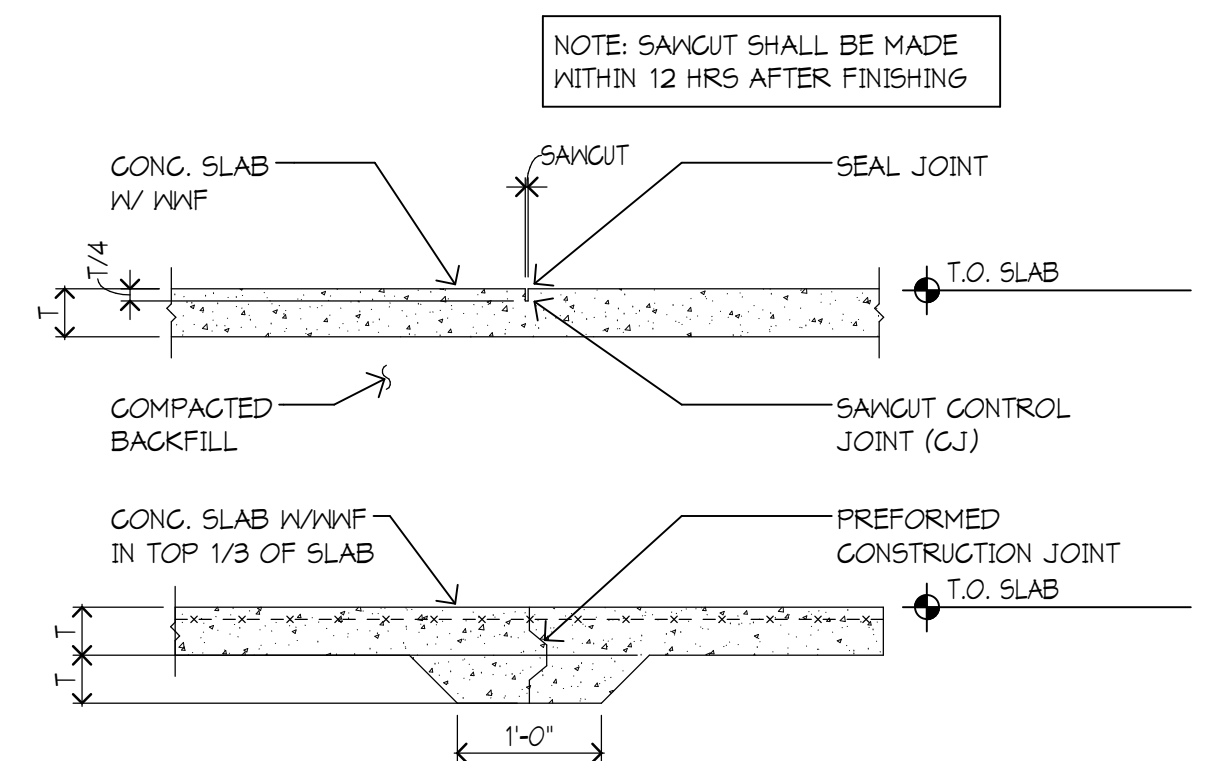
- Dimensional framing material shall bear the grade mark of an AISC approved agency, kiln dried, and have the following minimum stress grade:
 - 2x4 stud walls: Spruce-pine-fir, construction grade or better.
 - 2x6 and larger: Hem-Fir #2 or better.
- Anchor continuous blocking to masonry with minimum 1/2" anchor bolts 16" long plus 3" hook at 32" o.c. unless noted otherwise.
- Roof rafters anchored to continuous plates with one Simpson H3 anchor unless noted otherwise.

PLYWOOD ROOFING SHEATHING:

- Plywood for roof deck shall be minimum 7/16" thick APA rated sheathing, 24/16.
- Roof sheathing fastened with 8d common nails at 6" o.c. at panel edges and intermediate supports unless noted otherwise.
- Panel shall be laid in a staggered pattern, continuous over two spans.

WOOD TRUSSES:

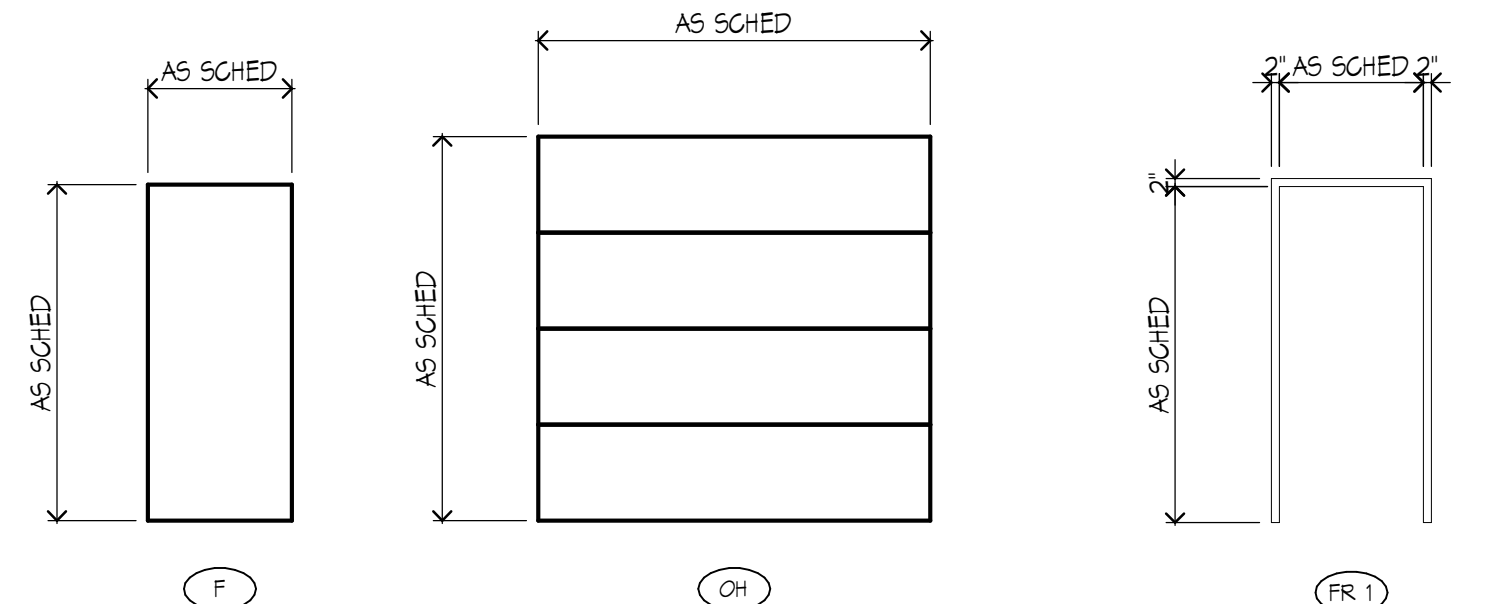
- Wood trusses shall be designed and fabricated per Truss Plate Institute, Inc. specifications.
- Design loads for trusses shall be:
 - Top chord live load - 25 psf
 - Top chord dead load - 15 psf
 - Bottom chord live load - 5 psf
 - Top chord wind uplift - calculate per 2009 Michigan Building Code 90 mph, exposure B, $I = 1.0$. Dependent on effective wind area and zone.
 - Loads shall be combined per 2009 Michigan Building Code load combinations.
- Shop drawings for trusses shall be prepared by the supplier and submitted to the Architect/Engineer for approval. Shop drawings shall be prepared under the supervision and shall be certified by a Registered Professional Engineer in the State of Michigan.
- Trusses shall be anchored to supports with one Simpson H2.5A anchor unless noted otherwise.
- Limit overhang member deflection to $L/180$.
- Rafter to truss connection design and specification by truss manufacturer. Minimum connection (2) Simpson L5 70 angles.
- Provide matching depth truss members at 2x framing connections.
- Lateral web bracing design and connections by Truss designer.
- All temporary truss bracing to be left in place as permanent bracing.
- Permanent bottom chord bracing design and connections by Truss designer.



CONTROL / CONSTRUCTION JOINTS

3/4" = 1'-0"

Item	Manufacturer	Color/Finish
07 61 13 Sheet Metal Roofing	Prefinished Metal Roofing And Flashing	Use-Clad
08 36 00 Sectional Metal Overhead Doors	Factory Painted Door Panels	As specified
07 46 13 Concrete Board Siding	Fiber Cement board trim and lap siding	As specified
08 71 00 Finish Hardware	Bar Hinges Exterior	As specified
	Flush Bolts	As specified
	Locks	As specified
	Push And Push Plates/ Bars	As specified
	Coverplates	As specified
	Closers	As specified
	Promoter Plates	As specified
	Wall Stops	As specified
	Thresholds	As specified
	Weatherstrips And Sweep Strips	As specified
	Flush Pull	As specified
09 91 00 Paint	PT 1 Exterior MM doors and frames	Sherrin Williams
		Color: SW 6027 Condoon

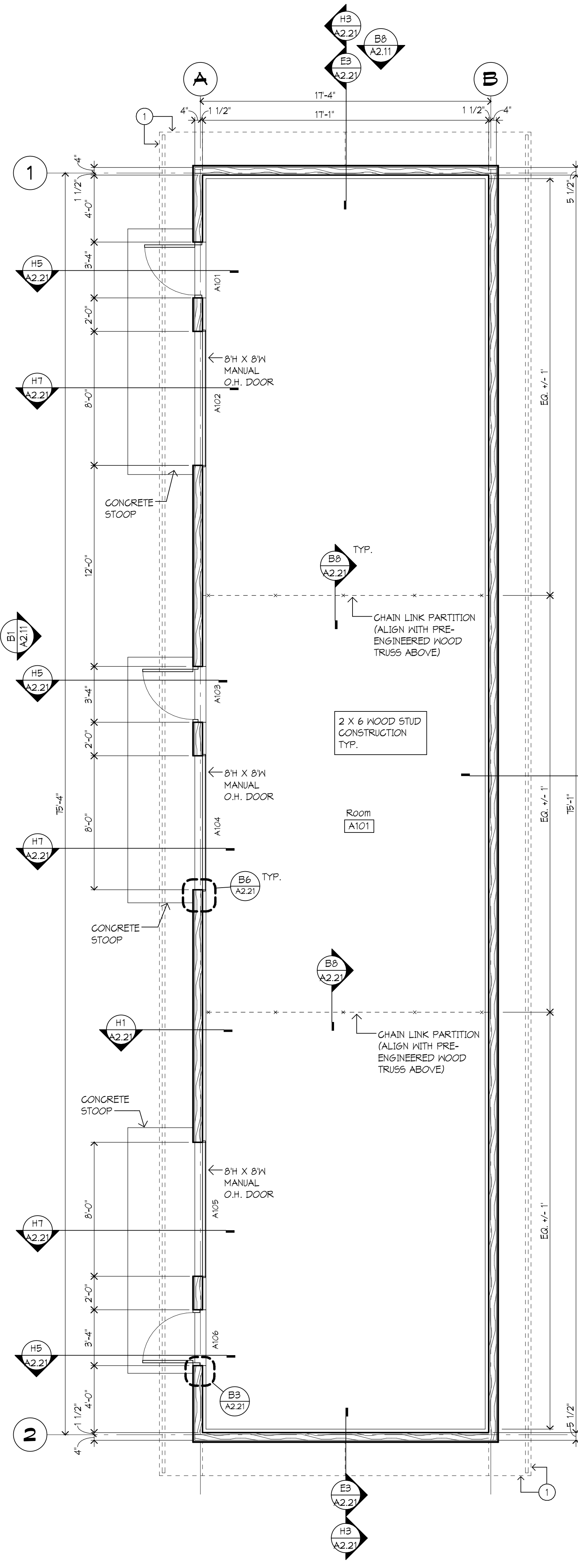


DOOR TYPES

HARDWARE GROUPS:

Storage room doors:	Totals:	Each Assembly to have:
(3)	1	EA CONTINUOUS HINGE 224HD 83"
(3)	1	EA LOCKSET L9076L LLL L0A L28A-150
(3)	1	EA MORTISE CYLINDER 1E74
(3)	1	EA FLUSH PULL 1111G
(3)	1	EA SURFACE CLOSER 4111 SQUISH SRI
(3)	1	EA OVERHEAD STOP 9045
(3)	1	EA KICK PLATE 8400 10" X 34.5"
(3)	1	EA SEALS 197NA 1/32" 2/84"
(3)	1	EA DOOR SWEEP 6607A 36"
(3)	1	EA THRESHOLD 425 36"
Overhead Doors:	(3)	1 EA CYLINDER AND CORE (CORD, W/ OH DOOR MF6.)

Operation: Door latchbolt must have holdback feature. No lever outside. Latchbolt retracted by key outside or lever inside. Auxiliary latch deadlocks latchbolt when door is closed. Inside lever is always free for immediate egress. Lockset must accept a B5T 6-pin core.



G4 MAIN LEVEL FLOOR PLAN
1/4" = 1'-0"

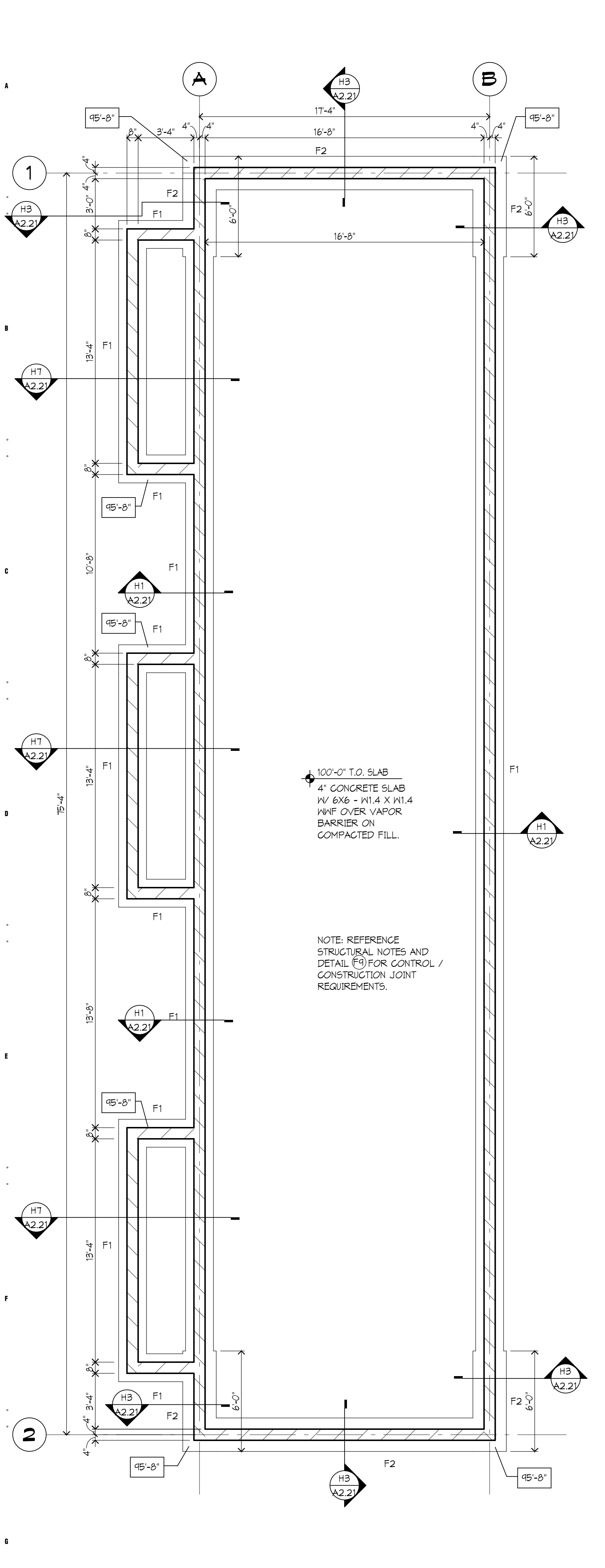
DOOR NO.	TYPE	QTY	WIDTH	HEIGHT	DOOR / OPENING SCHEDULE				MOUNTING CONDITIONS		HPN GRP	REMARKS			
					MATL	LAB	TYPE	DEPTH	MATL	SL			HEAD	JAMB	SILL
A101	F	1	8'-0"	7'-0"	6ALV. ST	--	FR 1	--	6ALV. H.M.	--	B1/2.21	B3/2.21	G1/2.21	1	1
A102	MCD	1	8'-0"	8'-0"	6ALV. ST	--	MIGD	--	--	--	B5/2.21	B6/2.21		2	1
A103	F	1	8'-0"	7'-0"	6ALV. ST	--	FR 1	--	6ALV. H.M.	--	B1/2.21	B3/2.21	G1/2.21	1	1
A104	MCD	1	8'-0"	8'-0"	6ALV. ST	--	MIGD	--	--	--	B5/2.21	B6/2.21		2	1
A105	MCD	1	8'-0"	8'-0"	6ALV. ST	--	MIGD	--	--	--	B5/2.21	B6/2.21		2	1
A106	F	1	8'-0"	7'-0"	6ALV. ST	--	FR 1	--	6ALV. H.M.	--	B1/2.21	B3/2.21	G1/2.21	1	1

DOOR SCHEDULE GENERAL NOTES:

- ALL DOORS ARE 1 3/4" THICK UNLESS OTHERWISE NOTED.
- FOR FRAME DEPTH, ONLY EXCEPTIONS TO THE FOLLOWING TABLE ARE SCHEDULED:
EXTERIOR WALLS: THROAT OF FRAME TO MATCH WALL THICKNESS.
FRAME DEPTHS ARE SCHEDULED IN NOMINAL DIMENSIONS.

DOOR SCHEDULE REMARKS:

- DOORS KEYS TO MATCH OWNER'S EXISTING BUILDING SYSTEM



G1 FOUNDATION PLAN
1/4" = 1'-0"

STRUCTURAL LEGEND

F1 FOOTING - SEE SCHEDULE

0'-0" BOTTOM OF FOOTING ELEVATION

FOOTING SCHEDULE			
MARK	SIZE	DEPTH	REINFORCING
F1	1'-8" CONT.	1'-0"	(2) #5 CONT.
F2	2'-0" CONT.	1'-0"	(3) #5 CONT. T & B

MI

A

I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly licensed ARCHITECT under the laws of the State of MICHIGAN

DANIEL KRITTA
130104782 Date: 10/31/2014

Description	Revisions	Date	Num

Comm: 134014
Date: June 4, 2013
Drawn: BAH/VLS
Check: DTK

FOUNDATION, MAIN LEVEL AND ROOF PLAN

Scale: As indicated

A1.11

**Hazel Park High School
Storage Building**

23400 Hughes
Hazel Park, MI 48030

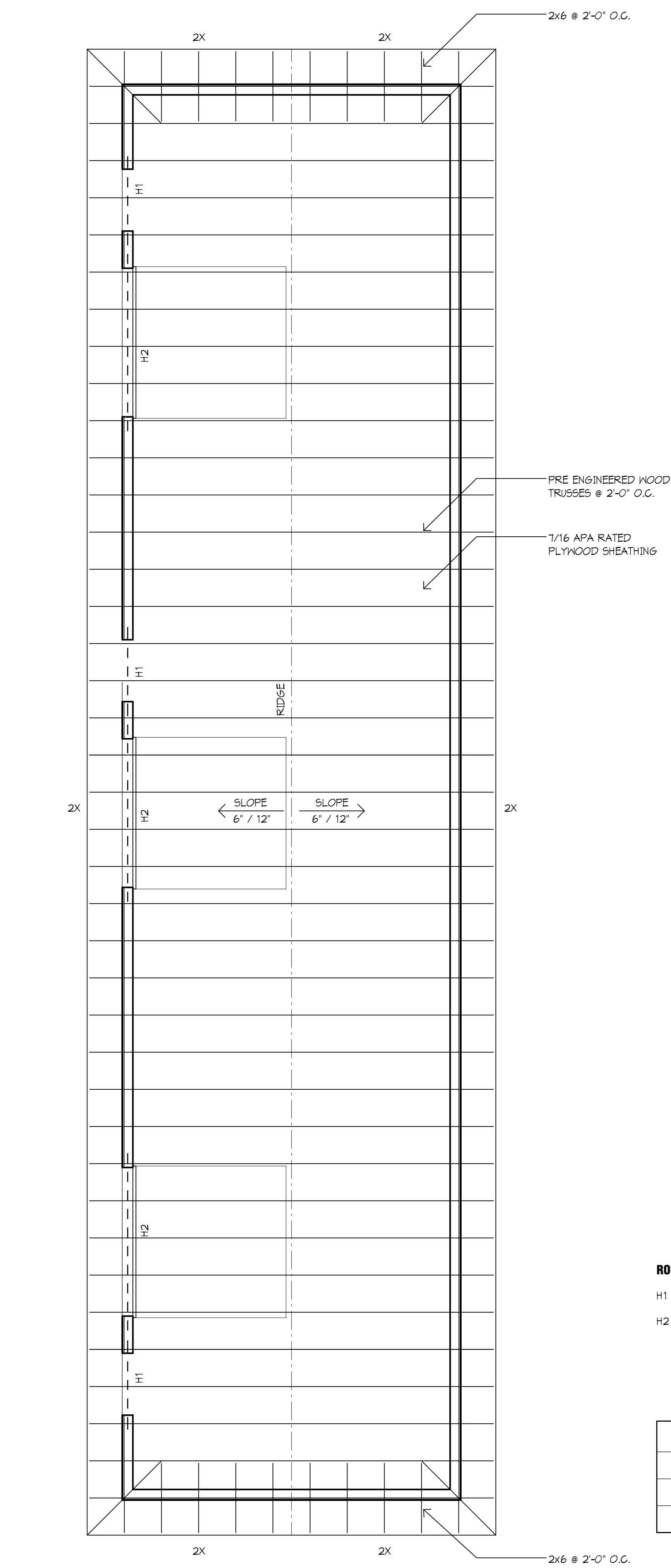
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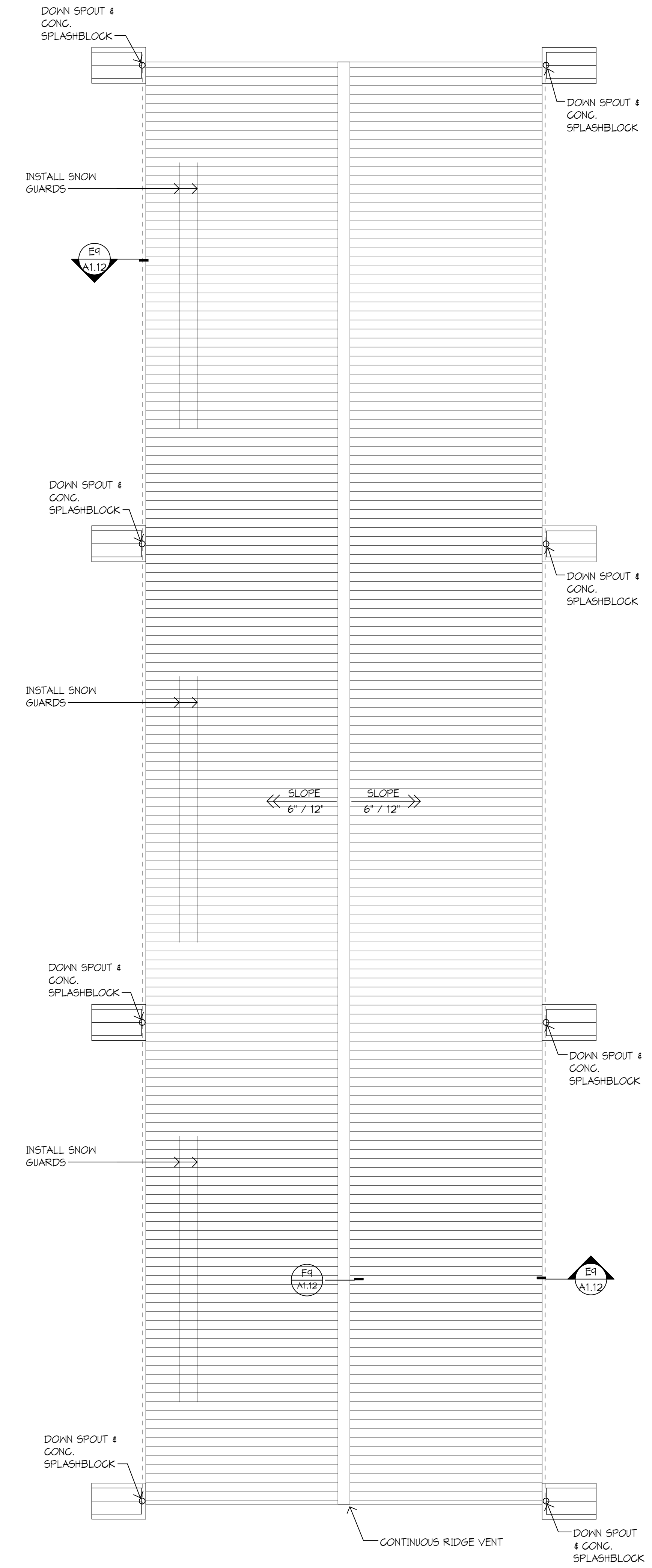
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62 ROOF FRAMING PLAN
1/4" = 1'-0"

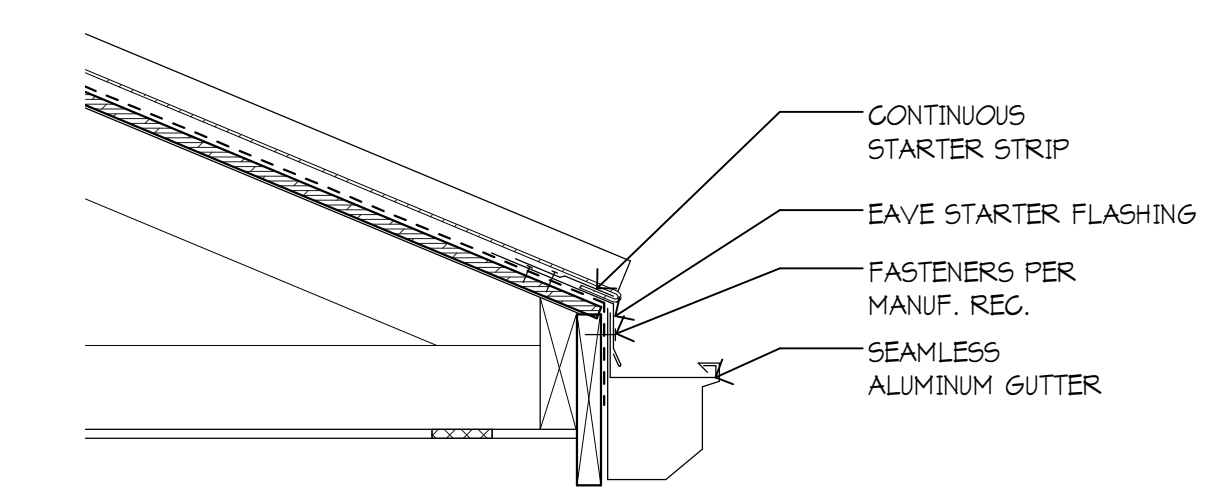
ROOF FRAMING PLAN LEGEND:
H1 - WOOD HEADER, SEE SCHEDULE
H2 - WOOD HEADER, SEE SCHEDULE

HEADER SCHEDULE		
MARK	SIZE	MIN BEARING
H1	(3) 2x6	(1) 2x6
H2	(3) 2x10	(2) 2x6

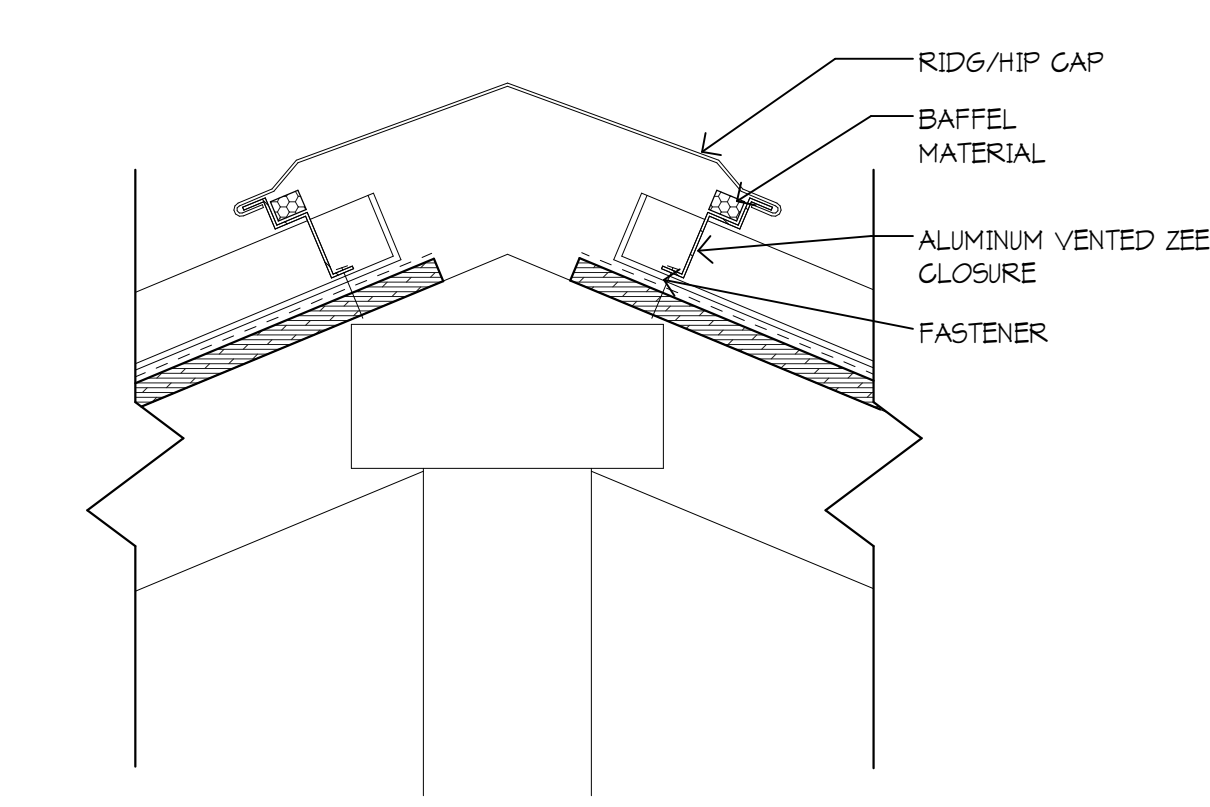


67 ROOF PLAN
1/4" = 1'-0"

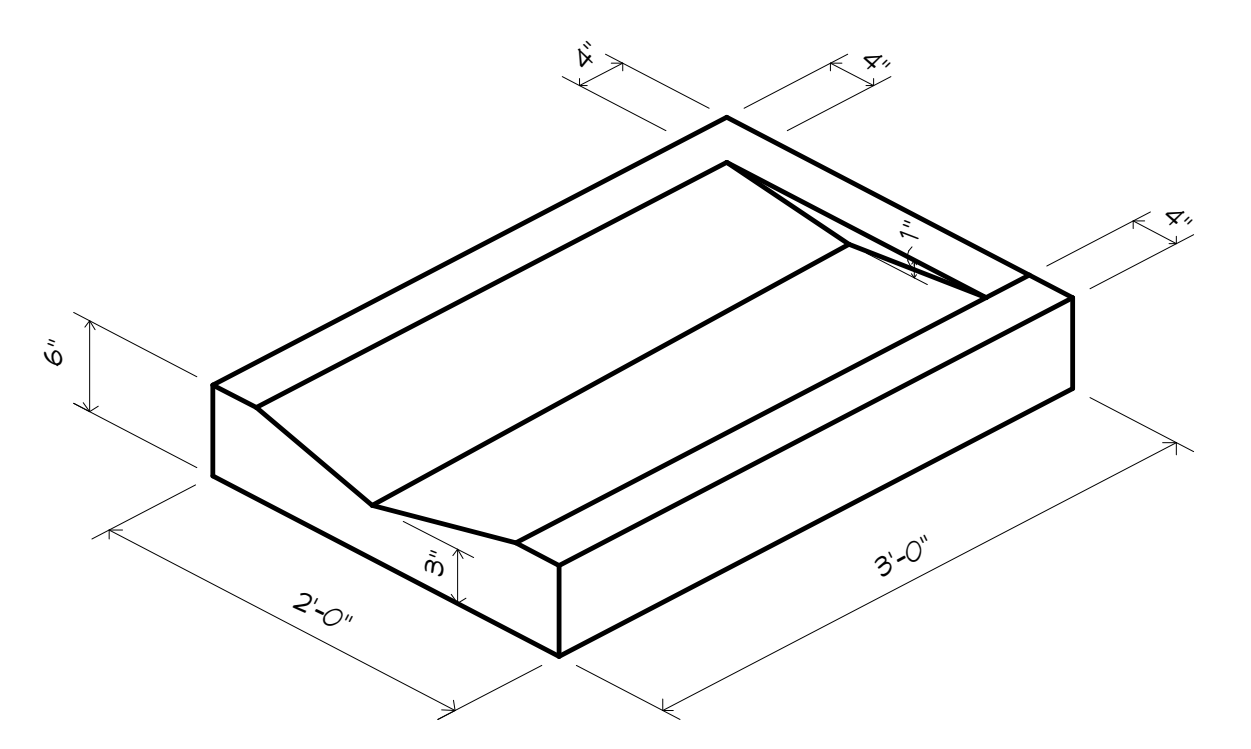
ROOF LEGEND
→ SLOPED STRUCTURE



69 DETAIL @ GUTTER
1 1/2" = 1'-0"



69 RIDGE VENT
3" = 1'-0"



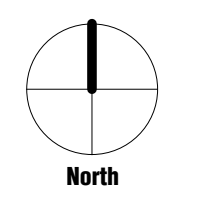
69 CONCRETE SPLASHBLOCK
1" = 1'-0"

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DANIEL KRITTA
Registration Number 1301047B92 Date 10/31/2014

Description	Revisions		Date	Num
	By	Check		

Comm: 134014
Date: June 4, 2013
Drawn: BAH/VLS
Check: DK



ROOF AND ROOF FRAMING PLANS AND DETAILS

Scale: As indicated

A1.12

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Storage Building**

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Hazel Park, MI 48030

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DANIAL KRITTA
Registration Number 1301047892 Date 10/31/2014

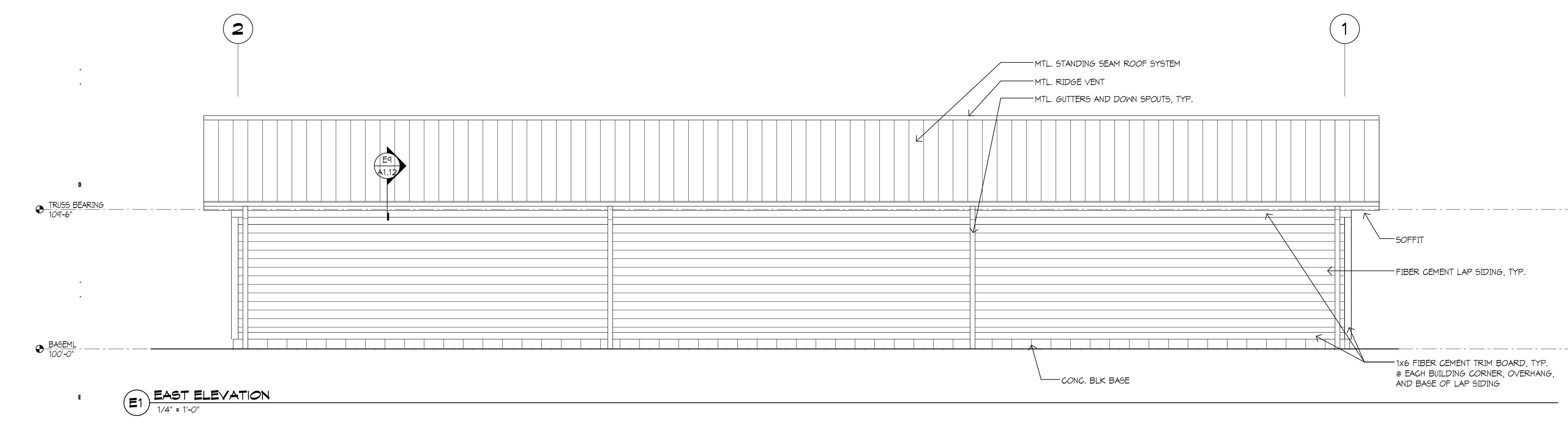
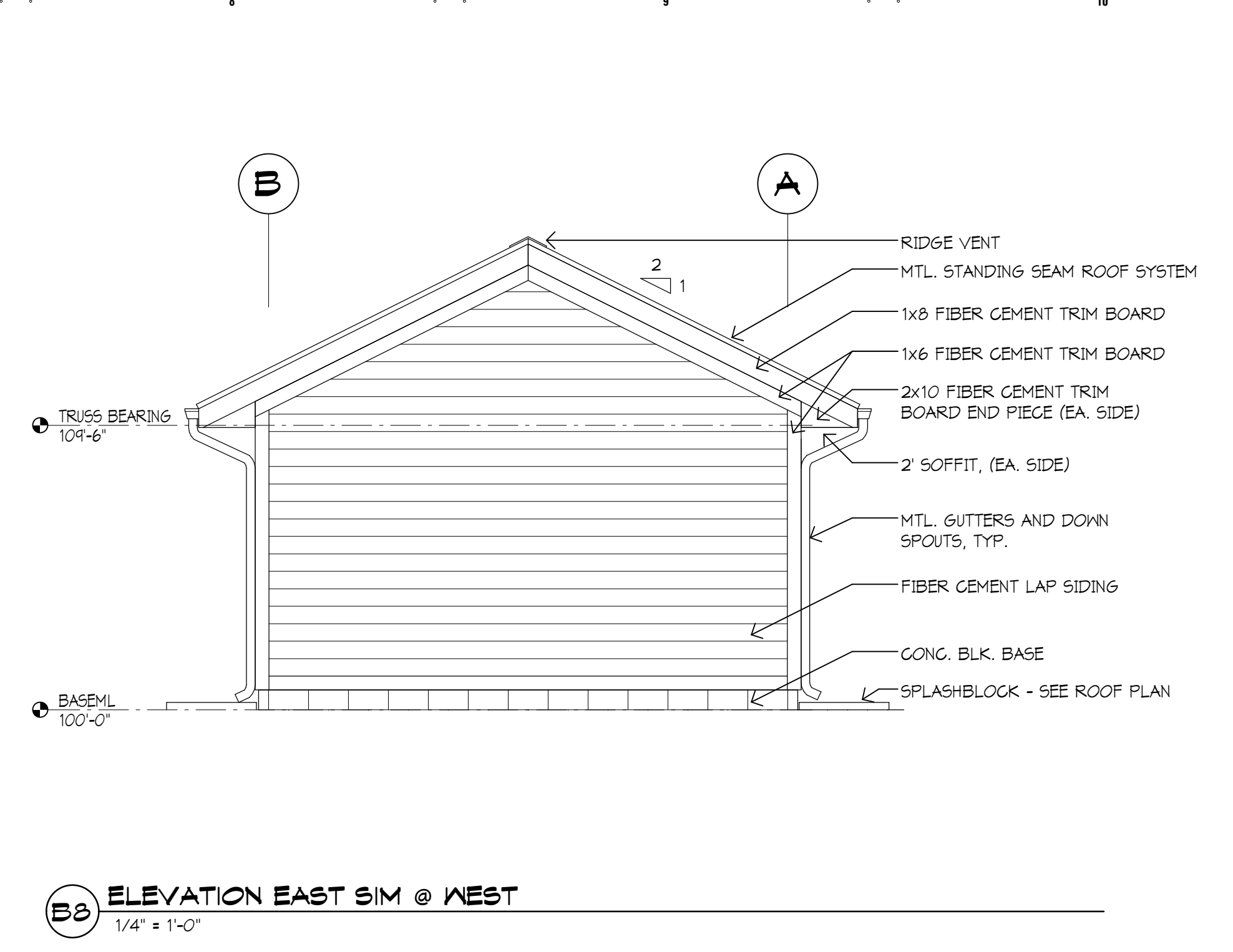
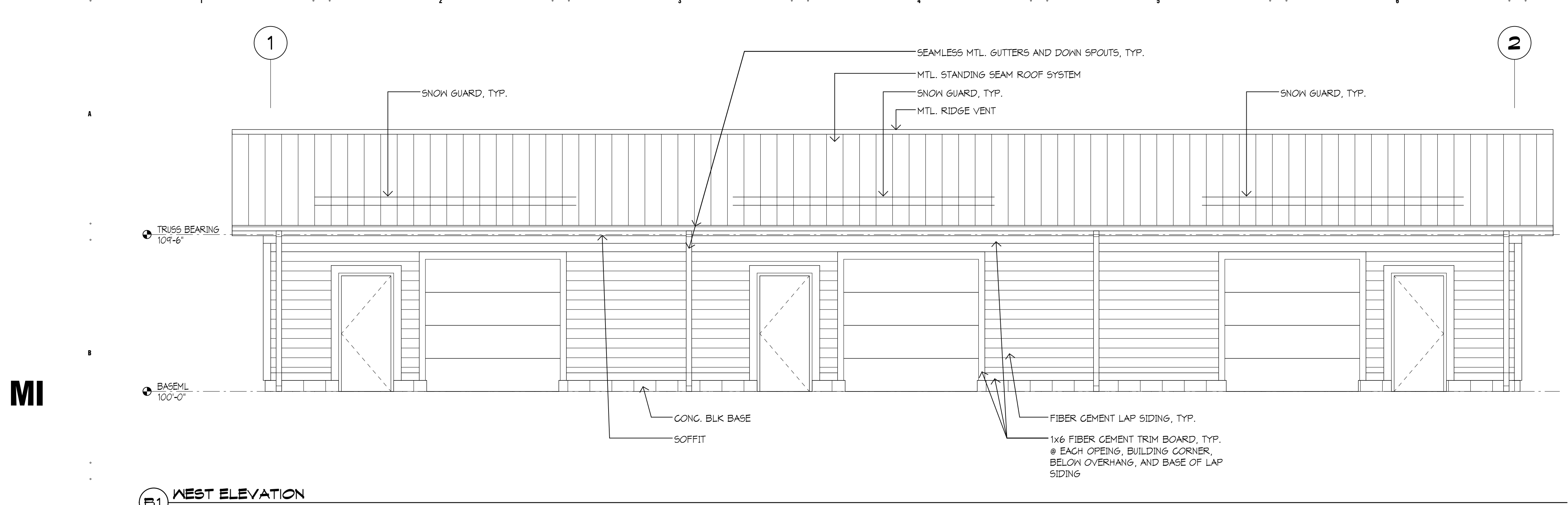
Description	Revisions	Date	Num

Comm: 134014
Date: June 4, 2013
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**EXTERIOR
ELEVATIONS AND
DOOR / FRAME
TYPES**

Scale: 1/4" = 1'-0"

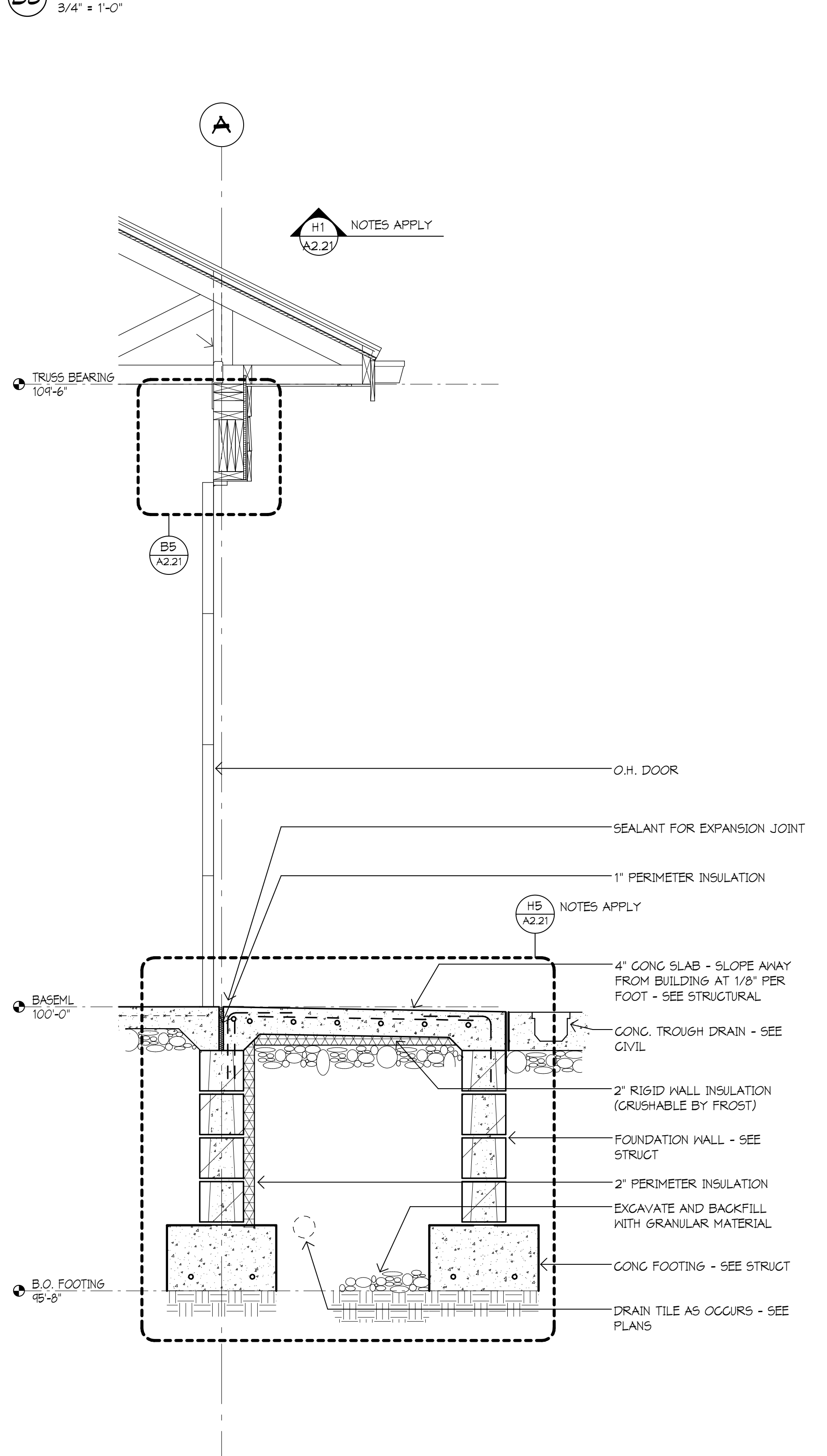
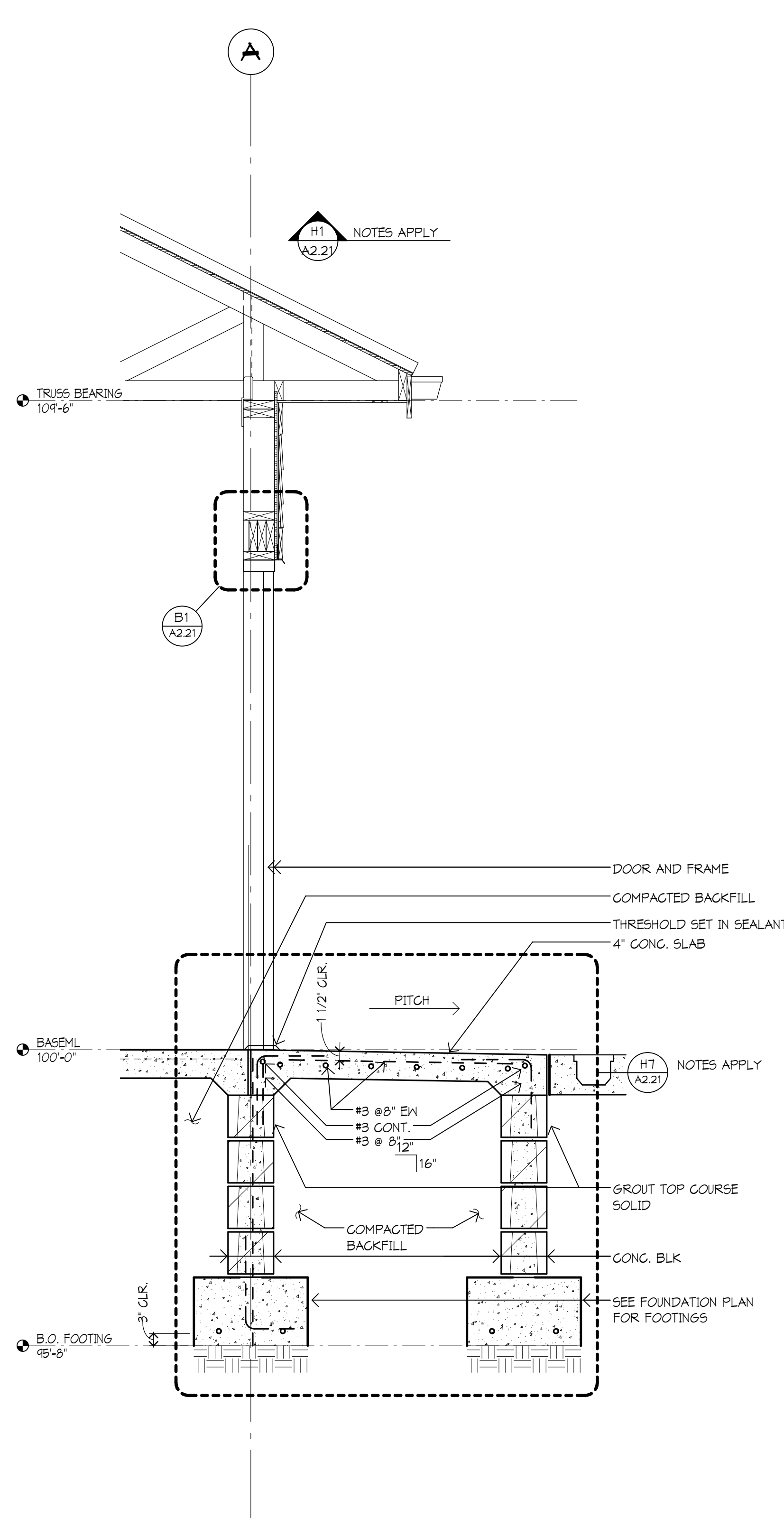
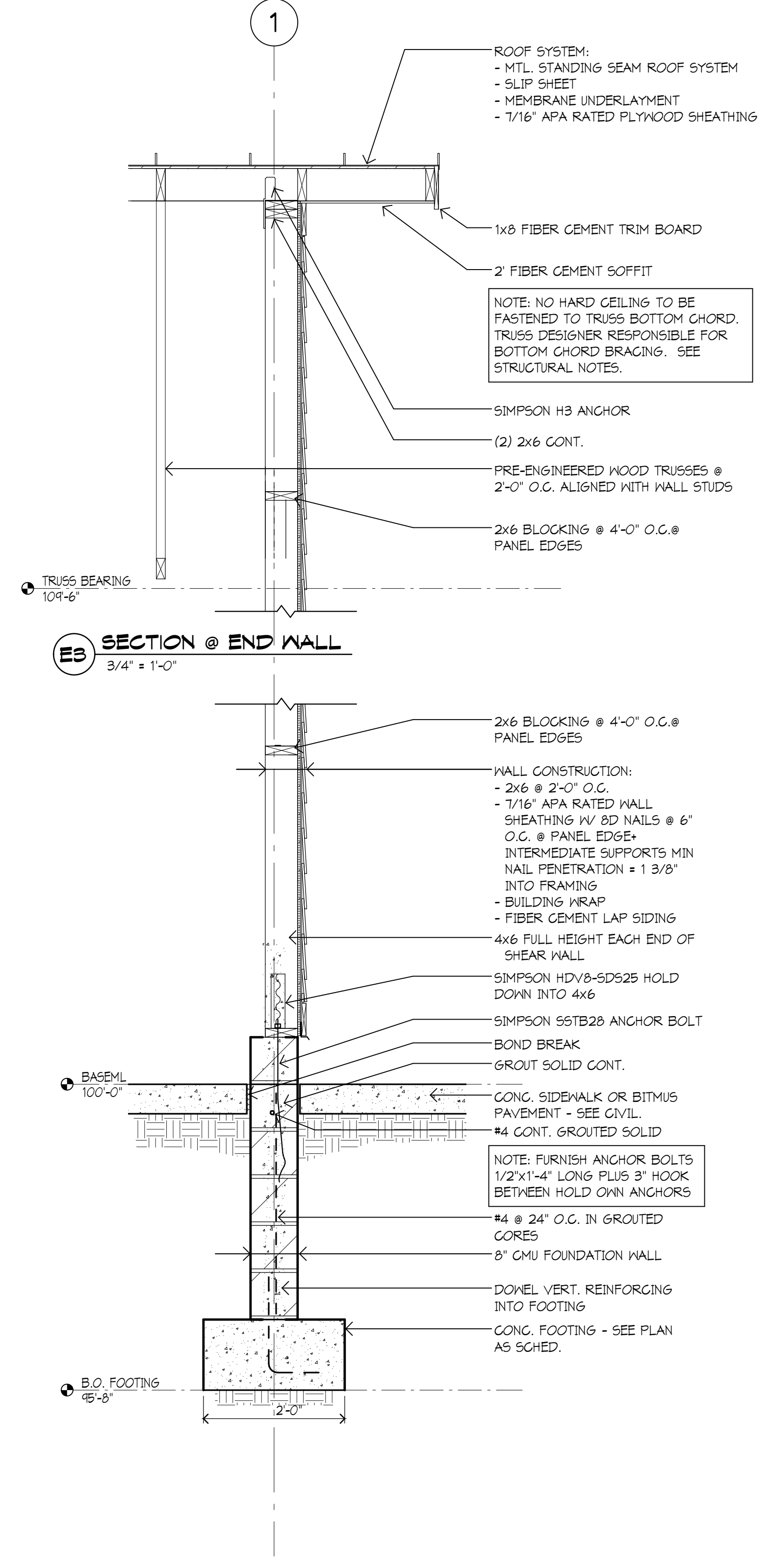
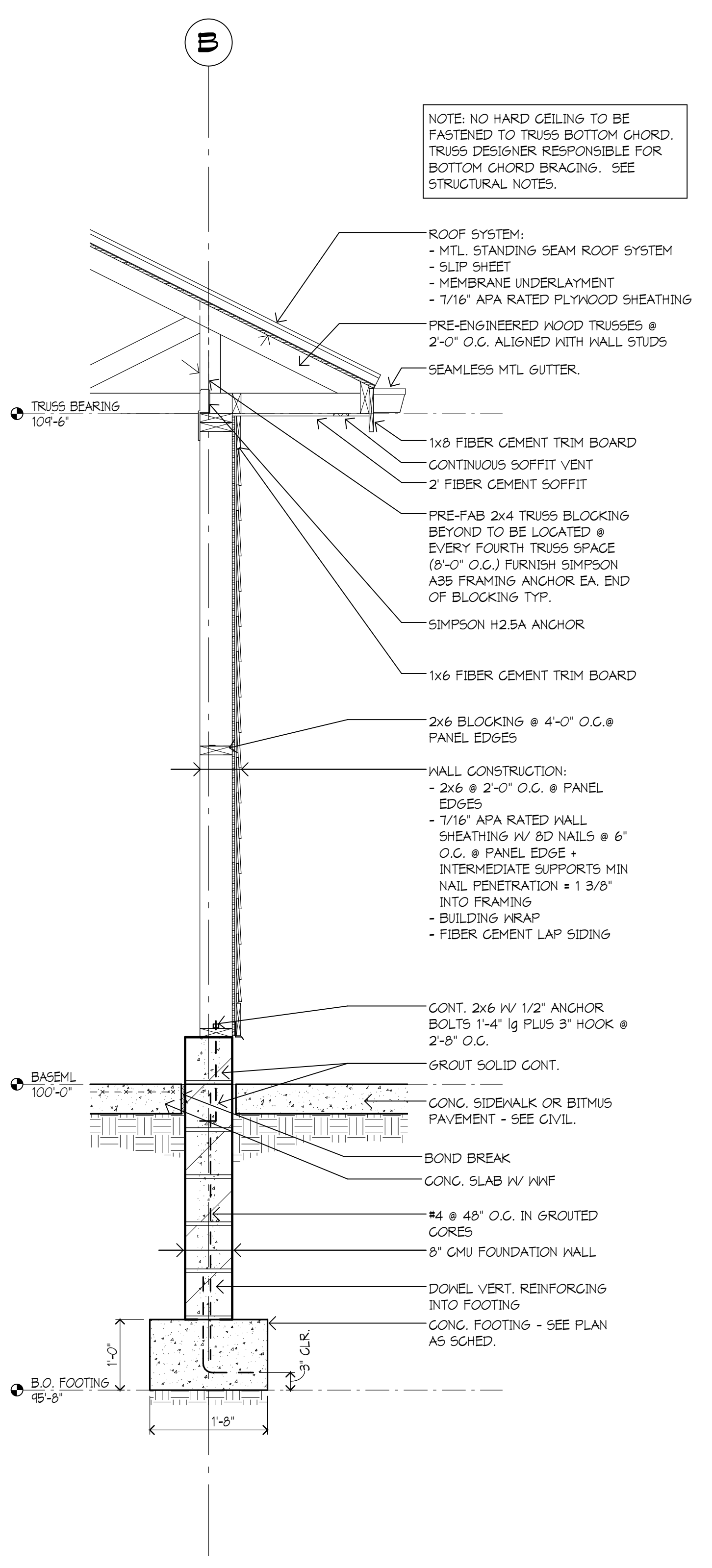
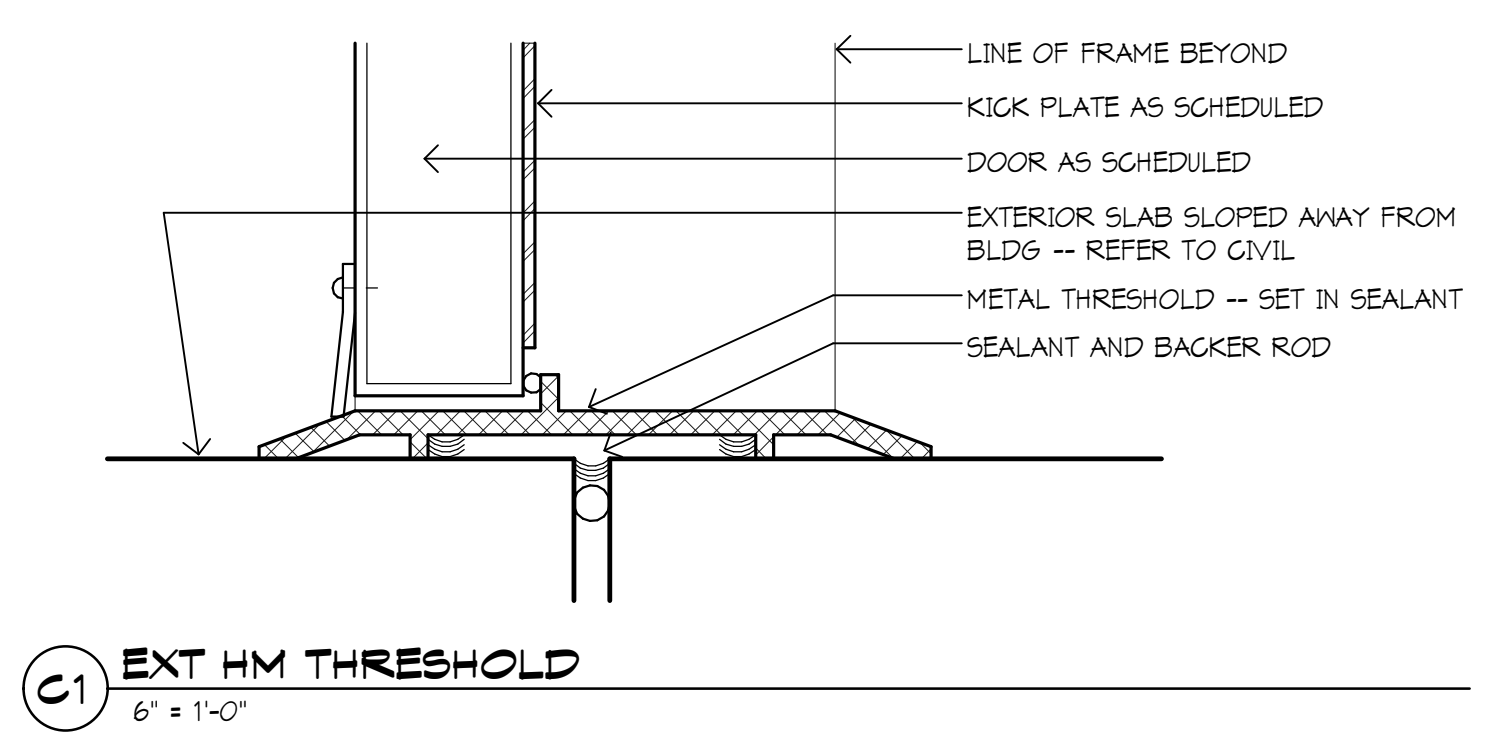
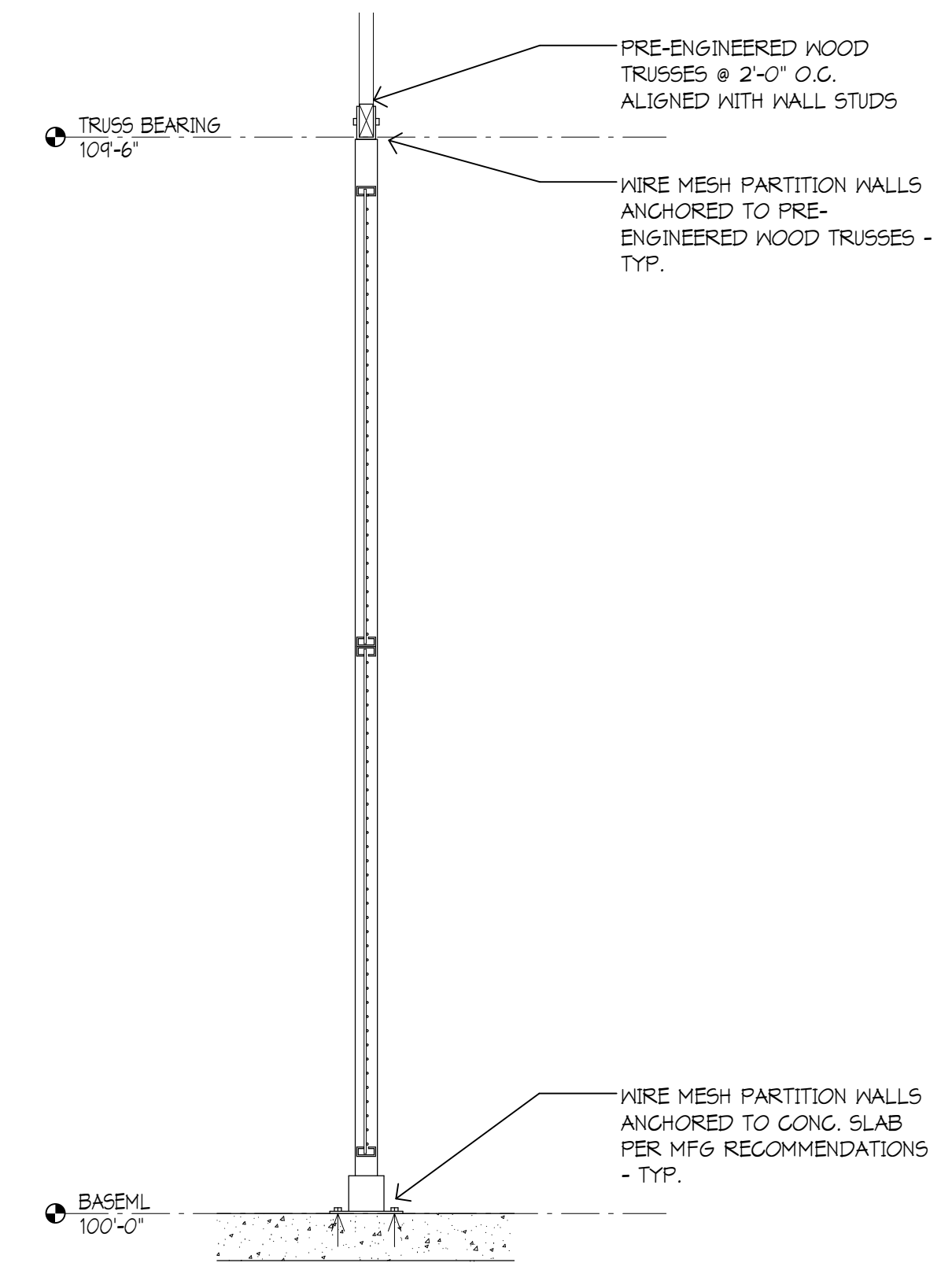
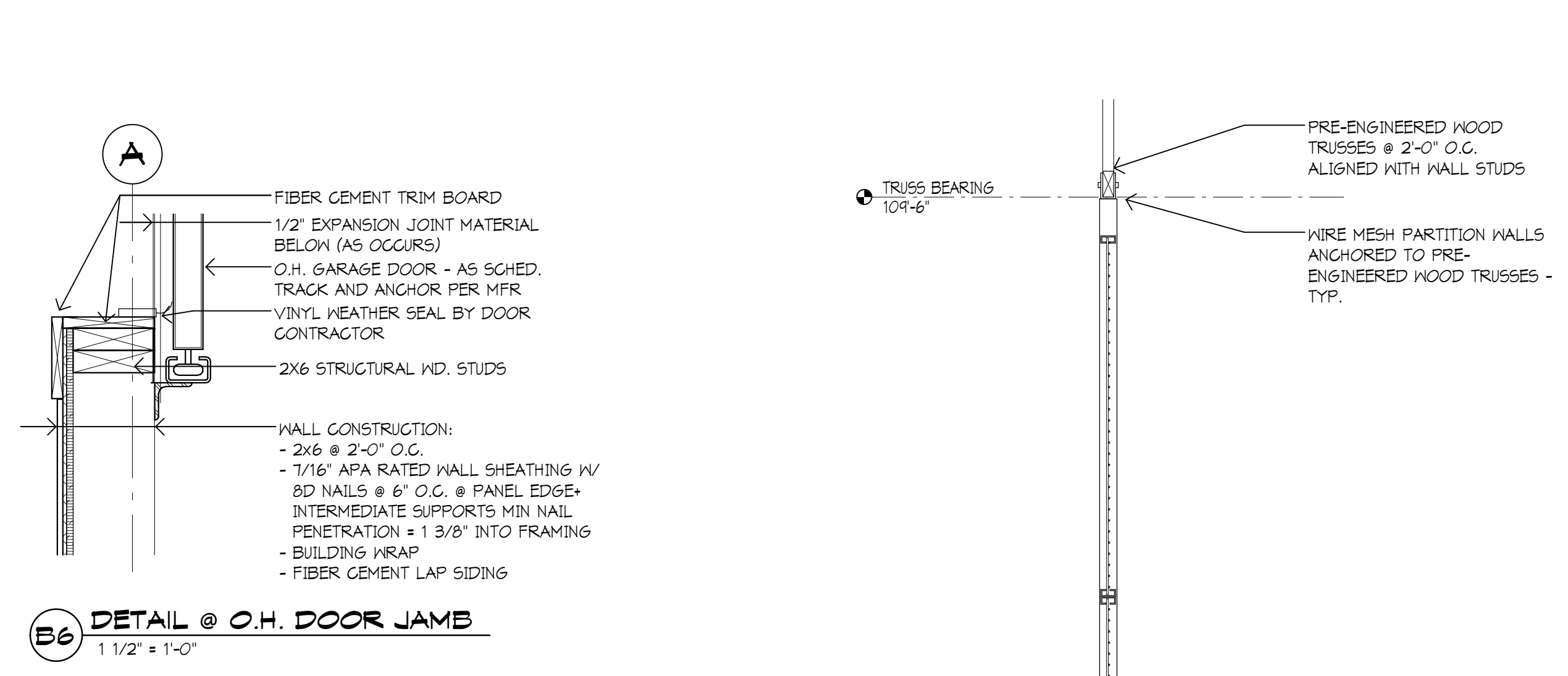
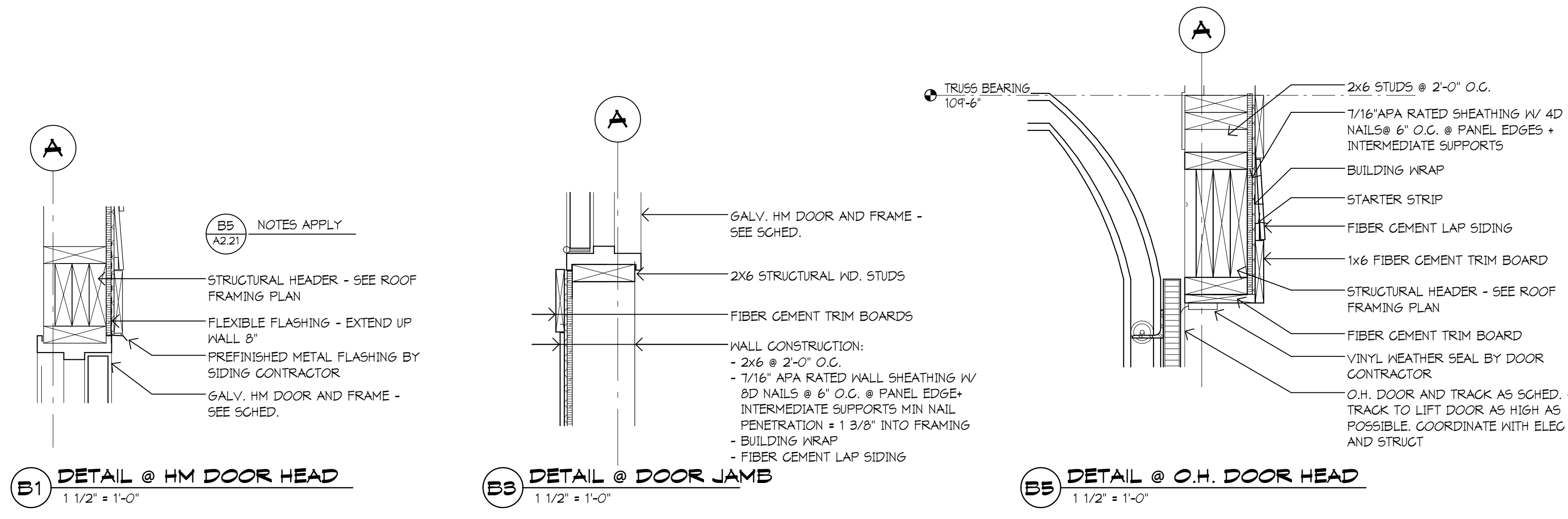
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Comm: 134014
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BUILDING AND WALL SECTIONS

Scale: As indicated

A2.21